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THERMAL STRAIN ANALYSIS OF ADVANCED MANNED SPACECRAFT HEAT SHIELDS

Final Report

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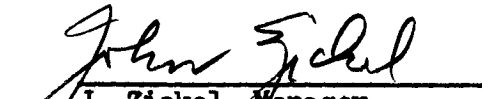
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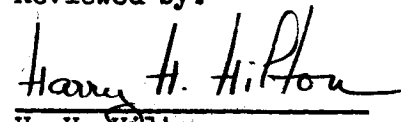
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ABSTRACT

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Numerical methods and computer programs are presented for the analysis of heat shields. The finite element technique is used to determine stresses and displacements developed in composite axisymmetric solids of arbitrary geometry subjected to axisymmetric thermal or mechanical loads. This technique is then applied to the development of an automated computer program for the analysis of axisymmetric heat shields subjected to axisymmetric thermal and pressure loadings. Finally, the numerical technique is extended to the analysis of heat shields subjected to non-axisymmetric thermal loading.

Several examples are presented to illustrate the application of the method and to demonstrate its validity. FORTRAN II card listings and descriptions of the use of the above programs are given in the appendices.

Author

TABLE OF CONTENTS

	<u>Page No.</u>
INTRODUCTION _____	1
PART I: METHOD OF ANALYSIS _____	3
A. INTRODUCTION _____	3
B. EQUILIBRIUM EQUATIONS FOR AN ARBITRARY FINITE ELEMENT _____	3
C. EQUILIBRIUM EQUATIONS FOR A SYSTEM OF FINITE ELEMENTS _____	8
D. SOLUTION OF EQUILIBRIUM EQUATIONS _____	10
E. ELEMENT STRESSES _____	12
PART II: GENERAL COMPUTER PROGRAM FOR THE ANALYSIS OF ARBITRARY AXISYMMETRIC STRUCTURES _____	13
A. INTRODUCTION _____	13
B. STIFFNESS OF TRIANGULAR RING _____	15
C. EQUILIBRIUM EQUATIONS FOR COMPLETE STRUCTURE _____	21
D. DETERMINATION OF DISPLACEMENTS AND STRESSES _____	22
E. COMPUTER PROGRAM _____	24
F. EXAMPLE _____	29
G. DISCUSSION _____	29

TABLE OF CONTENTS (cont'd)

	<u>Page No.</u>
PART III: AUTOMATED PROGRAM FOR AXISYMMETRIC HEAT SHIELDS SUBJECTED TO AXISYMMETRIC LOADS _____	32
A. INTRODUCTION _____	32
B. MESH GENERATION _____	33
C. DETERMINATION OF DISPLACEMENTS AND STRESSES _____	35
D. COMPUTER PROGRAM _____	36
E. EXAMPLES _____	40
F. DISCUSSION _____	43
PART IV: AUTOMATED PROGRAM FOR AXISYMMETRIC HEAT SHIELDS SUBJECTED TO NON-AXISYMMETRIC LOADS _____	45
A. INTRODUCTION _____	45
B. THEORY FOR THE ANALYSIS OF AN AXISYMMETRIC BODY SUBJECTED TO NON-AXISYMMETRIC LOADS _____	45
C. COMPUTER PROGRAM _____	55
D. EXAMPLES _____	59
E. DISCUSSION _____	65
APPENDIX A: SOLUTION OF EQUILIBRIUM EQUATIONS _____	A1
APPENDIX B: MATRIX FORMULATION OF THE LEAST SQUARES CURVE-FIT PROCEDURE _____	B1
APPENDIX C: MATHEMATICAL MODEL FOR SANDWICH SHELL _____	C1
APPENDIX D: PROGRAM LISTING-ARBITRARY AXISYMMETRIC STRUCTURES _____	D1
APPENDIX E: PROGRAM LISTING-AXISYMMETRIC HEAT SHIELDS _____	E1
APPENDIX F: PROGRAM LISTING-NON-AXISYMMETRIC HEAT SHIELDS _____	F1
APPENDIX G: SUMMARY OF EFFORTS TO SOLVE THE THERMAL STRAIN PROBLEM BY THE OVER-RELAXATION AND DIRECT INTEGRATION OF FINITE-DIFFERENCE FORMULATION _____	G1

LIST OF SYMBOLS

a_j, b_j, a_k, b_k = Element Dimensions

E = Modulus of Elasticity

u = Displacement in the r Direction

v = Displacement in the θ Direction

w = Displacement in the z Direction

α = Thermal Coefficient of Expansion

β = Over-Relaxation Factor

γ = Shear Strain

$\epsilon_{r,\theta,z}$ = Strain in the Radial, Circumferential and Longitudinal Direction

ν = Poisson's Ratio

$\sigma_{r,\theta,z}$ = Stress in the Radial, Circumferential and Longitudinal Direction

τ = Shearing Stress

$[]^T$ = Matrix Transpose

$[a]$ = Displacement Transformation Matrix

$[C]$ = Matrix of Elastic Coefficients

$[G_h]$ = Displacement Transformation Matrix - Harmonic n

$[S]$ = Matrix of Element Corner Forces

$[u]$ = Matrix of Element Corner Displacements

$[k]$ = Element Stiffness Matrix

$[F]$ = Nodal Point Displacements

$[R]$ = Nodal Point Loads

$[K]$ = Stiffness Matrix for Complete Structure

INTRODUCTION

The purpose of this investigation is the development of methods of analysis and digital computer programs to aid in establishing the structural integrity of manned spacecraft heat shields. The results of the analysis which are presented in this report indicate only the capabilities of the computer programs and do not necessarily represent the behavior of a specific heat shield. The final evaluation of the structural capability of a heat shield must be based on a certain amount of engineering judgement, in connection with the use of the computer programs.

In this investigation the finite element method is used to determine stresses and displacements developed in solids of revolution. First, a numerical procedure and a digital computer program are developed for the analysis of composite axisymmetric solids of arbitrary geometry subjected to axisymmetric thermal or mechanical loads. Second, this program is specialized to the analysis of axisymmetric heat shields. Finally, the same numerical technique is extended to the analysis of heat shields subjected to non-axisymmetric thermal loading. A description of the method of analysis and the use of the above computer programs is presented. In addition, FORTRAN II listings of the above programs are incorporated in this report.

During the initial phases of this contract, finite difference techniques were used to solve the governing differential equations for displacements of the system. However, considerable difficulty was encountered in the solution of the resulting set of linear equations. An iterative approach, coupled with over-relaxation techniques, resulted in inadequately convergent displacements. The direct solution technique gave a matrix for the set of simultaneous equations which was ill-conditioned. An additional difficulty of the finite difference technique was encountered in satisfying the boundary conditions at the edge of the heat shield. The finite element approach proved more practical and more versatile; therefore, the finite difference method was discontinued. A complete description of this initial investigation is given in Appendix G.

PART I: METHOD OF ANALYSIS

A. INTRODUCTION

The "finite element method" is a general method of structural analysis in which a continuous structure is replaced by a finite number of elements interconnected at a finite number of nodal points -- (such an idealization is inherent in the conventional analysis of frames and trusses). In this investigation the finite element method is applied to the determination of stresses and displacements developed in axisymmetric elastic structures of arbitrary geometry and material properties which are subjected to thermal and mechanical loads.

An assemblage of different types of axisymmetric elements is used to represent the continuous structure. Approximations are made on the displacements within each element of the system. Based on these approximations, equilibrium equations are developed for all elements. From "direct stiffness techniques", the equilibrium equations, in terms of unknown nodal point displacements, are developed at each nodal point. A solution of this set of equations constitutes a solution to the finite element system.

B. EQUILIBRIUM EQUATIONS FOR AN ARBITRARY FINITE ELEMENT

1. Strain-Displacement Relationship

The first step in the determination of the stiffness (corner forces in terms of corner displacements and temperature changes), of a

finite element is to assume a solution for the displacement field within the element. It is desirable that this assumed displacement field satisfies compatibility between other elements in the system. Based on this solution for the displacements within the element, it is possible to develop an expression for the strains at any point within the element in terms of the nodal points (corner) displacements. This expression in matrix form is

$$[\epsilon] = [a][u] \quad (1.1)$$

where $[\epsilon]$ is a column matrix of the M components of strain
 $[u]$ is a column matrix of the N nodal point displacements
 $[a]$ is an M x N strain-displacement transformation matrix - this matrix may be a function of space

2. Stress-Strain Relationship

For an elastic material, the stresses at any point within the element are expressed in terms of the corresponding strains by the elastic stress-strain relationship. Or, in matrix form

$$[\sigma] = [C][\epsilon] + [\tau] \quad (1.2)$$

where $\begin{bmatrix} \sigma \end{bmatrix}$ is a column matrix of the M components of stress
 $\begin{bmatrix} \epsilon \end{bmatrix}$ is a column matrix of the M components of strain
 $\begin{bmatrix} \tau \end{bmatrix}$ is a column matrix of the M components of thermal stress
 $\begin{bmatrix} C \end{bmatrix}$ is an MxM matrix of material property coefficients

The size (M) of these matrices will depend on the type of element being considered. The coefficients of matrices $\begin{bmatrix} C \end{bmatrix}$ and $\begin{bmatrix} \tau \end{bmatrix}$ will depend on material properties. Since $\begin{bmatrix} C \end{bmatrix}$ is completely arbitrary, anisotropic materials can be handled. Also, each element in the system may have different properties; therefore, composite structures are readily represented by the finite element idealization.

3. Internal Work

The internal work, or strain energy, which is associated with an infinitesimal volume element dV within the finite element is given by

$$dW_I = \frac{1}{2} (\epsilon_1 \sigma_1 + \epsilon_2 \sigma_2 + \dots + \epsilon_M \sigma_M) dV$$

or in matrix form

$$dW_I = \frac{1}{2} \begin{bmatrix} \epsilon \end{bmatrix}^T \begin{bmatrix} \sigma \end{bmatrix} dV \quad (1.3)$$

The substitution of Equation (1.2) into Equation (1.3) yields

$$dW_I = \frac{1}{2} \begin{bmatrix} \epsilon \end{bmatrix}^T \begin{bmatrix} C \end{bmatrix} \begin{bmatrix} \epsilon \end{bmatrix} dV + \frac{1}{2} \begin{bmatrix} \epsilon \end{bmatrix}^T \begin{bmatrix} \tau \end{bmatrix} dV \quad (1.4)$$

Equation (1.1) may be written in transposed form as

$$\begin{bmatrix} \epsilon \end{bmatrix}^T = \begin{bmatrix} u \end{bmatrix}^T \begin{bmatrix} a \end{bmatrix}^T \quad (1.5)$$

After Equations (1.1) and (1.5) are substituted into Equation (1.4), the internal work is given by

$$dW_I = \frac{1}{2} \begin{bmatrix} u \end{bmatrix}^T \begin{bmatrix} a \end{bmatrix}^T \begin{bmatrix} C \end{bmatrix} \begin{bmatrix} a \end{bmatrix} \begin{bmatrix} u \end{bmatrix} dV + \frac{1}{2} \begin{bmatrix} u \end{bmatrix}^T \begin{bmatrix} a \end{bmatrix}^T \begin{bmatrix} \tau \end{bmatrix} dV \quad (1.6)$$

The total strain energy stored within the element is found by integrating Equation (1.6) over the volume of the finite element. Or

$$W_I = \frac{1}{2} \begin{bmatrix} u \end{bmatrix}^T \int \begin{bmatrix} a \end{bmatrix}^T \begin{bmatrix} C \end{bmatrix} \begin{bmatrix} a \end{bmatrix} \begin{bmatrix} u \end{bmatrix} dV + \frac{1}{2} \begin{bmatrix} u \end{bmatrix}^T \int \begin{bmatrix} a \end{bmatrix}^T \begin{bmatrix} \tau \end{bmatrix} dV \quad (1.7)$$

4. External Work

The work supplied externally at the nodal points of the finite element is given by

$$W_E = \frac{1}{2} U_1 S_1 + \frac{1}{2} U_2 S_2 + \dots + \frac{1}{2} U_N S_N$$

or in matrix form

$$W_E = \frac{1}{2} \begin{bmatrix} u \end{bmatrix}^T \begin{bmatrix} S \end{bmatrix} \quad (1.8)$$

where $\begin{bmatrix} u \end{bmatrix}^T$ is a row matrix of the N nodal point displacements

$\begin{bmatrix} S \end{bmatrix}$ is a column matrix of the N corresponding nodal point forces

5. Energy Balance

The external work, Equation (1.8), is equated to the internal work, Equation (1.7), yielding

$$[u]^T [S] = [u]^T [k] [u] + [u]^T [L] \quad (1.9)$$

where the element stiffness matrix

$$[k] = \int [a]^T [C] [a] dV \quad (1.10)$$

and the thermal load matrix

$$[L] = \int [a]^T [\tau] dV \quad (1.11)$$

Equation (1.9) represents an energy balance (scalar equation) for a single nodal point displacement pattern. If the final displacements $[u_i]$ are assumed to be composed of N separate displacement patterns, $[\bar{u}_{i,j}]$ $j = 1, \dots, N$, and if the final forces $[S_i]$ are assumed to be composed of N corresponding sets of forces, $[S_{i,j}]$ $j = 1, \dots, N$, Equation (1.9) may be written as

$$[\bar{u}]^T [\bar{S}] = [\bar{u}]^T [k] [\bar{u}] + [\bar{u}]^T [L] \quad (1.12)$$

To eliminate the term $[\bar{u}]^T$, the displacement patterns must be selected in such a manner as to assure an inverse of $[\bar{u}]^T$. An acceptable matrix is a diagonal matrix of the final displacement, or

$$[\bar{u}]^T = [\bar{u}] - [u] \quad (1.13)$$

Equation (1.12) is now premultiplied by $[u]^{-1}$ yielding

$$[I][S] = [I][k][u] + [I][L] \quad (1.14)$$

where $[I]$ is a diagonal unit matrix.

Since only linear systems are considered, the N displacement patterns may be superimposed. Or

$$[S] = [k][u] + [L] \quad (1.15)$$

Since

$$s_i = \sum_{j=1, \dots, N} \bar{s}_{ij} \quad \text{and} \quad u_i = \bar{u}_{ii}$$

Equation (1.15) expresses nodal point forces in terms of nodal point displacements and temperature changes within the element.

C. EQUILIBRIUM EQUATIONS FOR A SYSTEM OF FINITE ELEMENTS

The first step in the procedure is to express all element forces in terms of external nodal point displacements for each element in the system. This is accomplished by expanding Equation (1.14) in terms of the N possible nodal point displacements; this will yield M matrix equations of the form

$$[S^m] = [k^m][r] + [L^m] \quad m = 1, \dots, M \quad (1.16)$$

where M is the total number of elements in the system.

The matrix $[k^m]$ is termed the complete stiffness of element m and involves only terms which are associated with the displacements of the connecting nodal points. Consequently, the majority of the coefficients of this matrix equation are zero. The matrix $[r]$ contains all possible nodal point displacements of the complete finite element system. The matrix $[S^m]$ is a column matrix containing the forces acting on element m in the direction of the nodal point displacements $[r]$. The thermal load matrix $[L^m]$ and the element stiffness matrix $[k^m]$ are given by Equations (1.11) and (1.10); however, the order (size) of these matrices has now been expanded to correspond with the total number of nodal point displacements.

In order to satisfy equilibrium of all nodal points, the sum of the internal element forces must be equal to the external nodal point loads.

Or

$$[P] = \sum_{m=1, \dots, M} [S^m] \quad (1.17)$$

where $[P]$ is the externally applied nodal point loads. The substitution of Equation (1.16) into Equation (1.17) yields

$$[P] = \sum_{m=1, \dots, M} [k^m][r] + \sum_{m=1, \dots, M} [L^m] \quad (1.18)$$

or rewritten in the following form:

$$[R] = [K][r] \quad (1.19)$$

where

$$[R] = [P] - \sum_{m=1, \dots, M} [L^m] \quad (1.20)$$

$$[K] = \sum_{m=1, \dots, M} [k^m] \quad (1.21)$$

Equation (1.19), which is an equilibrium relationship between external loads and internal forces, represents a system of N linear equations in terms of N unknown displacements.

D. SOLUTION OF EQUILIBRIUM EQUATIONS

Equation (1.19) represents the relationship between all nodal point forces and all nodal point displacements. Mixed boundary conditions are considered by rewriting Equation (1.19) in the following partitioned form:

$$\begin{bmatrix} R_a \\ \text{---} \\ R_b \end{bmatrix} = \begin{bmatrix} K_{aa} & K_{ab} \\ \text{---} & \text{---} \\ K_{ba} & K_{bb} \end{bmatrix} \begin{bmatrix} r_a \\ \text{---} \\ r_b \end{bmatrix} \quad (1.22)$$

where $\begin{bmatrix} R_a \end{bmatrix}$ = the specified nodal point forces
 $\begin{bmatrix} R_b \end{bmatrix}$ = the unknown nodal point forces
 $\begin{bmatrix} r_a \end{bmatrix}$ = the unknown nodal point displacements
 $\begin{bmatrix} r_b \end{bmatrix}$ = the specified nodal point displacements

Equation (1.22) may be expressed in terms of two separate equations, or

$$\begin{bmatrix} R_a \end{bmatrix} = \begin{bmatrix} K_{aa} \end{bmatrix} \begin{bmatrix} r_a \end{bmatrix} + \begin{bmatrix} K_{ab} \end{bmatrix} \begin{bmatrix} r_b \end{bmatrix} \quad (1.23)$$

$$\begin{bmatrix} R_b \end{bmatrix} = \begin{bmatrix} K_{ba} \end{bmatrix} \begin{bmatrix} r_a \end{bmatrix} + \begin{bmatrix} K_{bb} \end{bmatrix} \begin{bmatrix} r_b \end{bmatrix} \quad (1.24)$$

Equation (1.23) is rewritten in the following reduced form:

$$\begin{bmatrix} K_{aa} \end{bmatrix} \begin{bmatrix} r_a \end{bmatrix} = \begin{bmatrix} \bar{R}_a \end{bmatrix} \quad (1.25)$$

where the modified load vector, $\begin{bmatrix} \bar{R}_a \end{bmatrix}$ is given by

$$\begin{bmatrix} \bar{R}_a \end{bmatrix} = \begin{bmatrix} R_a \end{bmatrix} - \begin{bmatrix} K_{ab} \end{bmatrix} \begin{bmatrix} r_b \end{bmatrix} \quad (1.26)$$

In Part II of this report, the Gauss-Seidel iterative technique is used to solve Equation (1.25) for the unknown nodal point displacements $\begin{bmatrix} r_a \end{bmatrix}$. Appendix A gives a direct solution approach which is used in the automated computer programs for the thermal stress analysis of heat shields, Parts III and IV of this report.

E. ELEMENT STRESSES

After the nodal point displacements have been determined, the strains within any element in the system are evaluated by the direct application of Equation (1.1). The corresponding stresses are calculated from the stress-strain relationship, Equation (1.2).

PART II GENERAL COMPUTER PROGRAM FOR THE ANALYSIS
OF ARBITRARY AXISYMMETRIC STRUCTURES

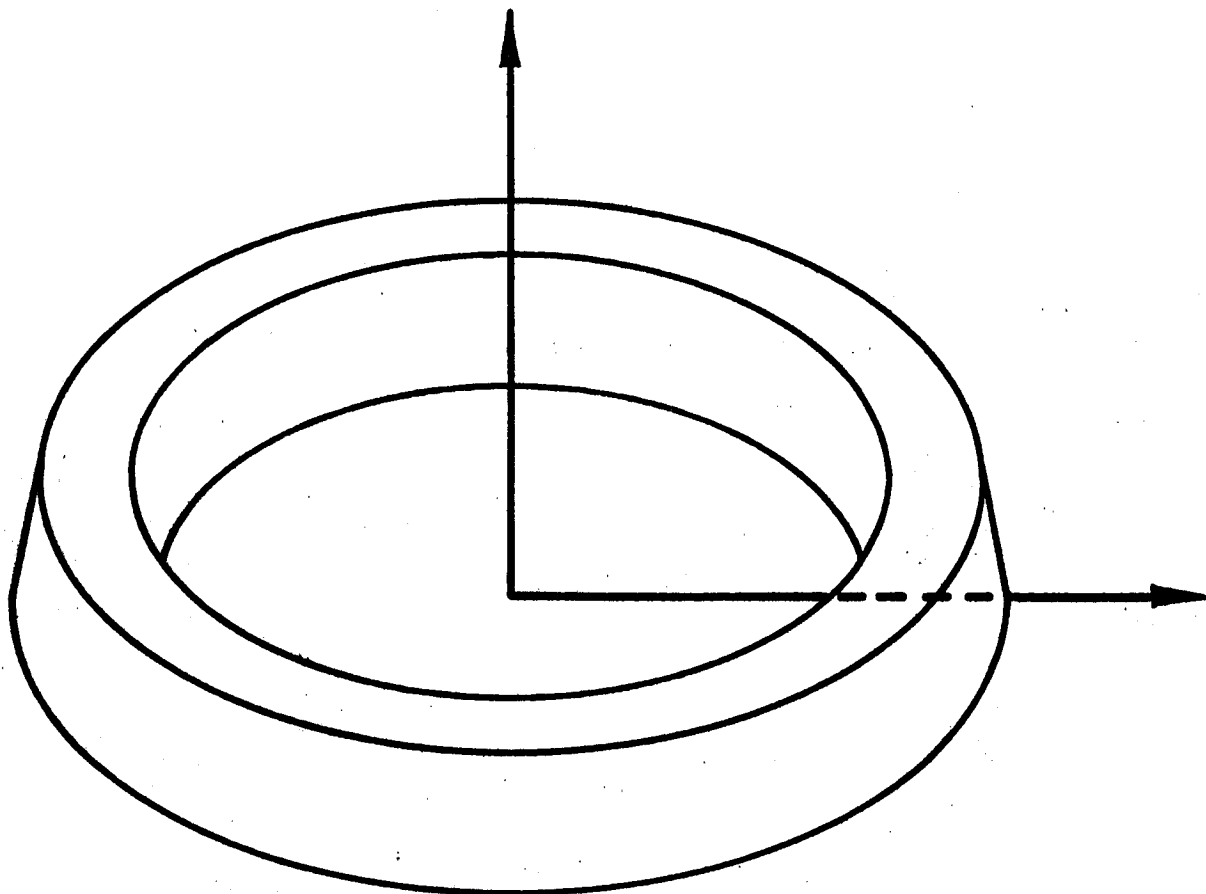
A. INTRODUCTION

The stress analysis of an axisymmetric structure of arbitrary shape, subjected to thermal and mechanical loads is of considerable practical interest. Although the governing differential equations have been known for many years, closed form solutions have been obtained for only a limited number of structures. Thus, the investigator must often rely on experimental or numerical procedures to solve this problem.

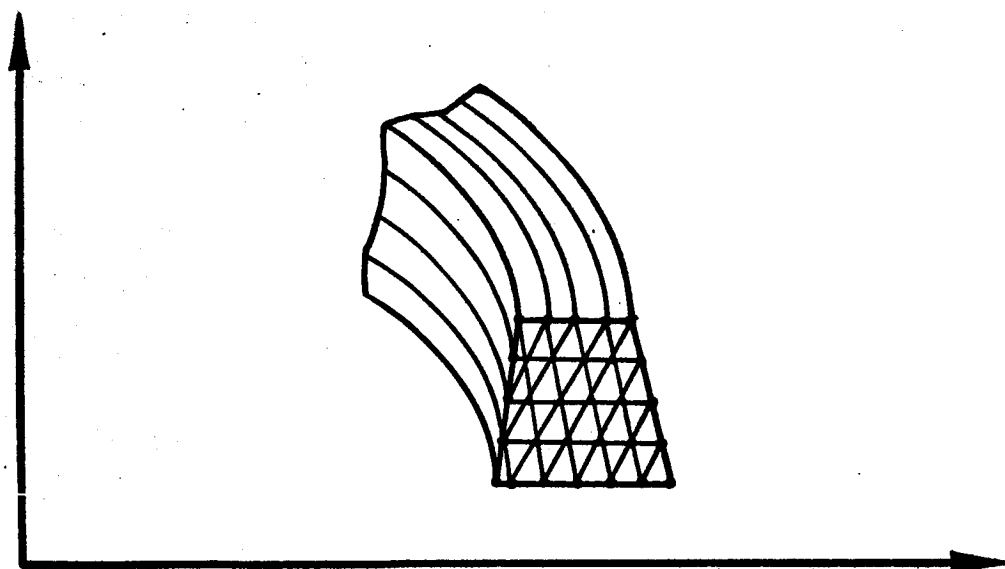
Experimental methods, such as Photoelasticity, have proven to be versatile tools in the analysis of many axisymmetric structures. However, for structures composed of several different materials or structures with thermal loading, this approach is limited.

The finite difference method, which involves the replacement of the derivatives in the differential equations and boundary conditions with difference equations, has been the most popular of the numerical techniques. However, for structures of composite materials and of arbitrary geometry, the procedure is difficult to apply.

In this section the finite element method is used to determine the stresses and displacements developed within arbitrary, elastic solids of revolution subjected to thermal or mechanical axisymmetric loads. The



a. ACTUAL RING



b. TRIANGULAR ELEMENT APPROXIMATION

FIG. 2.1 THE FINITE ELEMENT IDEALIZATION

finite element approach replaces the continuous structure with a system of triangular rings interconnected at a finite number of nodal points (joints). Loads acting on the structure are replaced by statically equivalent concentrated forces acting at the nodal points of the finite element system. Figure 2.1 illustrates a finite element idealization of a typical axisymmetric solid.

B. STIFFNESS OF TRIANGULAR RING

1. Strain-Displacement Relationship

Continuity between elements of the system is maintained by requiring that within each element "lines initially straight remain straight in their displaced position". This linear displacement field, which is illustrated in Figure 2.2, is defined in terms of $u(r,z)$ and

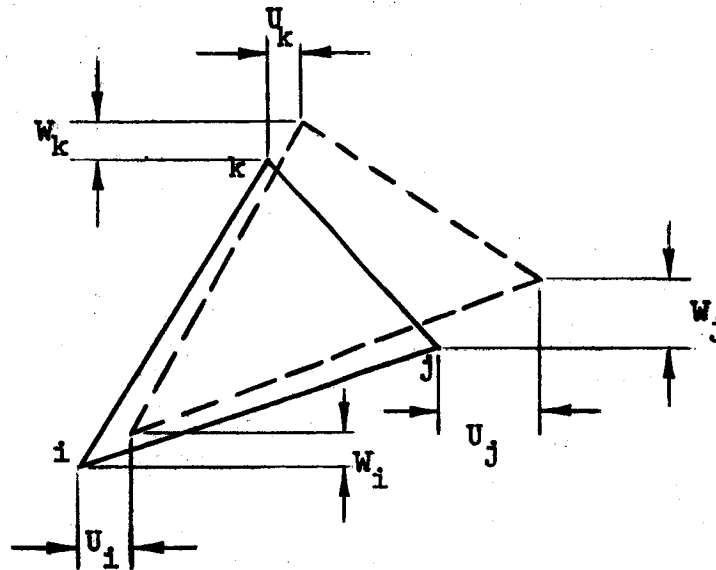


FIG. 2.2 ASSUMED DISPLACEMENT PATTERN

$w(r,z)$ by equations of the following form:

$$u(r,z) = C_1 + C_2 r + C_3 z \quad (2.1a)$$

$$w(r,z) = C_4 + C_5 r + C_6 z \quad (2.1b)$$

If Equations (2.1a) and (2.1b) are evaluated at the three corners i, j, k of the triangle, the following set of equations is obtained:

$$\begin{bmatrix} u_i \\ w_i \\ u_j \\ w_j \\ u_k \\ w_k \end{bmatrix} = \begin{bmatrix} 1 & r_i & z_i & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & r_i & z_i \\ 1 & r_j & z_j & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & r_j & z_j \\ 1 & r_k & z_k & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & r_k & z_k \end{bmatrix} \begin{bmatrix} C_1 \\ C_2 \\ C_3 \\ C_4 \\ C_5 \\ C_6 \end{bmatrix} \quad (2.2)$$

By solving the system of Equations (2.2) for the constants C_1, \dots, C_6 , they are expressed in terms of corner displacements. The strains in the rz -plane are obtained from the assumed displacement field by considering the basic definition of strain.

$$\bar{\epsilon}_r = \frac{\partial u}{\partial r} = c_2 \quad (2.3a)$$

$$\bar{\epsilon}_z = \frac{\partial w}{\partial z} = c_6 \quad (2.3b)$$

$$\bar{\gamma}_{rz} = \frac{\partial u}{\partial z} + \frac{\partial w}{\partial r} = c_3 + c_5 \quad (2.3c)$$

At any point within the element the tangential strain $\bar{\epsilon}_\theta$ is

$$\bar{\epsilon}_\theta(r,z) = \frac{u(r,z)}{r}$$

The average tangential strain is found by averaging the strains at the vertices of the triangle, or

$$\epsilon_\theta = \frac{1}{3} \left(\frac{u_i}{r_i} + \frac{u_j}{r_j} + \frac{u_k}{r_k} \right) \quad (2.3d)$$

After eliminating the constants C_n between Equations (2.2) and (2.3), the average element strains are expressed in terms of corner displacements by the following matrix equation:

$$\begin{bmatrix} \epsilon_r \\ \epsilon_z \\ \epsilon_\theta \\ \gamma_{rz} \end{bmatrix} = \frac{1}{\lambda} \begin{bmatrix} b_j - b_k & 0 & b_k & 0 & -b_j & 0 \\ 0 & a_k - a_j & 0 & -a_k & 0 & a_j \\ \frac{\lambda}{3r_i} & 0 & \frac{\lambda}{3r_j} & 0 & \frac{\lambda}{3r_k} & 0 \\ a_k - a_j & b_j - b_k & -a_k & b_k & a_j & -b_j \end{bmatrix} \begin{bmatrix} u_i \\ w_i \\ u_j \\ w_j \\ u_k \\ w_k \end{bmatrix} \quad (2.4a)$$

or in symbolic form

$$[\epsilon] = [a][u] \quad (2.4b)$$

where

$$a_j = r_j - r_i$$

$$a_k = r_k - r_i$$

$$b_j = z_j - z_i$$

$$b_k = z_k - z_i$$

$$\lambda = a_j b_k - a_k b_j$$

The geometry of a typical triangle is illustrated in Figure 2.3

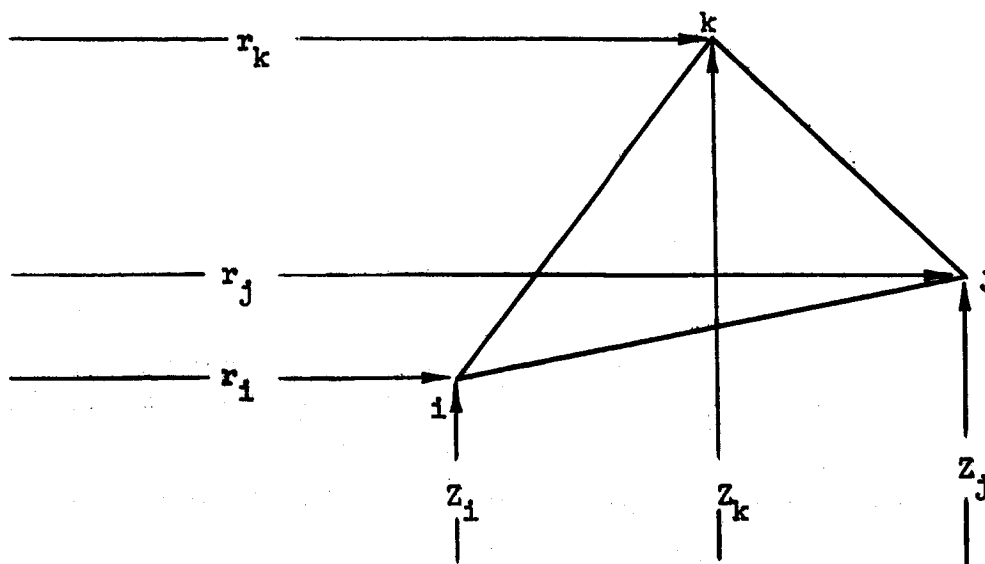


FIG. 2.3 ELEMENT DIMENSIONS

2. Stress-Strain Relationship

One important advantage of the finite element approach is that structures with anisotropic materials can be treated. In general, the stress-strain relationship is of the form

$$\begin{bmatrix} \sigma_r \\ \sigma_z \\ \sigma_\theta \\ \sigma_{rz} \end{bmatrix} = \begin{bmatrix} c_{11} & c_{12} & c_{13} & c_{14} \\ c_{21} & c_{22} & c_{23} & c_{24} \\ c_{31} & c_{32} & c_{33} & c_{34} \\ c_{41} & c_{42} & c_{43} & c_{44} \end{bmatrix} \begin{bmatrix} \epsilon_r \\ \epsilon_z \\ \epsilon_\theta \\ \gamma_{rz} \end{bmatrix} + \begin{bmatrix} \tau_r \\ \tau_z \\ \tau_\theta \\ \tau_{rz} \end{bmatrix} \quad (2.5a)$$

or symbolically $[\sigma] = [c][\epsilon] + [\tau] \quad (2.5b)$

where $[\tau]$ is the matrix of thermal stresses for a given temperature change. For example, the stress-strain relationship for an isotropic material is given by

$$\begin{bmatrix} \sigma_r \\ \sigma_z \\ \sigma_\theta \\ \sigma_{rz} \end{bmatrix} = \frac{E}{(1+\nu)(1-2\nu)} \begin{bmatrix} 1-\nu & \nu & \nu & 0 \\ \nu & 1-\nu & \nu & 0 \\ \nu & \nu & 1-\nu & 0 \\ 0 & 0 & 0 & \frac{1-2\nu}{2} \end{bmatrix} \begin{bmatrix} \epsilon_r \\ \epsilon_z \\ \epsilon_\theta \\ \gamma_{rz} \end{bmatrix} + \begin{bmatrix} \tau \\ \tau \\ \tau \\ 0 \end{bmatrix} \quad (2.6)$$

$$\text{where } \tau = \frac{E\alpha}{(1-2\nu)} \Delta T \quad (2.7)$$

3. Element Stiffness

The stiffness of a typical triangular ring, which is an expression for corner forces in terms of corner displacements, is given by Eq. (1.10) as

$$[k] = \int [a]^T [c] [a] dV$$

And the thermal load matrix is given by Eq. (1.11) as

$$[L] = \int [a]^T [\tau] dV$$

Since the coefficients in matrices $[a]$ and $[C]$ are assumed not to be a function of space the above equations reduce to

$$[k] = V [a]^T [c] [a] \quad (2.8)$$

$$[L] = V [a]^T [\tau] \quad (2.9)$$

If a one-radian segment is considered, an approximate expression for the volume of a triangular ring segment is

$$V = \bar{r} A \quad (2.10)$$

where \bar{r} is the average radius given by

$$\bar{r} = (r_i + r_j + r_k)/3 \quad (2.11)$$

and A is the cross-sectional area given by

$$A = (a_j b_k - a_k b_j)/2 \quad (2.12)$$

From Eq. (1.15) the six corner forces acting at the vertices of a one-radian triangular segment is given in terms of the six corner displacements and the temperature change within the element by

$$[S] = [k] [u] + [L] \quad (2.13)$$

C. EQUILIBRIUM EQUATIONS FOR COMPLETE STRUCTURE

The equilibrium of the complete system of triangular rings, which is an expression for nodal point loads in terms of nodal point displacements, is given by the following matrix equation:

$$[R] = [K] [r] \quad (2.14)$$

The stiffness matrix $[K]$ and the load matrix $[R]$ are determined by "direct stiffness" techniques as indicated in the previous section, Eqs. (1.20) and (1.21). In addition to the thermal loads, the $[R]$ matrix is composed of concentrated external forces acting at the nodal points of the system. Hence, pressures acting on the boundary of a segment of the structure are replaced by statically equivalent forces acting at the nodal points.

Mixed boundary conditions are considered by a simple transformation of Eq. (2.14); Eqs. (1.22) to (1.26) give the details of this modification.

D. DETERMINATION OF DISPLACEMENT AND STRESSES

Equation (2.14) is solved for the unknown nodal point displacements by the application of the well-known Gauss-Seidel iterative procedure. This involves the repeated calculation of new displacements from the equation

$$r_n^{(s)} = K_{nn}^{-1} \left[R_n - \sum_{i=1, \dots, n-1} K_{ni} r_i^{(s)} - \sum_{i=n+1, \dots, n} K_{ni} r_i^{(s-1)} \right] \quad (2.15)$$

where n is the number of the unknown and s is the cycle of iteration.

The only modification of the procedure introduced in this analysis is the simultaneous application of Equation (2.15) to both components

of displacements at each nodal point. Therefore, r_n and R_n become vectors with r and z components.

The rate of convergence of the Gauss-Seidel procedure can be greatly increased by the use of an over-relaxation factor. This factor is applied by first calculating the change in displacement $\Delta r_n^{(s)}$ of nodal point n and then determining the new displacement from the following equation:

$$r_n^{(s)} = r_n^{(s-1)} + \beta \Delta r_n^{(s)} \quad (2.16)$$

where β is the over-relaxation factor.

The solution of an over-relaxation factor, which gives the best convergence, depends on the characteristics of the particular problem. However, experience has indicated that for most structures, the optimum over-relaxation factor is between 1.8 and 1.95.

Since only the non-zero terms in Equation (2.14) are developed and stored by the computer program, a solution of several hundred equations is possible, thereby making it possible to solve large finite element systems.

For each element the average strains are calculated directly from the nodal point displacements by the application of Equation (2.4). The

average element stresses are then determined from the stress-strain relationship for the element, Equation (2.5). In addition, at each nodal point, stresses are computed by averaging the stresses in all elements attached to the point.

E. COMPUTER PROGRAM

The complete analysis of an axisymmetric solid by the finite element method involves three separate phases. First, the structure must be idealized by a system of triangular rings. Second, this system is solved for displacements and stresses from given nodal point forces. Third, the displacements and stresses are presented graphically for further evaluation and utilization.

The selection of the system of finite elements for a particular problem is completely arbitrary; therefore, axisymmetric structures, composed of many interacting components, of practically any shape may be handled. By numbering all elements and nodal points, in a convenient manner, the system can be defined in the form of three numerical arrays - nodal point array, element array and boundary point array. The nodal point array contains the coordinates and the loads or displacements that are associated with each nodal point of the system. The element array contains, for each element in the system, the location of the element (the three nodal point numbers defining the corners of the element and other possible parameters which are associated with the element (i.e.,

elastic constants, density and temperature changes). The boundary array indicates the type of restraint that exists at boundary nodal points.

These three arrays, along with some basic control information, constitute the numerical input for the digital computer program. The program itself performs three major tasks in the analysis of the finite element system of triangular rings. First, the equilibrium equations for the system are formed from the basic numerical description of the system. Second, this set of equations is solved for the nodal point displacements. Third, the internal stresses are determined from these displacements.

1. Input Information

To define the system of finite elements, all nodal points and elements are numbered as illustrated in Figure 2.4. Based on this numbering system, the following sequence of punched cards constitutes the input to the computer program.

a. TITLE CARD (72H)

Columns 2 to 72 of this card contain information to be printed with results

b. CONTROL CARD (614, 2E12.5)

Columns 1 - 4 Number of elements
5 - 8 Number of nodal points
9 - 12 Number of restrained boundary points
13 - 16 Cycle interval for print of unbalanced forces

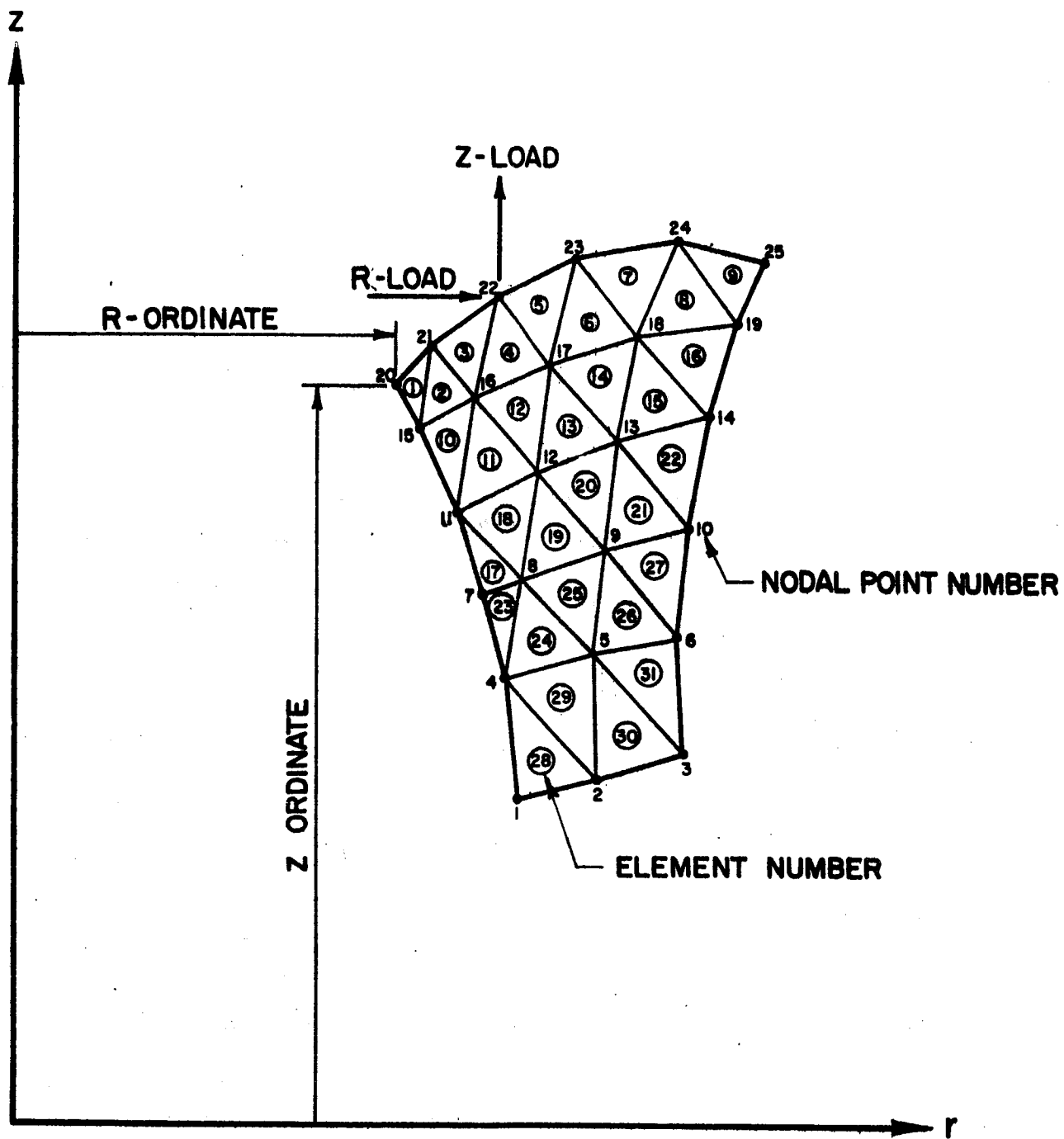


FIG. 2.4 NUMBERING SYSTEM FOR ELEMENTS AND NODAL POINTS

17-20 Cycle interval for print of results
21-24 Maximum number of cycles problem may
run
25-36 Convergence limit for unbalanced forces
37-48 Over-relaxation factor

c. ELEMENT ARRAY - 1 card per element (4I4, 4E12.4,
F8.4)

Column	1-4	Element Number	
	5-8	Nodal point number i	
	9-12	Nodal point number j	} in counter- clockwise order
	13-16	Nodal point number k	
	17-28	Modulus of elasticity E	
	29-40	Density of element ρ	
	41-52	Poisson's ratio ν	
	53-64	Coefficient of thermal expansion α	
	65-72	Temperature change within element ΔT	

d. NODAL POINT ARRAY - 1 card per nodal point (1I4,
4F8.1, 2F12.8)

Column	1-4	Nodal point number	
	5-12	R-ordinate	
	13-20	Z-ordinate	
	21-38	R-load	} Total force acting on a one radian segment.
	29-36	Z-load	
	37-48	R-displacement	
	49-60	Z-displacement	

On free nodal points, the displacements are initial guesses for the iterative solution. On restrained nodal points, the input displacements are the specified final displacements of the nodal point.

- e. BOUNDARY POINT ARRAY - 1 card per restrained nodal point (2I4, IF6)

Columns 1-4 Nodal point numbers

5-8 0 if point is fixed in both directions

1 if point is fixed in the R-direction

2 if point is free to move along a line of slope S

9-16 Slope S (type 2 points only)

2. Output Information

The following information is generated and printed by the computer program:

- a. Input Data
- b. Nodal Point Displacement
- c. Average Element Stresses
- d. Average Nodal Point Stresses

3. Timing

For the IBM 7094 the computational time required by the program is approximately $0.004 \times n \times m$ seconds, where n equals the number of nodal points and m equals the number of cycles of iteration. Depending on the desired degree of convergence, it may be necessary to extend the iteration process.

4. Program Listing

A card listing of the FORTRAN II source deck for the general axisymmetric program is included in Appendix D of this report. This program is compiled for a maximum size of 550 elements or 340 nodal points.

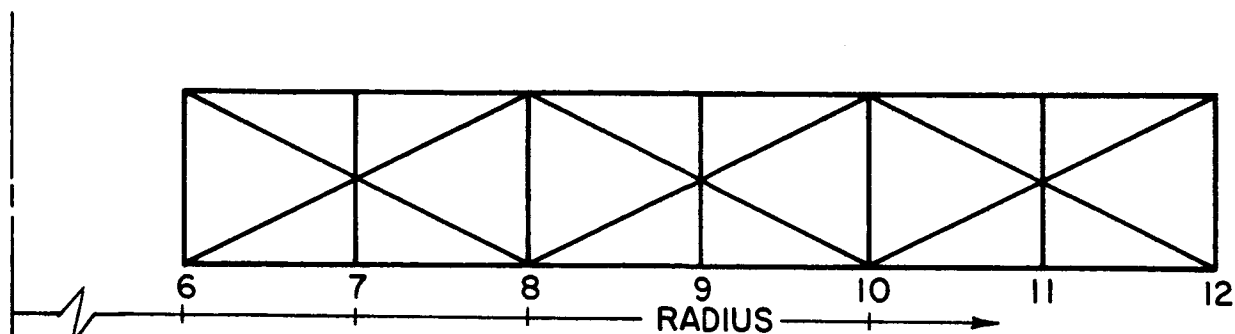
F. EXAMPLE

An infinite cylinder subjected to steady state temperature distribution, for which an exact solution is known, is selected as a means of verifying the finite element analysis. A finite element idealization of a section of the cylinder is shown in Figure 2.5a. The temperature distribution which is assumed constant within each element, is plotted in Figure 2.5b. The hoop stresses are compared with the exact solution in Figure 2.5c. Considering the coarse mesh, agreement with the exact solution is very good except at the two boundary points. This discrepancy is due to the fact that nodal point stresses are calculated by averaging the stresses in the attached elements. Therefore, the boundary nodal point stress reflects the average stress in the elements near the boundary.

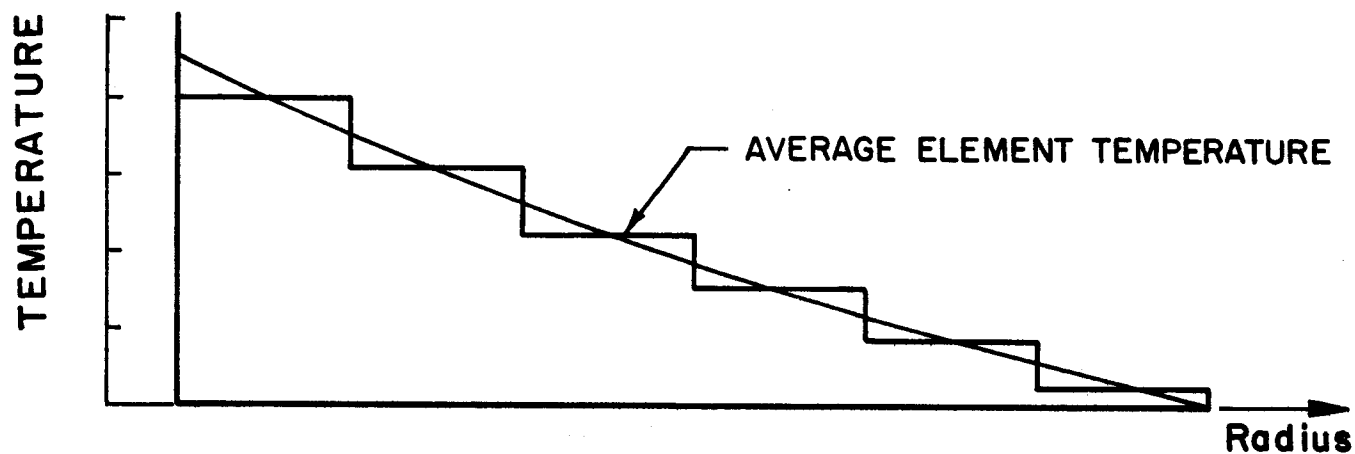
In general, good boundary stresses are obtained by plotting the interior stresses and extrapolating to the boundary. This type of engineering judgement is always necessary in evaluating results from a finite element analysis.

G. DISCUSSION

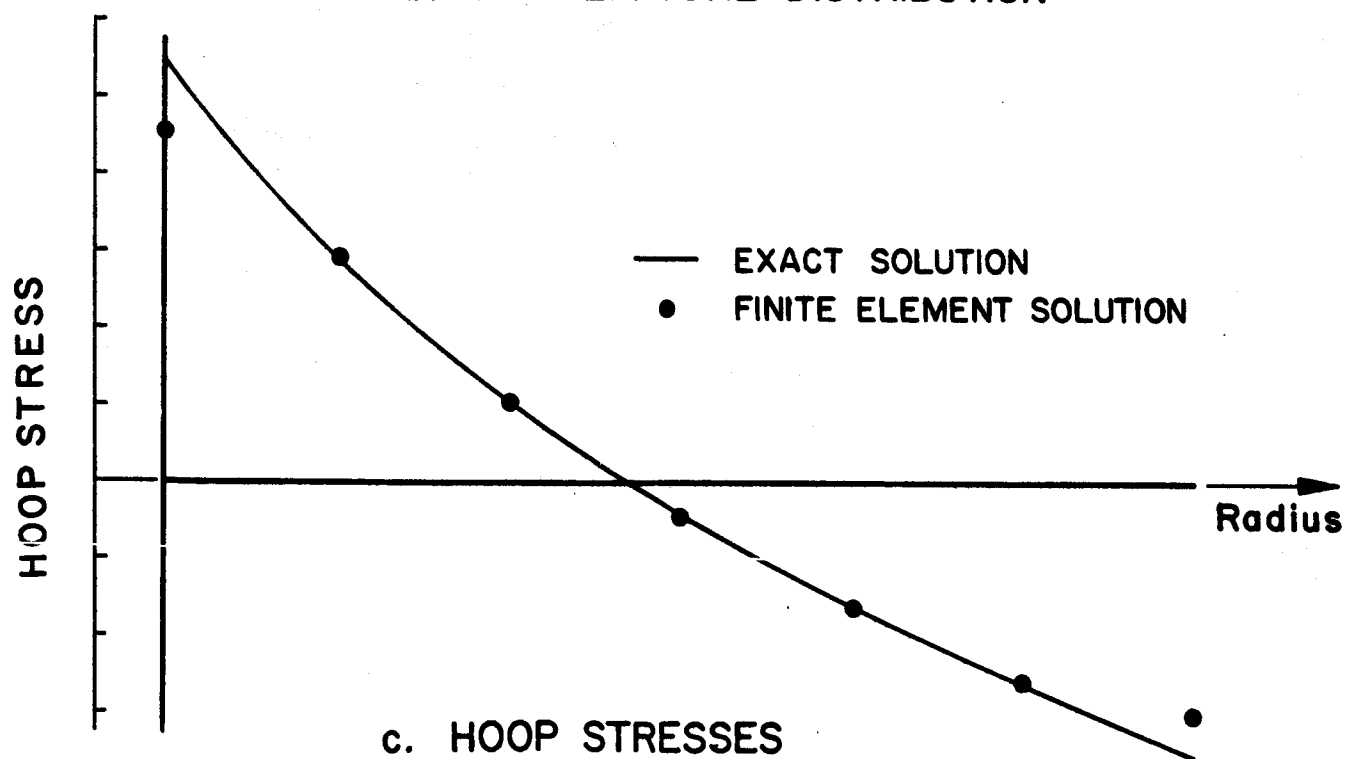
This section demonstrates the application of the finite element technique to the stress analysis of structures of revolution. The approach reduces the stress analysis to a simple procedure. In order to use the



a. FINITE ELEMENT IDEALIZATION - INFINITE CYLINDER



b. TEMPERATURE DISTRIBUTION



c. HOOP STRESSES

FIG. 2.5 ANALYSIS OF INFINITE CYLINDER

program, it is only necessary to select an element idealization of the structure and to supply the computer program with data that numerically define the system of elements. Therefore, the program may be used as a tool in design since changes in materials and geometry of the structures may involve only minor changes in the input data.

In addition, the program may be extended to include the effects of anisotropic materials. For this case, the input to the program must be expanded to include the general elastic constants defined by Eq. (2.5).

For the analysis of a specific type of structure, this program can be further automated by incorporating a mesh generator and the calculation of temperature-dependent material properties. In the next section of this report the program is specialized to the thermal stress analysis of axisymmetric heat shields for manned spacecraft.

PART III AUTOMATED PROGRAM FOR AXISYMMETRIC HEAT
SHIELDS SUBJECTED TO AXISYMMETRIC LOADS

A. INTRODUCTION

The general computer program for the analysis of arbitrary axisymmetric structure, as given in the previous section, can be applied to the thermal stress analysis of heat shields. However, the use of this program for such a complex structure involves a large amount of detail work to select the finite element idealization and to prepare the computer input. In addition, the convergence of the Gauss-Seidel iteration procedure is slow for this type of structure and a solution may require an excessive amount of computer time.

By restricting the general computer program to the analysis of heat shields and by automating the input, a considerably more efficient program can be developed. The geometry of the heat shield is supplied to the computer program in the form of R and Z coordinates and ablator thickness at various points along the bond line. The required triangular mesh and the temperature at the grid points are generated within the program. Material properties at various temperatures are supplied in tabular form and the program automatically develops analytical expressions for the material properties by least square techniques. The flanges of the sandwich shell are idealized by special conical shell elements and the honeycomb core material is treated as a separate material. This approach

eliminates the need for the establishing of a pseudo-thickness for the composite sandwich shell. The solution of the equilibrium equations, which was previously obtained by an iterative approach is accomplished by a direct solution procedure. Because of their significance, stresses within the sandwich plates and at the bond line are included in the computer output.

B. MESH GENERATION

A typical finite element idealization of the cross-section of a heat shield is shown in Figure 3.1. The basic element in this system is a quadrilateral ring, which in turn is composed of two triangular rings (Part II). In this particular case, the sandwich shell is represented by the first two rows of elements and the ablator is idealized by four rows of elements; there are 30 points in the meridional direction. The specific mesh configuration is a variable which is supplied to the computer program.

In general, the geometry of the shell is given by the R-Z coordinates of the points at the bond line between the sandwich shell and the ablator. The points on lines perpendicular to the bond line inside the variable thickness ablator and inside the constant thickness sandwich shell are generated automatically within the program. Thin shell cone elements are used to represent the face plates of the sandwich shell.

From a given temperature distribution at the bond line, the grid point temperatures are assumed to be constant within the shell and are assumed to vary parabolically within the ablator.

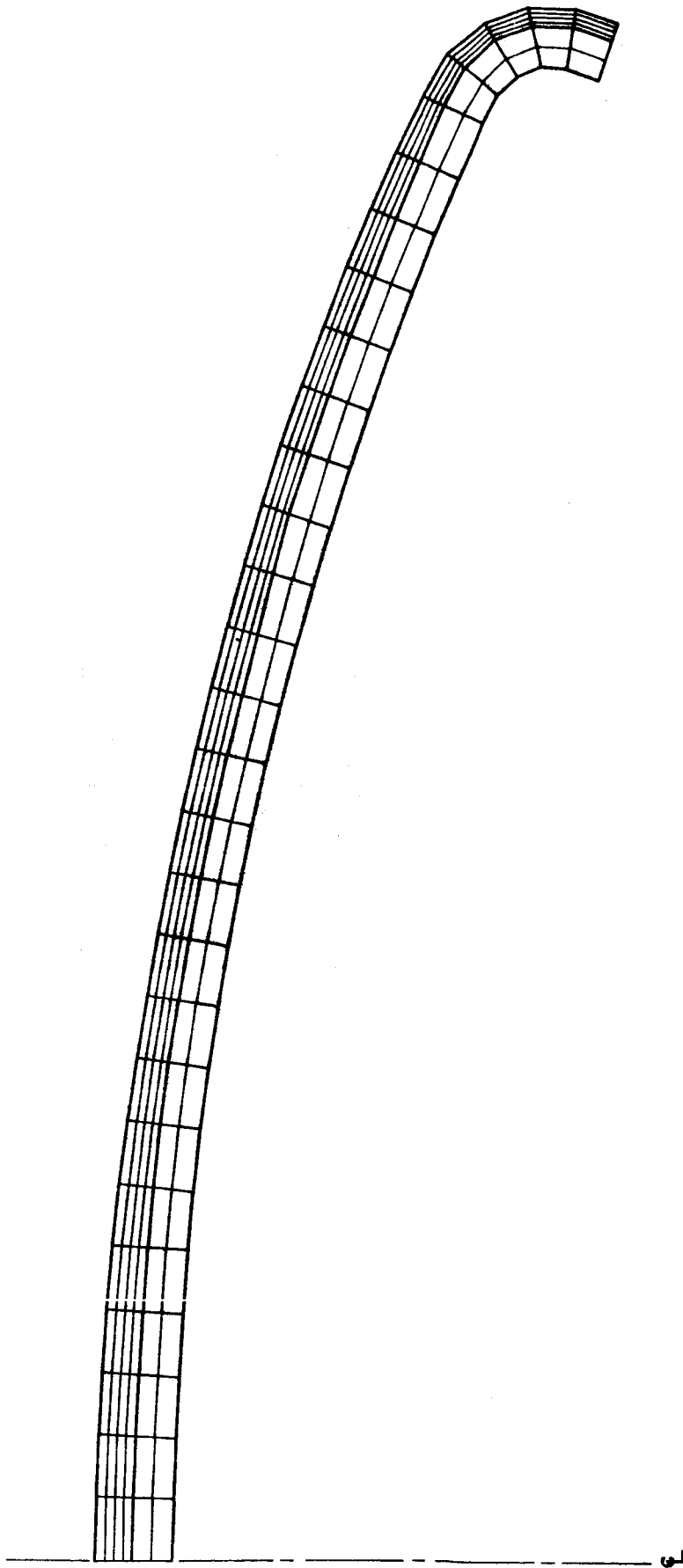


Fig. 3.1 Typical Finite Element Idealization of Heat Shield

C. DETERMINATION OF DISPLACEMENTS AND STRESSES

Based on temperature dependent material properties, the equilibrium relationship for each quadrilateral ring is developed and then combined to form the equilibrium equations of the complete system of rings. Similarly the stiffness properties of the face plates of the sandwich shell are incorporated into the equilibrium of the system. The axisymmetric behavior of a typical conical element is a special case of the non-axisymmetric behavior which is given in Appendix C. The unknowns in this set of equations are the vertical and radial displacements at each grid point in the system. The satisfying of possible displacement boundary conditions requires that these equations be modified as indicated by Equation (1.25). Because of the physical characteristics of the heat shield, the resulting set of equations is in band form. Appendix A indicates the necessary modification to restrict the standard Gaussian elimination procedure to the solution of symmetrical band systems. This approach results in a definite increase in capacity and speed over the iterative technique and it eliminates the problem of convergence.

After the equilibrium equations are solved for the unknown grid point displacements, the average stresses within each triangular ring are calculated as indicated in Part II of this report. Based on the stresses in the two triangular rings, average quadrilateral stresses are calculated for each quadrilateral ring in the system.

D. COMPUTER PROGRAM

The first step in the stress analysis of an axisymmetric heat shield is to select points at the bond line at regular intervals along the meridian of the shield. A quadrilateral mesh is automatically developed by the program from the R and Z coordinates. The material properties vs temperature for the ablator and bond are supplied to the computer in tabular form and the computer program automatically determines analytical expressions for the properties by a least square procedure (see Appendix B for details of method). Finally, the grid points which are to be restrained and the external loads which act at grid points are specified.

1. Input Information

The following sequence of punched cards numerically defines the heat shield to be analyzed.

a. FIRST CARD - (72H)

Columns 1 to 72 of this card contains information
to be printed with results

b. SECOND CARD - (6I5, 2F10.2)

Columns 1 - 5 Number of points along the shield - NMAX
6 - 10 Number of points thru the thickness-MMAX
11 - 15 Location of bond line - MB
16 - 20 Number of material property cards - NP

- 21 - 25 Number of points with radial and axial loads - NL
- 26 - 30 Number of additional boundary conditions - NB
- 31 - 40 Surface temperature of ablator
- 41 - 50 Zero stress temperature

c. THIRD CARD - Properties of Sandwich Core (4F10.2)

Columns 1 - 10 Modulus of elasticity

11 - 20 Poisson's ratio

21 - 30 Coefficient of thermal expansion

31 - 40 Thickness of core

d. FOURTH CARD - Properties of Sandwich Face Plates (4F12.2)

Columns 1 - 10 Modulus of elasticity

11 - 20 Poisson's ratio

21 - 30 Coefficient of thermal expansion

31 - 40 Thickness of single face plate

e. GEOMETRY CARDS - (4F10.2)

One card per point along shield, in order from axis of symmetry to edge (NMAX cards).

Columns 1 - 10 R-ordinate at bond line

11 - 20 Z-ordinate at bond line

21 - 30 Temperature at bond line

31 - 40 Normal thickness of ablator

f. MATERIAL PROPERTY CARDS - (4F10.2)

One card for each temperature (NP cards)

Columns 1 - 10 Temperature

11 - 20 Modulus of elasticity of ablative material

21 - 30 Modulus of elasticity of bond material

31 - 40 Coefficient of thermal expansion for ablator and bond materials

g. LOAD CARDS - (2I5, 2F10.2)

One card for each point which is loaded externally

(NL cards). N and M specify the grid location of the point.

Columns 1 - 5 N (Meridional direction)

6 - 10 M (Thickness direction)

11 - 20 Radial Load

21 - 30 Axial Load } Total load acting on one radian segment

h. BOUNDARY CONDITION CARDS - (3I5)

One card for each point which is restrained (NB cards).

N and M specify the grid location of the point.

Columns 1 - 5 N (Meridional direction)

6 - 10 M (Thickness direction)

11 - 15 Boundary Code

Code = 1 point fixed in R-direction

Code = 2 point fixed in Z-direction

Code = 3 point fixed in both the R and Z-directions

2. Output Information

The following information is generated and printed by the computer program:

- a. Input data
- b. Least squares evaluation of the temperature dependent material property data
- c. Coordinates and temperatures of all grid points
- d. R and Z displacement at all grid points
- e. Average stresses in quadrilateral rings
- f. Stresses in sandwich face plates
- g. Stresses in bond layer

3. Timing

The computer time required by this program for an axisymmetric analysis of a heat shield is approximately

$$\text{time} = A + B \cdot (NMAX) \cdot (MMAX)^2 \text{ (seconds)}$$

The constants A and B depend on the specific computer system which is employed. For the IBM 7094 A=20 and B=0.02, and the time required for a 30 x 7 mesh is 50 seconds.

4. Program Listing

A card listing of the FORTRAN II source deck for the automated computer program for the axisymmetric stress analysis of heat shields is given in Appendix E. The program is compiled for a maximum

grid size of 40 points in the meridional direction and 10 points through the thickness. Material properties can be specified by a maximum of 50 cards.

E. EXAMPLES

Several axisymmetric analyses of a heat shield were conducted to evaluate the significance of the various structural parameters. A typical finite element idealization of the heat shield is shown in Figure 3.1.

1. Effect of Mesh Size

The first example was selected to illustrate the effect of mesh size on the accuracy of the displacements and stresses developed within the heat shield. For a structure fixed at the edge, typical results of two analyses with different meshes are shown in Figure 3.2. This example illustrates that two layers of elements in the sandwich shell are adequate for the purposes of predicting stresses. It is of interest to note that the stress distribution varies linearly within the sandwich shell, thereby confirming the assumption made in thin shell theory. The displacements for these two analyses differed by less than one percent.

2. Effect of Ablator Thickness on Stress Distribution

Figure 3.3 shows typical results of three analyses of heat shields with different ablator thicknesses. In general, the magnitude of

- Four Layers in Shell
Four Layers in Ablator
- × Two Layers in Shell
Six Layers in Ablator

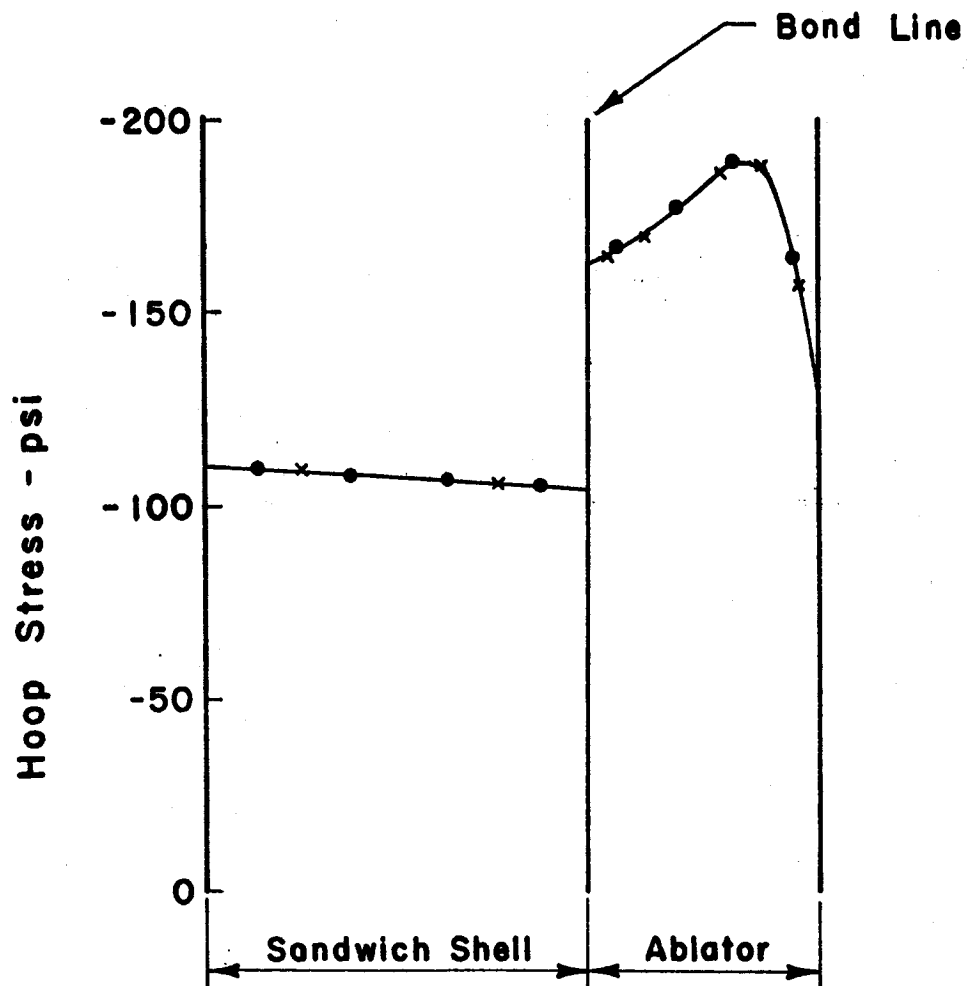


Fig.3.2 Effect of Mesh Size on Stress Distribution

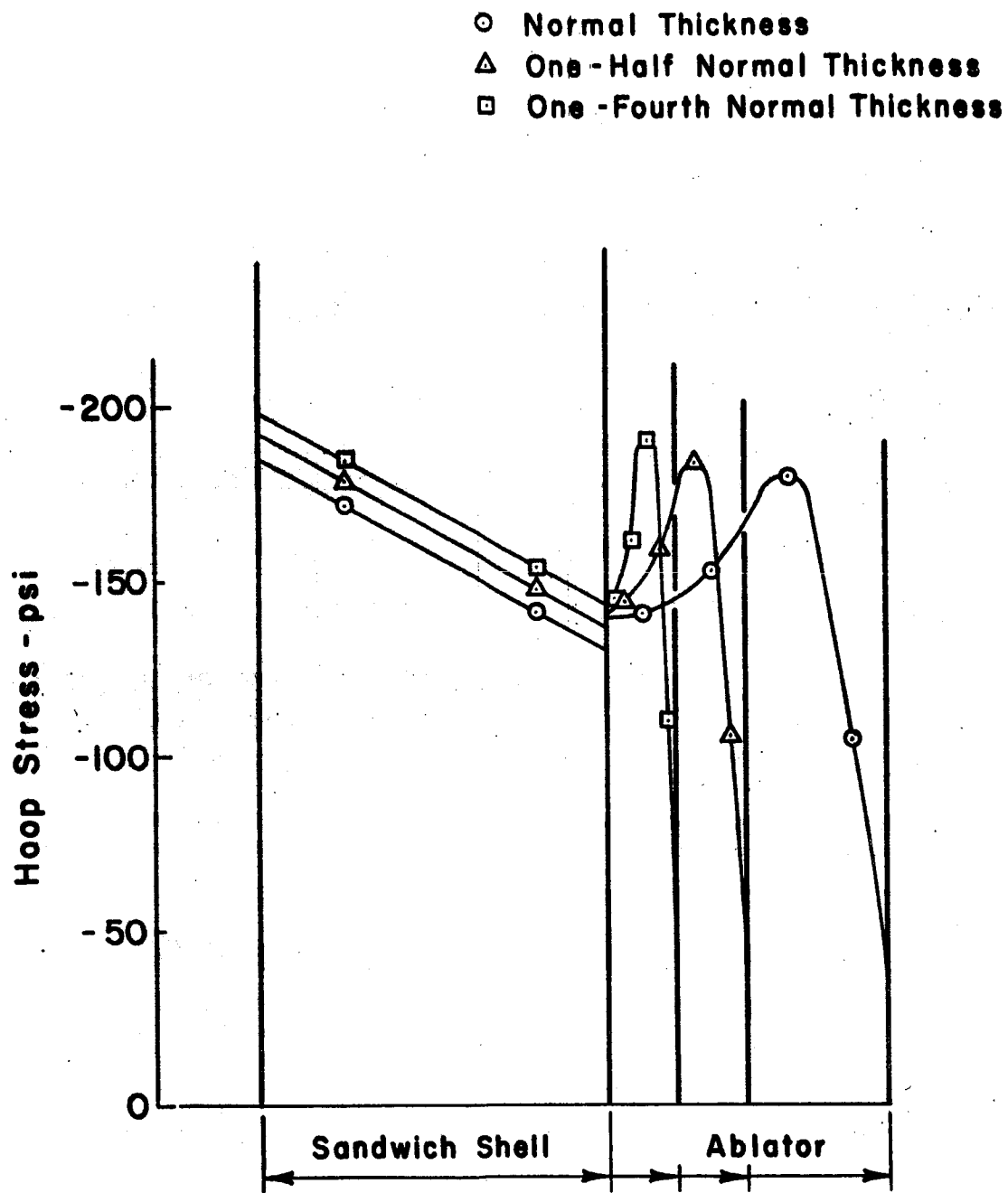


Fig. 3.3 Effect of Thickness of Ablator on Stress Distribution

the maximum stresses in the ablator were in good agreement. This example illustrates that the thickness of the ablator is not an important structural parameter at the temperature of re-entry.

3. Effect of Boundary Conditions on the Behavior of the Heat Shield

The support condition which is imposed on the heat shield is an extremely important parameter. Figure 3.4 illustrates the deflected position of the bond line for two different support conditions. The resulting stresses differ significantly. Therefore, it is important that the boundary condition which is imposed on the finite element system be a realistic approximation of the physical support condition which exists in the actual heat shield.

F. DISCUSSION

The automated computer program presented in this section reduces the analysis of an arbitrary heat shield subjected to axisymmetric thermal or mechanical loads to a simple procedure. The program automatically generates the finite element grid, evaluates temperature-dependent material properties, solves the equilibrium equations for the grid point displacements and calculates stresses within elements, sandwich shell face plates and at the bond layer. Arbitrary boundary conditions can be imposed since any of the grid points may be restrained in either the R or Z directions.

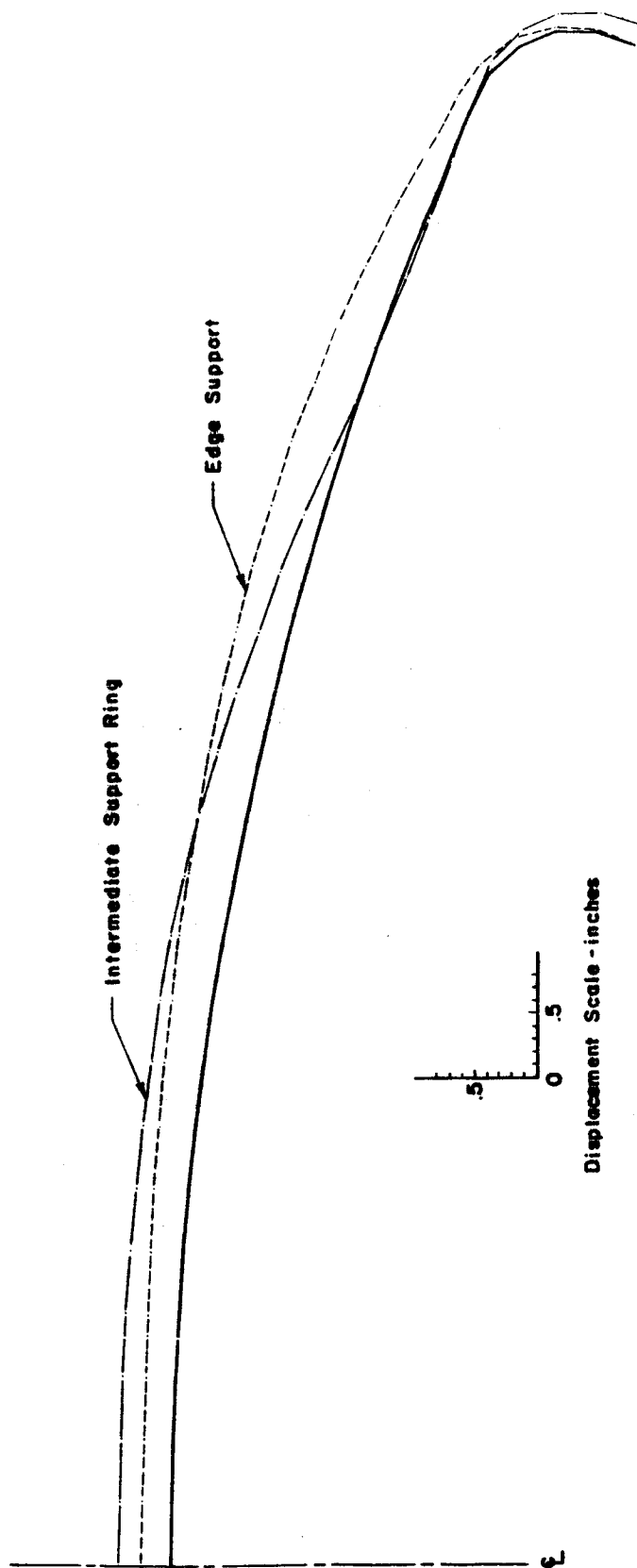


Fig. 3.4 Effect of Boundary Conditions on Deflected Shape (Bond Line)

PART IV: AUTOMATED PROGRAM FOR AXISYMMETRIC HEAT SHIELDS
SUBJECTED TO NON-AXISYMMETRIC LOADS

A. INTRODUCTION

In general, the heat shield of a manned spacecraft is composed of a constant thickness sandwich shell and an ablator which varies in thickness in both the meridional and circumferential directions. The temperature distribution experienced by the heat shield is also non-axisymmetric. Because the ablator, at high temperatures, is not a major structural element, contributing to the overall behavior of the heat shield, an approximation of its properties in the circumferential direction is justified. The approximation, that it is axisymmetric, reduces the stress analysis of a non-axisymmetric heat shield to the stress analysis of an axisymmetric structure subjected to non-axisymmetric thermal loads. This involves the expansion of the temperature distribution and the final displacements of the system in Fourier series. By making use of the orthogonality properties of the harmonic functions the three-dimensional analysis is divided into a series of uncoupled two-dimensional analyses.

B. THEORY FOR THE ANALYSIS OF AN AXISYMMETRIC BODY SUBJECTED
TO NON-AXISYMMETRIC LOADS

A theory is presented for the analysis of solids of revolution subjected to non-axisymmetric loads which are symmetric about a plane

containing the axis of revolution. Figure 4.1, a view of a plane perpendicular to the axis of revolution, shows the trace of the plane of symmetry. Anisotropic material properties, which are constant along any circumferential line, are included in this formulation.

The structure is idealized as a series of rings with triangular cross-sections; the rings are interconnected at their nodal circles, i.e., at the circles containing the vertices of the triangles, (Figure 4.2). Loads acting on the structure are replaced by statically equivalent concentrated forces acting along the nodal circles.

1. Strain-Displacement Relationship

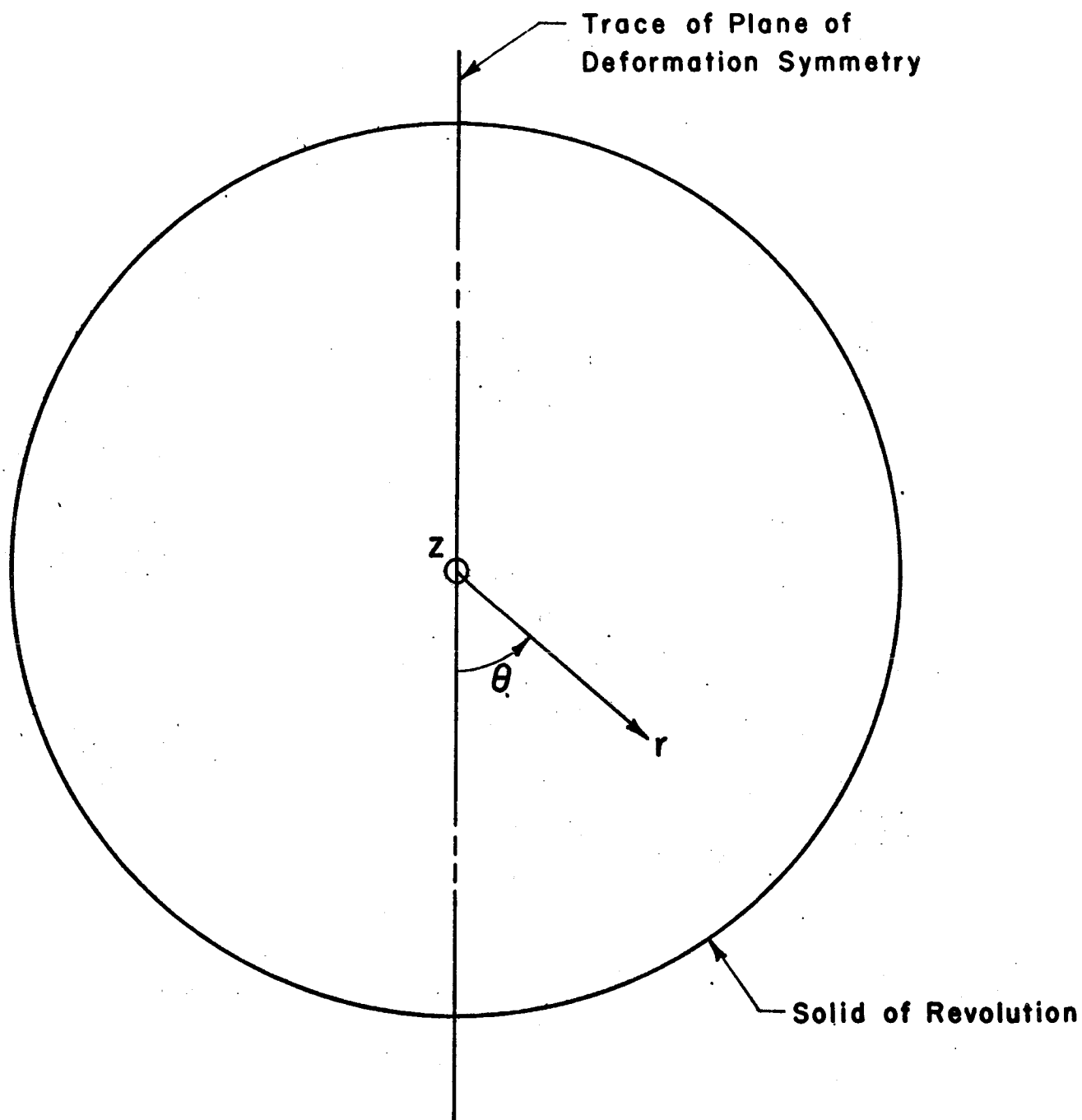
By noting the axisymmetry of the geometry and the material properties of the body and the plane of symmetry for deformations, the displacements in r , θ , z coordinates may be written in the following form:

$$u_r = \sum u_{rn}(r,z) \cos n\theta \quad (4.1a)$$

$$u_\theta = \sum u_{zn}(r,z) \sin n\theta \quad (4.1b)$$

$$u_z = \sum u_{\theta n}(r,z) \cos n\theta \quad (4.1c)$$

Within each ring element the r and z variation of the Fourier coefficients of the displacements are assumed to be linear, i.e.,



**Fig. 4.1 Cylindrical Coordinate System
Embedded in a Solid of Revolution**

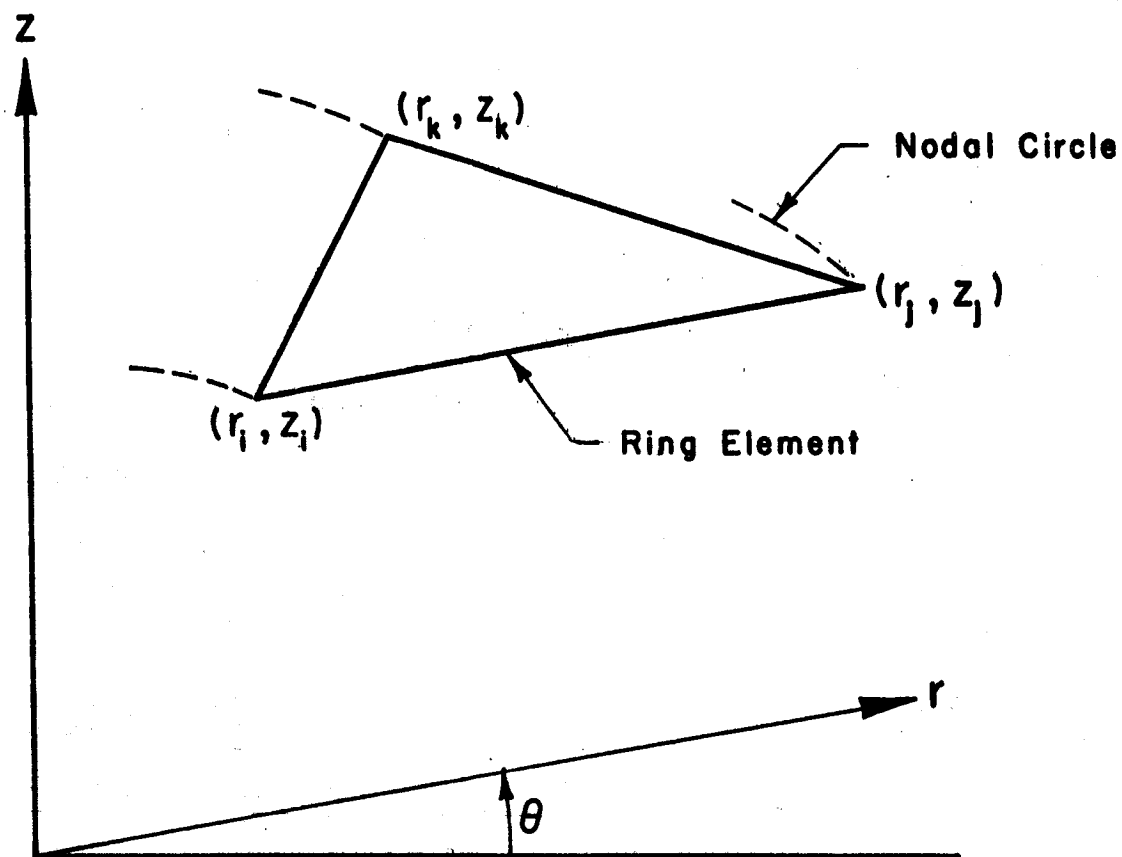


Fig. 4.2 Cross Section of a Ring Element

$$u_{rn} \approx k_{1n} + k_{2n} r + k_{3n} z \quad (4.2a)$$

$$u_{\theta n} \approx k_{4n} + k_{5n} r + k_{6n} z \quad (4.2b)$$

$$u_{zn} \approx k_{7n} + k_{8n} r + k_{9n} z \quad (4.2c)$$

Now expressing the constants k_{mn} in terms of the corner values of the Fourier coefficients of the displacements, i.e., in terms of

$$\begin{matrix} u_{rn}^i, u_{\theta n}^i, u_{zn}^i, u_{rn}^j, u_{\theta n}^j, u_{zn}^j, u_{rn}^k, u_{\theta n}^k \text{ and } u_{zn}^k \end{matrix}$$

$$\begin{bmatrix} k_{1n} & k_{4n} & k_{7n} \\ k_{2n} & k_{5n} & k_{8n} \\ k_{3n} & k_{6n} & k_{9n} \end{bmatrix} = [T] \begin{bmatrix} u_{rn}^i & u_{\theta n}^i & u_{zn}^i \\ u_{rn}^j & u_{\theta n}^j & u_{zn}^j \\ u_{rn}^k & u_{\theta n}^k & u_{zn}^k \end{bmatrix} \quad (4.3a)$$

with

$$[T] = \frac{1}{D} \begin{bmatrix} r_j z_k - z_j r_k & r_k z_i - r_i z_k & r_i z_j - r_j z_i \\ z_j - z_k & z_k - z_i & z_i - z_j \\ r_k - r_j & r_i - r_k & r_j - r_i \end{bmatrix} \quad (4.3b)$$

$$\text{and } D = r_j(z_k - z_i) + r_i(z_j - z_k) + r_k(z_i - z_j) \quad (4.3c)$$

Combining Equation (4.1) with the strain-displacement relationships, the following expressions for the strains are found:

$$\epsilon_r = \frac{\partial u_r}{\partial r} = \sum \epsilon_{rn} \cos n\theta \quad (4.4a)$$

$$\epsilon_{\theta} = \frac{1}{r} \frac{\partial u_{\theta}}{\partial \theta} + \frac{u_r}{r} = \sum \epsilon_{\theta n} \cos n\theta \quad (4.4b)$$

$$\epsilon_z = \frac{\partial u_z}{\partial z} = \sum \epsilon_{zn} \cos n\theta \quad (4.4c)$$

$$\gamma_{r\theta} = \left(\frac{1}{r} \frac{\partial u_r}{\partial \theta} + \frac{\partial u_{\theta}}{\partial r} - \frac{u_{\theta}}{r} \right) = \sum \gamma_{r\theta n} \sin n\theta \quad (4.4d)$$

$$\gamma_{rz} = \left(\frac{\partial u_z}{\partial r} + \frac{\partial u_r}{\partial z} \right) = \sum \gamma_{rzn} \cos n\theta \quad (4.4e)$$

$$\gamma_{\theta z} = \left(\frac{\partial u_{\theta}}{\partial z} + \frac{1}{r} \frac{\partial u_z}{\partial \theta} \right) = \sum \gamma_{\theta zn} \sin n\theta \quad (4.4f)$$

where

$$\epsilon_{rn} = \frac{\partial u_{rn}}{\partial r} \quad (4.5a)$$

$$\epsilon_{\theta n} = \frac{1}{r} (n u_{\theta n} + u_r) \quad (4.5b)$$

$$\epsilon_{zn} = \frac{\partial u_{zn}}{\partial z} \quad (4.5c)$$

$$\gamma_{r\theta n} = \frac{\partial u_{\theta n}}{\partial r} - \frac{u_{\theta n}}{r} - \frac{n u_{rn}}{r} \quad (4.5d)$$

$$\gamma_{rzn} = \frac{\partial u_{zn}}{\partial r} + \frac{\partial u_{rn}}{\partial z} \quad (4.5e)$$

$$\gamma_{\theta zn} = \frac{\partial u_{\theta n}}{\partial z} - n \frac{u_{zn}}{r} \quad (4.5f)$$

Within a given ring the approximate values of strain for the harmonic n are calculated by combining Equations (4.2), (4.3) and (4.5). The hoop strain is assumed to be constant within the ring and $\frac{u_{rn}}{r}$ is approximated by

$$\frac{u_{rn}^i}{3\bar{r}} + \frac{u_{rn}^j}{3\bar{r}} + \frac{u_{rn}^k}{3\bar{r}}, \text{ with } \bar{r} = \frac{1}{3} (r_i + r_j + r_k)$$

Thus, for the harmonic n the six components of strain within the element are given in terms of the nine corner displacements by the following matrix equation:

$$[\epsilon_n] = [G_n] [u_n] \quad (4.6a)$$

The strain-displacement transformation matrix (for convenience it is written in its transposed form) is defined on the next page, Eq. (4.6b).

2. Stress-Strain Relationship

The stress-strain relationship for the harmonic n is written in the following symbolic form:

$$[\sigma_n] = [C] [\epsilon_n] + [\tau_n] \quad (4.7a)$$

For an isotropic material this becomes

$$\begin{bmatrix} \sigma_{rn} \\ \sigma_{\theta n} \\ \sigma_{zn} \\ \sigma_{r\theta n} \\ \sigma_{rzn} \\ \sigma_{\theta zn} \end{bmatrix} = \begin{bmatrix} \bar{\alpha} & \bar{\beta} & \bar{\beta} & 0 & 0 & 0 \\ \bar{\beta} & \bar{\alpha} & \bar{\beta} & 0 & 0 & 0 \\ \bar{\beta} & \bar{\beta} & \bar{\alpha} & 0 & 0 & 0 \\ 0 & 0 & 0 & \mu & 0 & 0 \\ 0 & 0 & 0 & 0 & \mu & 0 \\ 0 & 0 & 0 & 0 & 0 & \mu \end{bmatrix} \begin{bmatrix} \epsilon_{rn} \\ \epsilon_{\theta n} \\ \epsilon_{zn} \\ \epsilon_{r\theta n} \\ \epsilon_{rzn} \\ \epsilon_{\theta zn} \end{bmatrix} + \begin{bmatrix} \tau_n \\ \tau_n \\ \tau_n \\ 0 \\ 0 \\ 0 \end{bmatrix} \quad (4.7b)$$

$$\begin{aligned}
 [G_n]^T = & \begin{bmatrix}
 \frac{z_j - z_k}{D} & \frac{1}{3\bar{r}} & 0 & -\frac{n}{3\bar{r}} & \frac{r_k - r_j}{D} & 0 \\
 \frac{z_k - z_i}{D} & \frac{1}{3\bar{r}} & 0 & -\frac{n}{3\bar{r}} & \frac{r_i - r_k}{D} & 0 \\
 \frac{z_i - z_j}{D} & \frac{1}{3\bar{r}} & 0 & -\frac{n}{3\bar{r}} & \frac{r_j - r_i}{D} & 0 \\
 0 & \frac{1}{3\bar{r}} & 0 & (\frac{z_j - z_k}{D} - \frac{1}{3\bar{r}}) & 0 & \frac{r_k - r_j}{D} \\
 0 & \frac{n}{3\bar{r}} & 0 & (\frac{z_j - z_k}{D} - \frac{1}{3\bar{r}}) & 0 & \frac{r_k - r_j}{D} \\
 0 & \frac{n}{3\bar{r}} & 0 & (\frac{z_k - z_i}{D} - \frac{1}{3\bar{r}}) & 0 & \frac{r_i - r_k}{D} \\
 0 & 0 & \frac{r_k - r_j}{D} & 0 & \frac{z_j - z_k}{D} & -\frac{n}{3\bar{r}} \\
 0 & 0 & \frac{r_i - r_k}{D} & 0 & \frac{z_k - z_i}{D} & -\frac{n}{3\bar{r}} \\
 0 & 0 & \frac{r_j - r_i}{D} & 0 & \frac{z_i - z_j}{D} & -\frac{n}{3\bar{r}}
 \end{bmatrix} \quad (4.6b)
 \end{aligned}$$

where

$$\bar{\alpha} = \frac{(1-\nu) E}{(1+\nu)(1-2\nu)} \quad (4.8a)$$

$$\bar{\beta} = \frac{\nu E}{(1+\nu)(1-2\nu)} \quad (4.8b)$$

$$\mu = \frac{E}{2(1+\nu)} \quad (4.8c)$$

$$\tau_n = - \frac{E\alpha}{(1-2\nu)} T_n \quad (4.8d)$$

T_n is the Fourier coefficient for the expansion of the average temperature change within the ring

$$T = \sum T_n \cos n\theta \quad (4.9)$$

3. Equilibrium Equation for Harmonic n

By recognizing the orthogonality of the harmonic functions the same procedure which was used in Part I may be used to develop the equilibrium equations for an element subjected to harmonic loading.

Therefore, Equation (1.15) is rewritten as

$$[S_n] = [k_n] [u_n] + [L_n] \quad (4.10)$$

$$\text{where} \quad [k_n] = \int [G_n]^T [C] [G_n] dV \quad (4.11)$$

$$[L_n] = \int [G_n]^T [\tau_n] dV \quad (4.12)$$

Within a ring the matrices $[G_n]$ and $[C]$ are not a function of space; therefore, Equations (4.11) and (4.12) reduce to

$$[k_n] = V \cdot [G_n]^T [C] [G_n] \quad (4.13)$$

$$[L_n] = V \cdot [G_n]^T [\tau_n] \quad (4.14)$$

where the volume V is given by Equation (2.10). Equilibrium of the over-all structure requires that the sum of the nodal circle forces for all rings with a common nodal circle must equal the applied nodal force.

This results in an equation of the following form for each harmonic:

$$[K_n] [r_n] = [R_n] \quad (4.15)$$

where the displacement vector $[r_n]$ contains all the displacement amplitudes u_{rn} , $u_{\theta n}$ and u_{zn} for all nodal circles in the system.

The equilibrium of the face plates is incorporated by a similar procedure. Appendix C gives the details of this development.

4. Determination of Displacements and Stresses

The number of harmonics required to represent the three-dimensional temperature distribution indicates the number of two-dimensional problems which must be solved. For each harmonic Equation (4.15) must be solved for the unknown displacement amplitudes. The corresponding strain amplitudes are calculated for each finite element by Equation (4.6) and then stress amplitudes are found by the application of Equation (4.7). The final displacements of the system for any angle are calculated from Equation (4.1). The final stresses are determined from the stress amplitudes by the following equations:

$$\sigma_r(r, z, \theta) = \sum \sigma_{rn} \cos n\theta \quad (4.16a)$$

$$\sigma_z(r, z, \theta) = \sum \sigma_{zn} \cos n\theta \quad (4.16b)$$

$$\sigma_\theta(r, z, \theta) = \sum \sigma_{\theta n} \sin n\theta \quad (4.16c)$$

$$\sigma_{rz}(r, z, \theta) = \sum \sigma_{rzn} \cos n\theta \quad (4.16d)$$

$$\sigma_{r\theta}(r, z, \theta) = \sum \sigma_{r\theta n} \sin n\theta \quad (4.16e)$$

$$\sigma_{\theta z}(r, z, \theta) = \sum \sigma_{\theta zn} \sin n\theta \quad (4.16f)$$

C. COMPUTER PROGRAM

The use of the non-axisymmetric heat shield program is similar to the axisymmetric program (Part III). The only additional input required is the three-dimensional temperature distribution. The computer program automatically develops the necessary Fourier coefficients for the temperature distribution and sums the series of two-dimensional analyses to produce the final displacements and stresses in the system.

1. Input Information

The following sequence of punched cards numerically defines the heat shield to be analyzed:

a. FIRST CARD - (72H)

Columns 1 to 72 of this card contains information to be printed with results

b. SECOND CARD - (615, 2F10.2)

Columns 1 - 5 Number of points along meridian
of shield - NMAX
6 - 10 Number of points through thickness -
MMAX
11 - 15 Location of bond line - MB
16 - 20 Number of material property cards - NP
21 - 25 Number of harmonic to be used in
analysis - NL
26 - 30 Number of boundary condition cards - NB
31 - 40 Surface temperature of ablator
41 - 50 Temperature of zero stress

c. THIRD CARD - Properties of Sandwich Core (4F10.2)

Columns 1 - 10 Modulus of elasticity
11 - 20 Poisson's ratio
21 - 30 Coefficient of thermal expansion
31 - 40 Thickness of core

d. FOURTH CARD - Properties of Sandwich Face Plates (4F10.2)

Columns 1 - 10 Modulus of elasticity
11 - 20 Poisson's ratio
21 - 30 Coefficient of thermal expansion
31 - 40 Thickness of single face plate

e. GEOMETRY CARDS - (4F10.2)

One card per point along shield in order from axis of
symmetry to edge (NMAX cards).

Columns 1 - 10 R-ordinate at bond line
11 - 20 Z-ordinate at bond line
21 - 30 Temperature at bond line
31 - 40 Normal thickness of ablator

The temperature information is used by the program to determine the axisymmetric temperature-dependent material properties.

f. MATERIAL PROPERTY CARDS - (4F10.2)

One card for each temperature (NP cards)

Columns 1 - 10 Temperature
11 - 20 Modulus of elasticity of ablative material
21 - 30 Modulus of elasticity of bond material
31 - 41 Coefficient of thermal expansion for ablative and bond materials

g. THREE-DIMENSIONAL TEMPERATURE DISTRIBUTION CARDS

A table of bond line temperature values at 10 degree increments along 9 circumferential lines is punched in the following form:

1st. card - (9F8.0)

R-ordinates of 9 circumferential points on bond line

2nd card - (9F8.0)

Z-ordinates of 9 circumferential points on bond line

3rd to 21st card - (9F8.0)

One card for each 10 degree increment (0 to 180°).

Each card contains the 9 temperatures which correspond to the above R and Z-ordinates.

h. BOUNDARY CONDITION CARDS - (315)

One card per restrained nodal circle (NB cards)

Column 1 - 5 N } mesh point N, M
6 - 10 M }

11 - 15 = 1 restrained in R-direction
2 restrained in θ -direction
3 restrained in Z-direction
4 restrained in R and θ -directions
5 restrained in R and Z-directions
6 restrained in θ and Z-directions
7 restrained in R, θ and Z directions

i. PRINT ANGLE CARDS - (1F5.0)

Column 1 - 5 angle θ

For each "print angle card" a complete set of displacement and stresses are printed for angle θ .

2. Output Information

The following information is generated and printed by the computer program:

- a. Input data
- b. Least squares evaluation of the temperature-dependent material property data
- c. Coordinates and temperature of all grid points
- d. Two-dimensional Fourier temperature coefficients
- e. For each print angle
 - (1) R, Z, and θ displacement at all grid points
 - (2) Average stresses in quadrilateral rings
 - (3) Stresses in sandwich face plates

3. Timing

The computer time required by this program for the non-axisymmetric analysis of a heat shield is approximately

$$\text{time} = A + B \cdot (NMAX) \cdot (MMAX)^2 \cdot NL \text{ (seconds)}$$

For the IBM 7094 computer $A=20$ and $B=0.05$, and the time required for a 30×7 mesh with 4 harmonics is approximately 5 minutes.

4. Program Listing

A card listing of the FORTRAN II source deck for the computer program for the non-axisymmetric analysis of heat shields is given in Appendix F. The program is compiled for a maximum grid size of 30 points in the meridional direction and 8 points through the thickness. A maximum of 10 harmonics may be considered. Material properties can be specified by a maximum of 50 cards.

Standard input tape 5 and output tape 6 are used by the program. Tape 20 is used for temporary storage within the program; it may be necessary to change this tape unit to conform with local computer center policy.

D. EXAMPLES

Two analyses of axisymmetric heat shields subjected to non-axisymmetric temperature distribution were conducted to illustrate the application of the

program. In both cases, the axisymmetric finite element mesh was similar to the mesh given by Figure 3.1. For the purpose of reference the station layout along the bond line is shown in Figure 4.3. The three-dimensional bond layer temperature and ablator thickness distribution for angles $\theta = 0, 90^\circ, 180^\circ$ are plotted in Figure 4.4.

In Analysis A the axisymmetric properties of the ablator are assumed to be equal to the properties of the actual ablator at $\theta = 0$. In Analysis B the ablator properties at $\theta = 180^\circ$ are used. For both analyses the surface temperature of the ablator is 1000 F and the temperature at the bond surface is given by Figure 4.5. Station 20 (Figure 4.3) is restrained at the inside surface of the shield to simulate the effect of an intermediate support ring.

The computer output for a non-axisymmetric analysis contains displacements and stresses at many points in the heat shield; however, only the typical results are presented. For Analysis A the deflected shape of the bond line at three sections is plotted in Figure 4.6a. The non-axisymmetric behavior is significant. The displacements from Analysis B are shown in Figure 4.6b; they are essentially the same as those found by Analysis A. This again indicates that the ablator's thickness and property variations in the circumferential direction are not of major importance at these temperatures. Hoop stresses at station 15 are

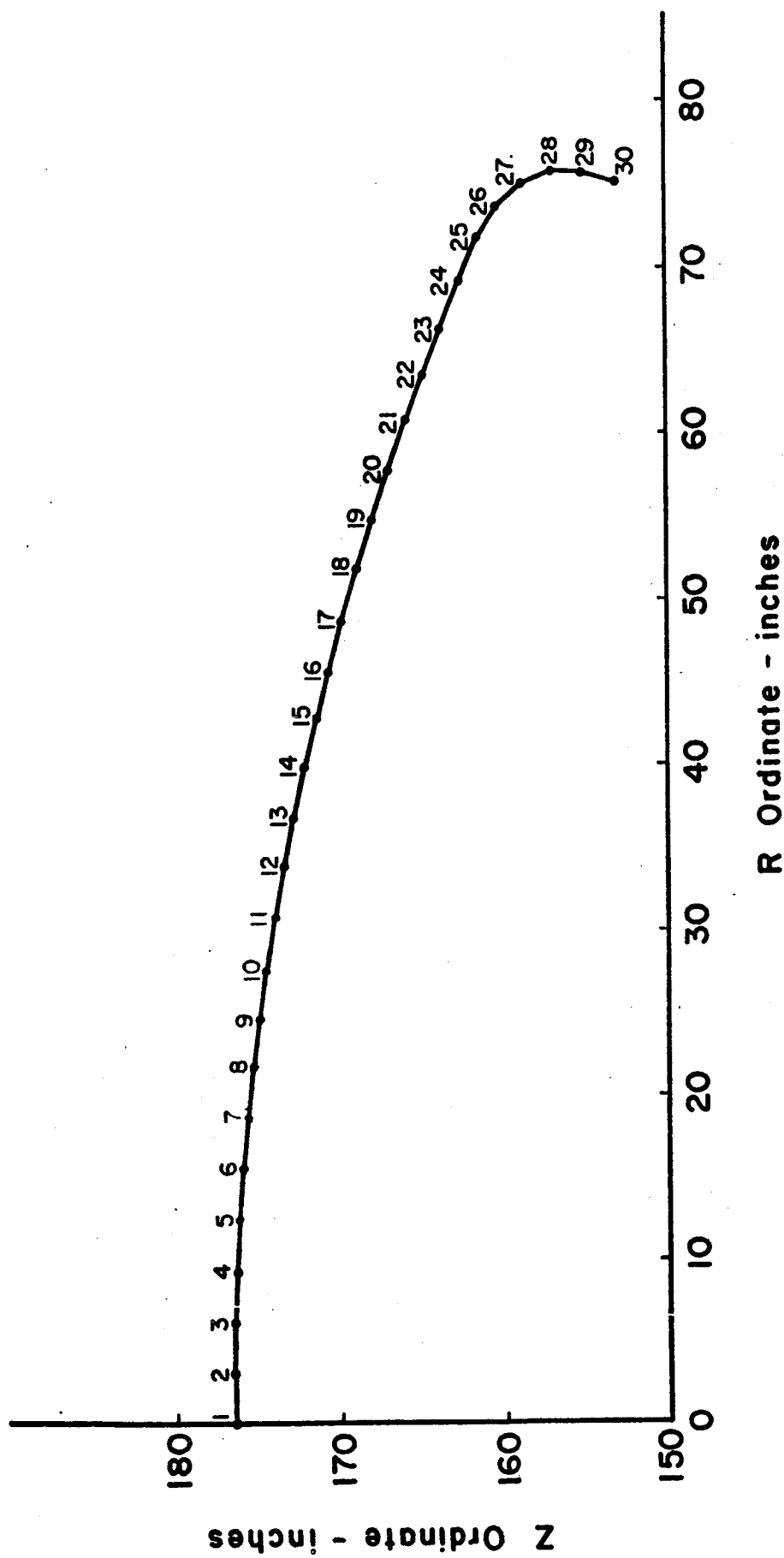


Fig. 4.3 Station along Bond Line

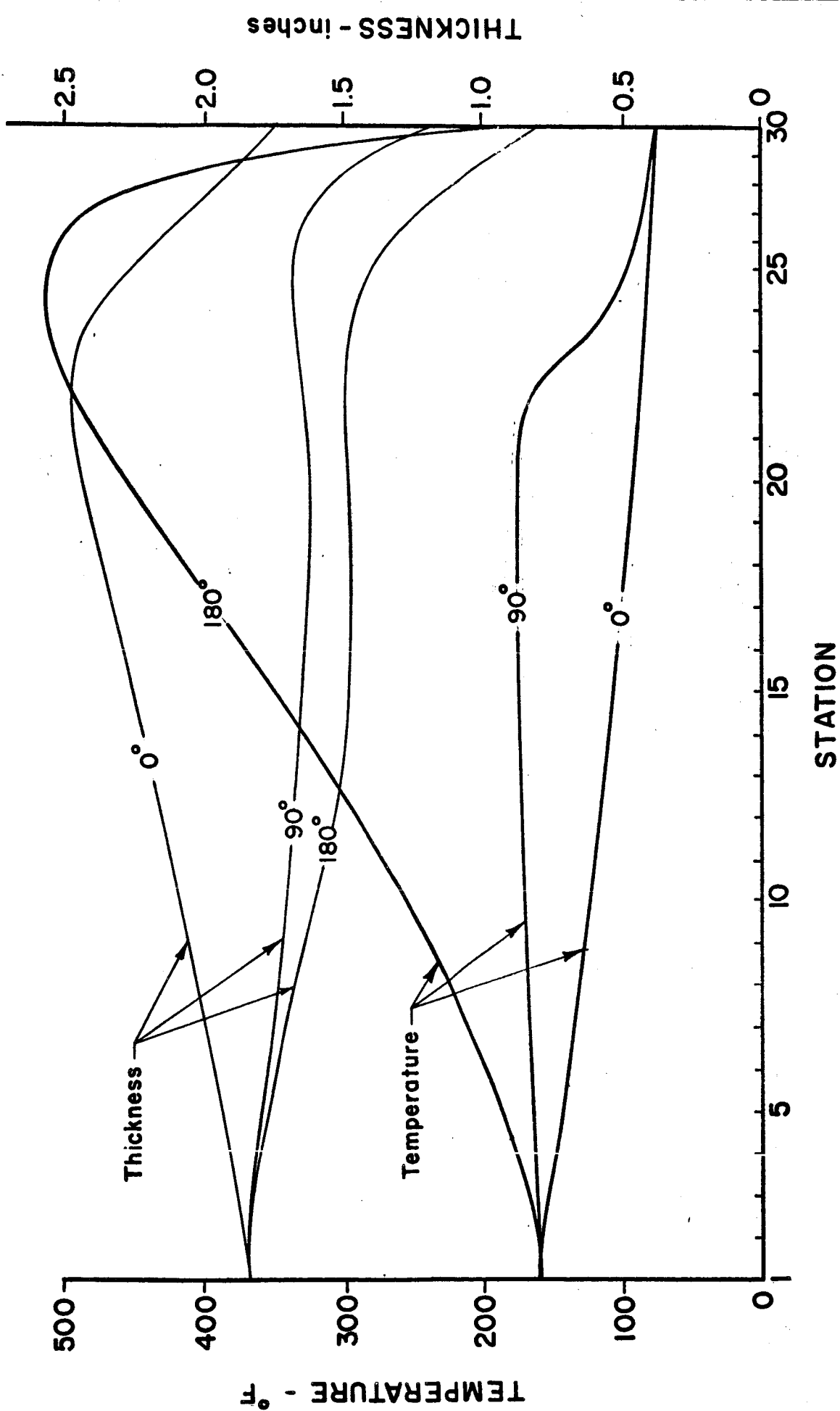


Fig. 4.4 Bond Layer Temperature and Ablator Thickness Distribution

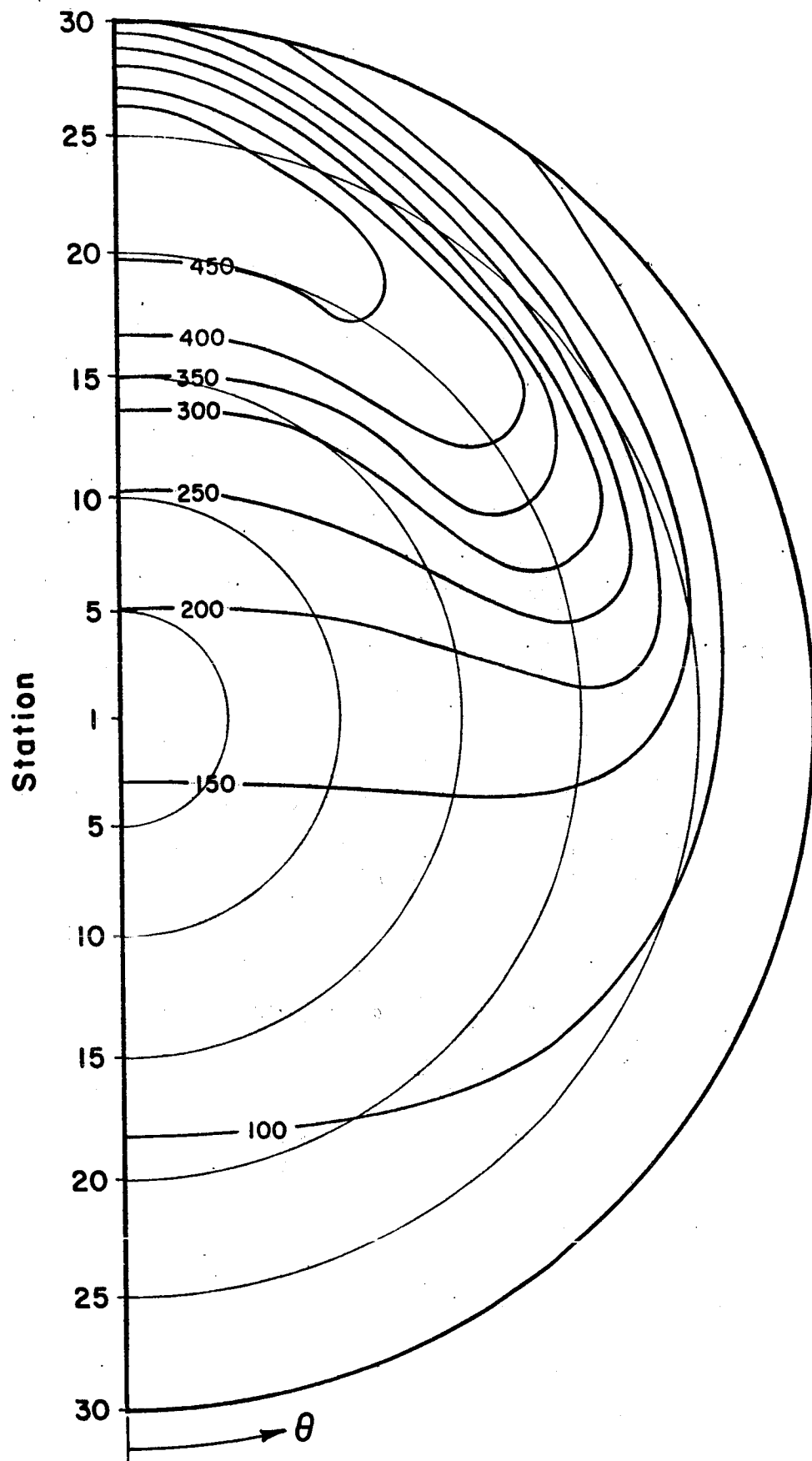


Fig. 4.5 Temperature at Bond Surface

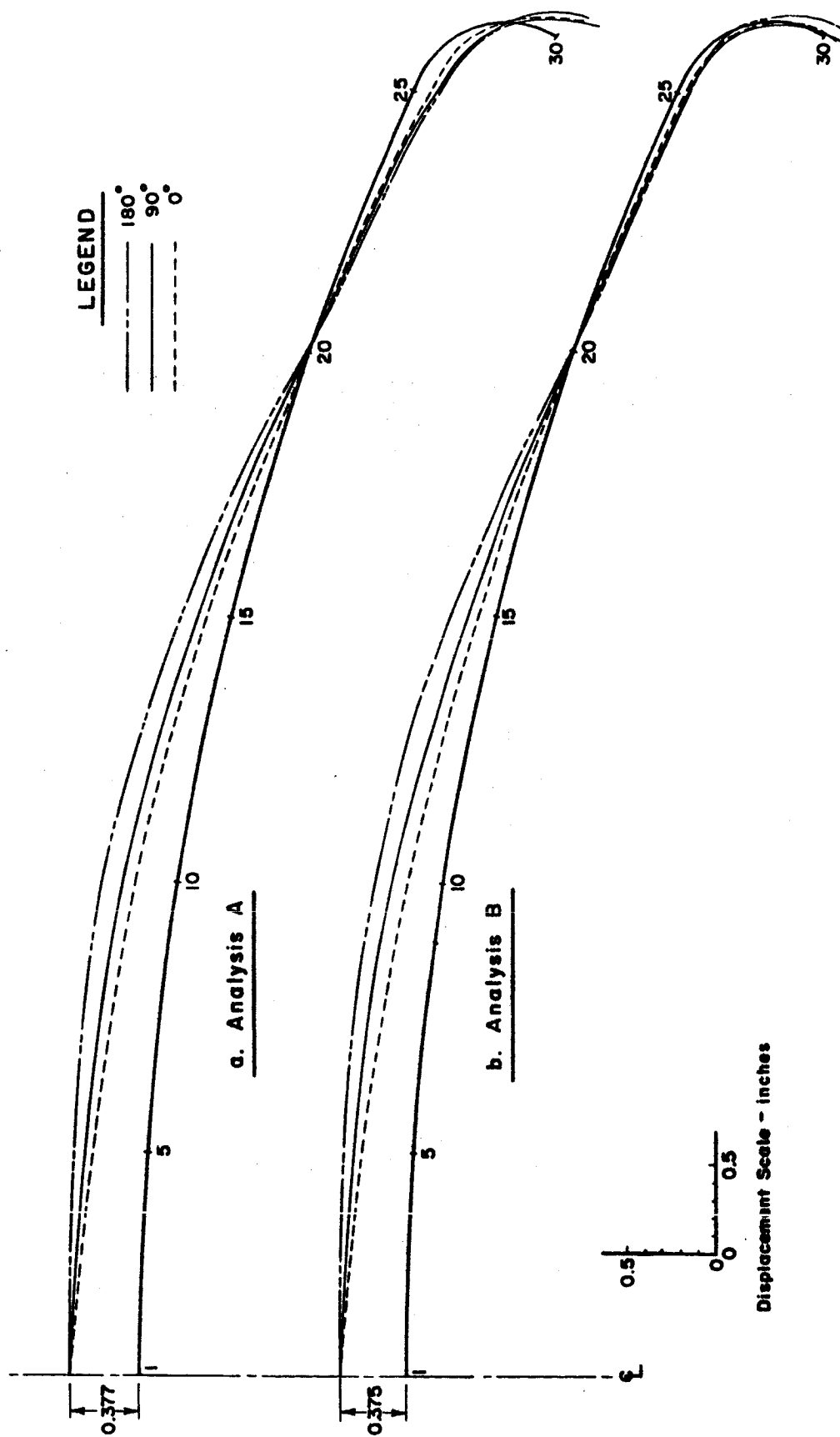


Fig. 4.6 Deflected Shape at Bond Surface

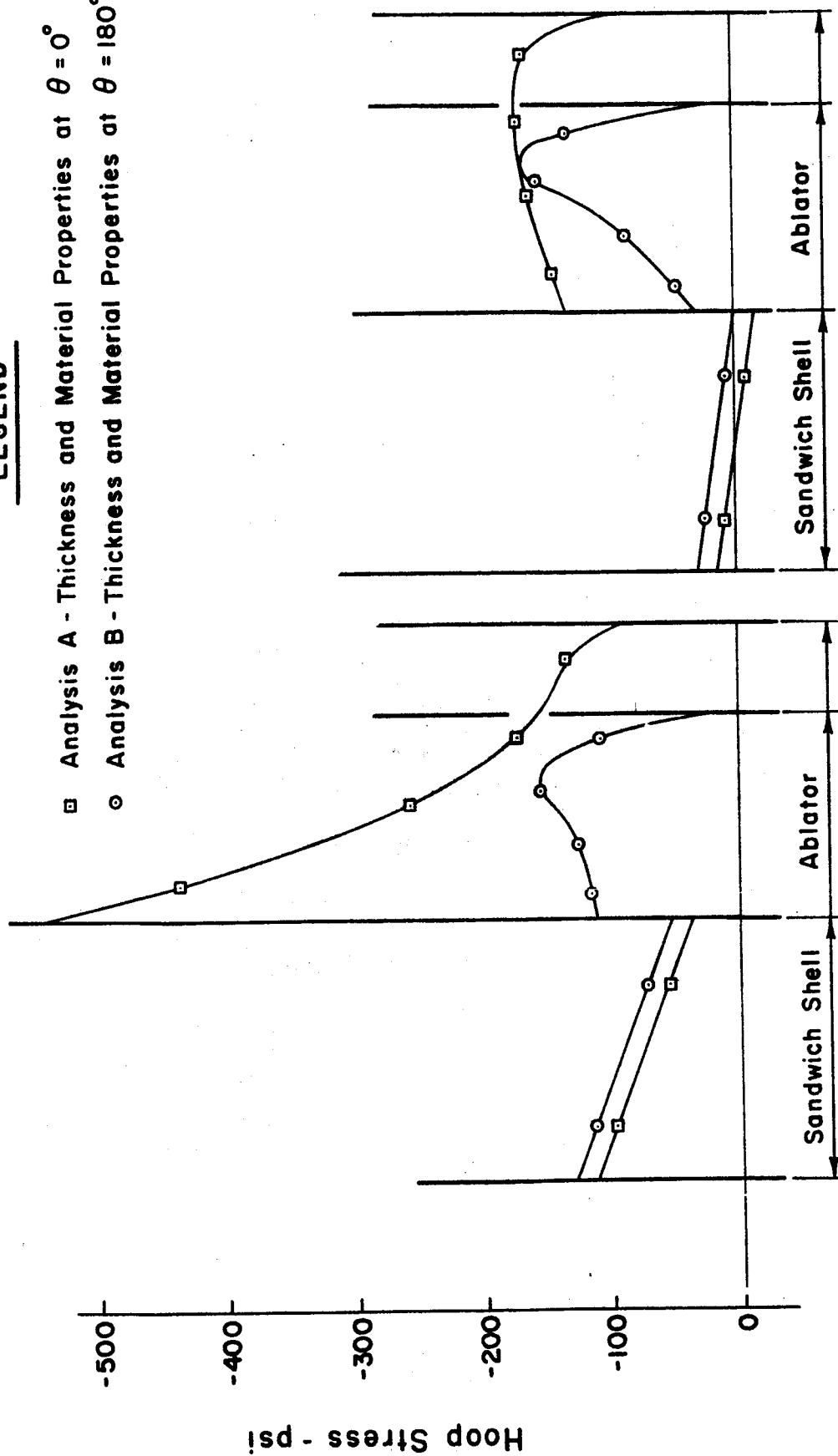
plotted in Figure 4.7 for two values of θ . In both analyses the stresses within the sandwich shell are in fair agreement since the material properties do not change at these temperatures. However, within the ablator, where the material properties are strongly temperature-dependent, the stresses differ significantly. Of course, the particular solution is most reasonable if the assumed material properties correspond with those at the position of the desired stress. Hence, for $\theta = 0$, Analysis A is considered the best approximation and similarly for $\theta = 180^\circ$, Analysis B is the most reasonable.

E. DISCUSSION

In this section a method and the resulting computer program are presented for the analysis of axisymmetric heat shields subjected to non-axisymmetric thermal loads. The program may be used to analyze an approximate solution to a non-axisymmetric heat shield if a number of solutions are obtained and then are judiciously evaluated. At the section where the assumed axisymmetric ablator properties (temperature and thickness) correspond to the local properties, the stresses will be a good approximation of the actual state of stress in the non-axisymmetric heat shield. Therefore, for each angle θ for which stresses are desired a separate structure must be evaluated.

LEGEND

- ▣ Analysis A - Thickness and Material Properties at $\theta = 0^\circ$
- Analysis B - Thickness and Material Properties at $\theta = 180^\circ$



a. Hoop Stress - Station at $\theta = 180^\circ$ b. Hoop Stress - Station at $\theta = 0^\circ$

Fig. 4.7 Typical Results of Non-Axisymmetric Heat Shield Analysis

It should be pointed out that the computer program can be extended to include non-axisymmetric pressure loading, displacement boundary conditions and the effects of elastic supports. However, this additional investigation was beyond the scope of the present effort.

F. COLD SOAK CONDITION

A necessary approximation of the method of analysis which is presented in this report is that the non-axisymmetric ablator of the heat shield is approximated by an axisymmetric ablator. Since the stiffness of the ablator is reduced at high temperatures, this approximation is justified at the temperature of re-entry. However, at low temperatures the stiffness of the ablator, as compared to the stiffness of the sub-structure, is significant.

Figure 4.8 illustrates typical stresses developed within two different heat shields when subjected to a uniform reduction in temperature (185°F to -250°F). The ablator thicknesses used in the analyses correspond to the thicknesses at sections 0° and 180° (Fig. 4.4). The resulting stresses are comparable which indicates that for an axisymmetric heat shield, the thickness of the ablator does not affect the stress distribution significantly. It is also reasonable to expect that the actual behavior of the non-axisymmetric heat shield will be bracketed by these results.

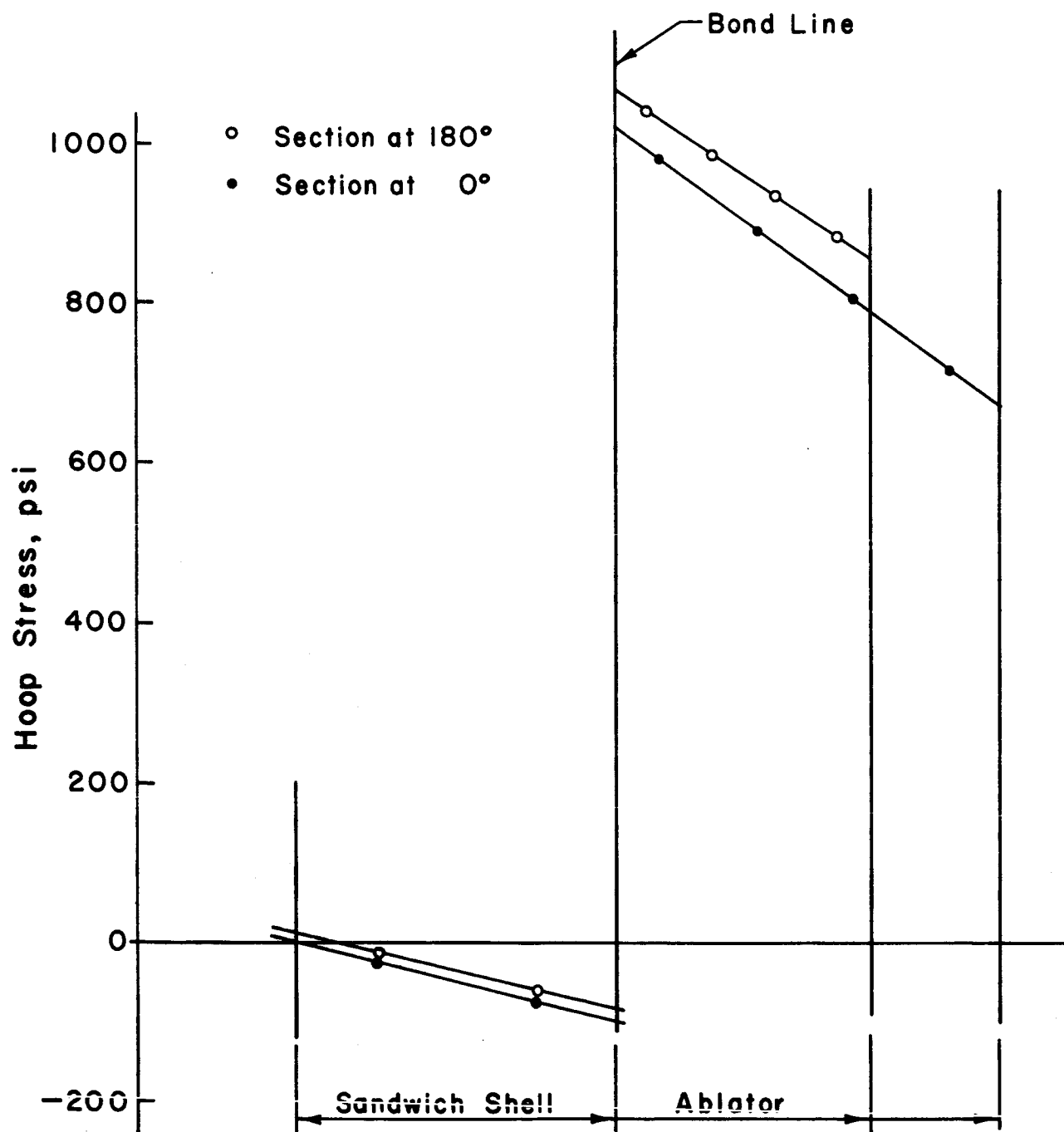


Fig. 4.8 Typical Results of Cold Soak Analysis

APPENDIX A

SOLUTION OF EQUILIBRIUM EQUATIONS

The equilibrium equations for a system of finite elements may be written in the following form:

$$A_{11} X_1 + A_{12} X_2 + A_{13} X_3 \text{ ----- } + A_{1N} X_N = B_1 \quad (A1a)$$

$$A_{21} X_1 + A_{22} X_2 + A_{23} X_3 \text{ ----- } + A_{2N} X_N = B_2 \quad (A1b)$$

$$A_{31} X_1 + A_{32} X_2 + A_{33} X_3 \text{ ----- } + A_{3N} X_N = B_3 \quad (A1c)$$

$$A_{N1} X_1 + A_{N2} X_2 + A_{N3} X_3 \text{ ----- } + A_{NN} X_N = B_N \quad (-)$$

or symbolically

$$[A] [X] = [B] \quad (A1)$$

where

$[A]$ = the stiffness matrix

$[X]$ = the unknown displacements

$[B]$ = the applied loads

Gaussian Elimination

The first step in the solution of the above set of equations is to solve Equation (A1a) for X_1 , Or

$$X_1 = B_1/A_{11} - (A_{12}/A_{11}) X_2 - (A_{13}/A_{11}) X_3 - \dots - (A_{1N}/A_{11}) X_N \quad (A2)$$

If Equation (A2) is substituted into Equations (A1b, c, ..., N) a modified set of $N-1$ equations is determined.

$$A_{22}^1 X_2 + A_{23}^1 X_3 - \dots + A_{2N}^1 X_N = B_2^1 \quad (A3a)$$

$$A_{32}^1 X_2 + A_{33}^1 X_3 - \dots + A_{3N}^1 X_N = B_3^1 \quad (A3b)$$

$$A_{N2}^1 X_2 + A_{N3}^1 X_3 - \dots + A_{NN}^1 X_N = B_N^1$$

where $A_{ij}^1 = A_{ij} - A_{i1} A_{1j}/A_{11} \quad i, j = 2, \dots, N \quad (A4a)$

$$B_i^1 = B_i - A_{i1} B_1/A_{11} \quad i = 2, \dots, N \quad (A4b)$$

A similar procedure is used to eliminate X_2 from Equation (A3), etc.

A general algorithm for the elimination of X_n may be written as

$$X_n = (B_n^{n-1}/A_{nn}^{n-1}) - \sum (A_{nj}^{n-1}/A_{nn}^{n-1}) X_j \quad j = n+1, \dots, N \quad (A5)$$

$$A_{ij}^n = A_{ij}^{n-1} - A_{in}^{n-1} (A_{nj}^{n-1}/A_{nn}^{n-1}) \quad i, j = n+1, \dots, N \quad (A6)$$

$$B_i^n = B_i^{n-1} - A_{in}^{n-1} (B_n^{n-1}/A_{nn}^{n-1}) \quad i = n+1, \dots, N \quad (A7)$$

Equations A5, A6 and A7 may be rewritten in compact form:

$$X_n = D_n - \sum C_{nj} X_j \quad j = n+1, \dots, N \quad (A8)$$

$$A_{ij}^n = A_{ij}^{n-1} - A_{in}^{n-1} C_{nj} \quad i, j = n+1, \dots, N \quad (A9)$$

$$B_i^n = B_i^{n-1} - A_{in}^{n-1} D_n \quad i = n+1, \dots, N \quad (A10)$$

where

$$D_n = B_n^{n-1}/A_{nn}^{n-1}$$

$$C_{nj} = A_{nj}^{n-1}/A_{nn}^{n-1}$$

After the above procedure is applied N-1 times the original set of equations is reduced to the following single equation

$$A_{NN}^{N-1} X_N = B_N^{N-1}$$

which is solved directly for X_N

$$X_N = B_N^{N-1} / A_{NN}^{N-1}$$

In terms of the previous notation, this is

$$X_N = D_N \quad (A11)$$

The remaining unknowns are determined in reverse order by the repeated application of Equation (A8).

Simplification for Band Matrices

For many finite element systems it is possible to place the stiffness matrix in a "band" form which results in the concentration of the elements of the stiffness matrix along the main diagonal. Therefore, the following simplifications in the general algorithm (Equations A8, A9 and A10) are possible:

$$X_n = D_n - \sum C_{nj} X_j \quad j = n+1, \dots, n+M-1 \quad (A12)$$

$$A_{ij}^n = A_{ij}^{n-1} - A_{in}^{n-1} C_{nj} \quad i, j = n+1, \dots, n+M-1 \quad (A13)$$

$$B_i^n = B_i^{n-1} - A_{in}^{n-1} D_n \quad i = n+1, \dots, n+M-1 \quad (A14)$$

where M is the band width of the matrix.

The number of numerical operations can further be reduced by recognizing that the reduced matrix at any stage of procedure is symmetric. Accordingly, Equation (A13) may be replaced by the following equation:

$$A_{ij}^n = A_{ij}^{n-1} - A_{in}^{n-1} C_{nj} \quad \begin{array}{l} i = n+1, \dots, n+M-1 \\ j = i, \dots, n+M-1 \end{array} \quad (A15)$$

since

$$A_{ji}^n = A_{ij}^n$$

The number of numerical operations required for the solution of a band matrix is proportional to NM^2 as compared to N^3 which is required for the solution of a full matrix. Also, the computer storage required by the band matrix procedure is NM as compared to N^2 required by a set of N arbitrary equations.

This technique has been used in the automated axisymmetric program for the analysis of a typical heat shield idealized by a 10 x 40 mesh of quadrilateral elements. A solution to 800 simultaneous equations was necessary, which required less than two minutes of computing time on the IBM 7094.

APPENDIX B

MATRIX FORMULATION OF THE LEAST SQUARE CURVE-FIT PROCEDURE

Consider the problem of selecting the "best" polynomial of the form $y = C_1 + C_2X + C_3X^2 + C_4X^3, \dots, C_NX^{N-1}$ to represent the following set of data points:

$$X_1, Y_1; X_2, Y_2; \text{-----}; X_M, Y_M$$

If the above polynomial is evaluated at points X_1 to X_M , M equations of the following form are found:

$$C_1 + C_2 X_m + C_3 X_m^2 + \text{-----} C_N X_m^{N-1} = Y_m \quad m = 1, \dots, M$$

or in matrix form

$$\begin{bmatrix} 1 & X_1 & X_1^2 & \text{-----} & X_1^{N-1} \\ 1 & X_2 & X_2^2 & \text{-----} & X_2^{N-1} \\ \text{-----} & & & & \\ 1 & X_m & X_m^2 & \text{-----} & X_m^{N-1} \\ \text{-----} & & & & \\ 1 & X_M & X_M^2 & \text{-----} & X_M^{N-1} \end{bmatrix} \begin{bmatrix} C_1 \\ C_2 \\ - \\ - \\ C_N \end{bmatrix} = \begin{bmatrix} Y_1 \\ Y_2 \\ \text{---} \\ Y_m \\ \text{---} \\ Y_M \end{bmatrix} \quad (\text{Bla})$$

or symbolically

$$[A][C] = [Y] \quad (B1b)$$

where

$$[A] = \text{a } M \times N \text{ matrix}$$

$$[C] = \text{a } N \times 1 \text{ matrix}$$

$$[Y] = \text{a } M \times 1 \text{ matrix}$$

If Equation (B1) is premultiplied by $[A]^T$, a set of N linear equations in N unknowns is created. Consequently,

$$[B][C] = [D] \quad (B2)$$

where

$$[B] = [A]^T [A] = \text{a } N \times N \text{ matrix}$$

$$[D] = [A]^T [Y] = \text{a } N \times 1 \text{ matrix}$$

Equation (B2) can now be solved directly for the unknown coefficients $[C]$.

This procedure is numerically equivalent to the standard least square procedure. However, it is presented here in a form which is readily programmed for the digital computer. The technique is not restricted to polynomials.

Figure B1 illustrates the application of the method in the evaluation of the elastic modulus for temperature dependent materials. A fourth order polynomial was used.

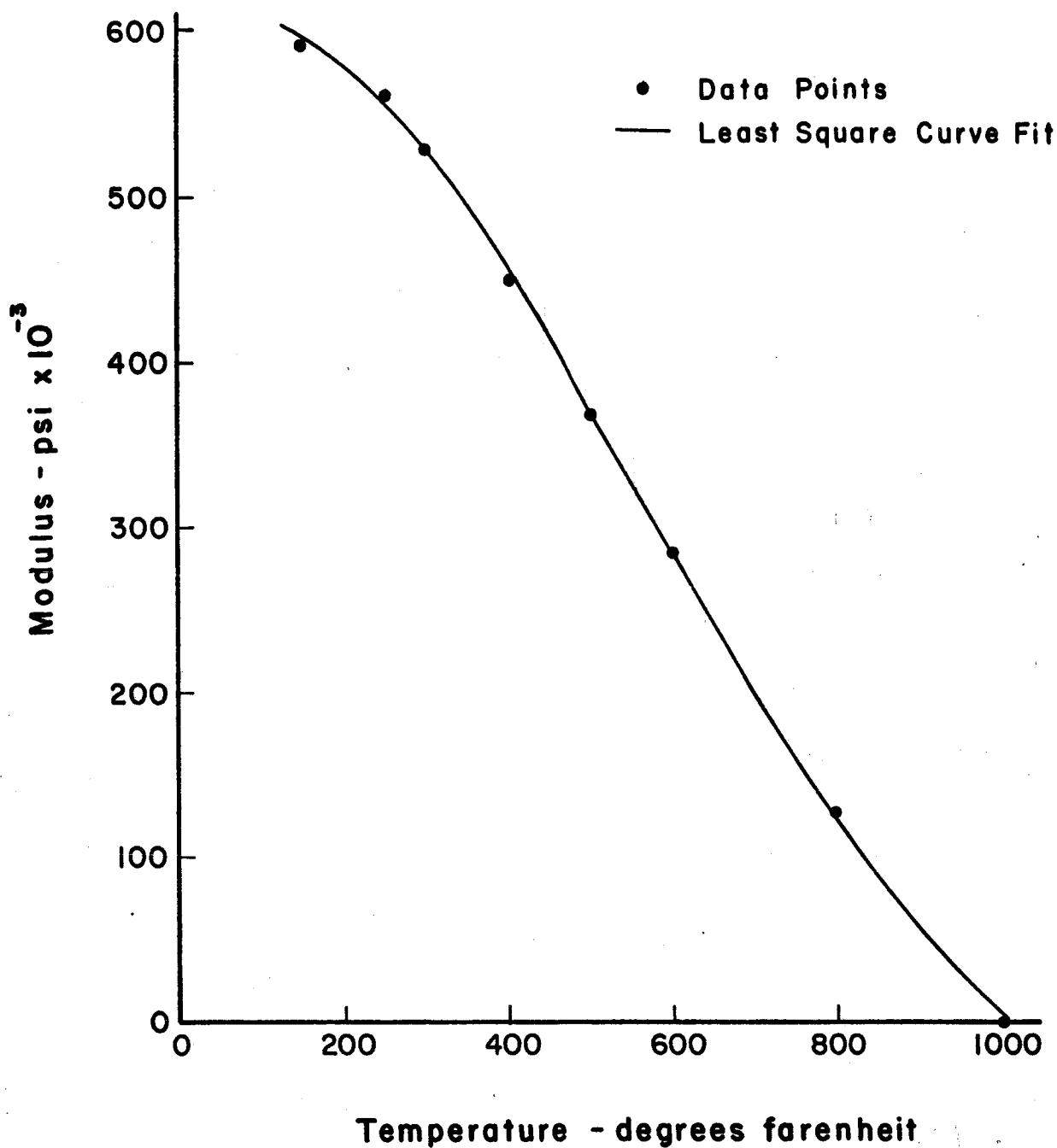


Fig. B1 Example of Least Square Curve Fit

APPENDIX C - MATHEMATICAL MODEL OF SANDWICH SHELL

The sandwich shell substructure is composed of a honeycomb core material and two steel face plates. The orthotropic core material is readily represented by solid triangular rings as indicated in Part IV of this report. However, for the description of the behavior of the face plates, a shell type element is used. The appropriate theory is given in this appendix.

It is assumed that the face plates are idealized by series of truncated cone elements which are connected at mesh points of the finite element system. The cross section of a typical truncated cone element is shown in Figure C1.

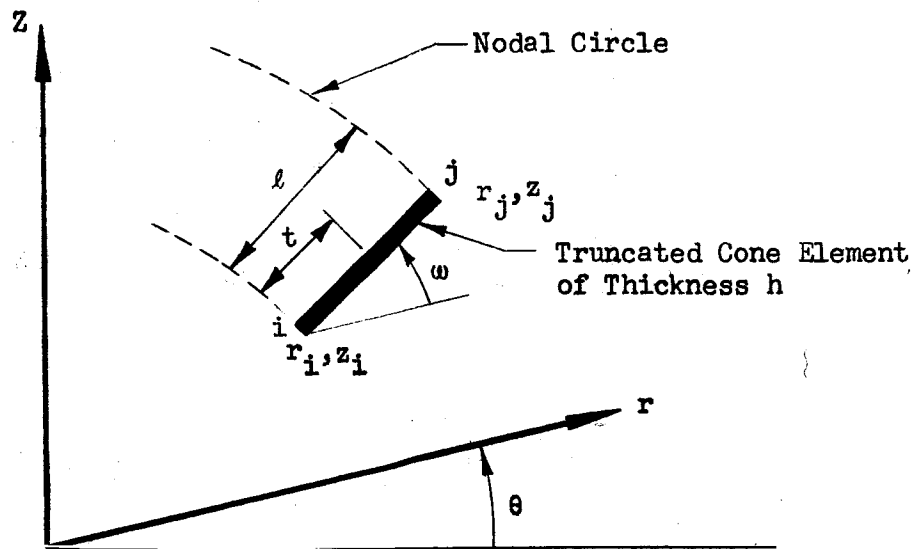


FIGURE C1 - CROSS SECTION OF TRUNCATED CONE ELEMENT

The displacements of the system in the r, θ, z coordinate system are written in the following form:

$$u_r = \sum u_{rn}(r, z) \cos n\theta$$

$$u_z = \sum u_{zn}(r, z) \cos n\theta$$

$$u_\theta = \sum u_{\theta n}(r, z) \sin n\theta$$

where $u_{rn}(r, z)$, $u_{zn}(r, z)$ and $u_{\theta n}(r, z)$ are the two-dimensional displacement functions (Fourier coefficients) associated with the harmonic n .

Within each truncated cone element, the two-dimensional displacement functions are assumed to vary linearly. The displacement at some point t within the element is given in terms of the nodal circle displacements by

$$u_r(t, \theta) = \sum \left[\frac{\ell-t}{\ell} u_{rn}^i + \frac{t}{\ell} u_{rn}^j \right] \cos n\theta \quad (C1)$$

$$u_z(t, \theta) = \sum \left[\frac{\ell-t}{\ell} u_{zn}^i + \frac{t}{\ell} u_{zn}^j \right] \cos n\theta \quad (C2)$$

$$u_\theta(t, \theta) = \sum \left[\frac{\ell-t}{\ell} u_{\theta n}^i + \frac{t}{\ell} u_{\theta n}^j \right] \sin n\theta \quad (C3)$$

Accordingly, the displacement in the t -direction is

$$u_t(t, \theta) = u_r(t, \theta) \cos \omega + u_z(t, \theta) \sin \omega \quad (C4)$$

$$\text{where} \quad \cos \omega = \frac{a}{l}$$

$$\sin \omega = \frac{b}{l}$$

The inplane strains within the truncated cone element are

$$\epsilon_t = \frac{\partial u_t}{\partial t} = \sum \epsilon_{tn} \cos n\theta \quad (C5)$$

$$\epsilon_\theta = \frac{1}{r} \frac{\partial u_\theta}{\partial \theta} + \frac{u_r}{r} = \sum \epsilon_{\theta n} \cos n\theta \quad (C6)$$

$$\gamma_{\theta t} = \frac{1}{r} \frac{\partial u_t}{\partial \theta} + \frac{\partial u_\theta}{\partial t} = \sum \gamma_{\theta tn} \sin n\theta \quad (C7)$$

By combining Equations (C1) through (C7), the inplane strains for harmonic n are expressed in terms of nodal circle displacements by the following matrix equations:

$$\begin{bmatrix} \epsilon_{tn} \\ \epsilon_{\theta n} \\ \gamma_{\theta tn} \end{bmatrix} = \begin{bmatrix} -\frac{a}{l^2} & -\frac{b}{l^2} & 0 & \frac{a}{l^2} & \frac{b}{l^2} & 0 \\ \frac{l-t}{rl} & 0 & \frac{n(l-t)}{rl} & \frac{t}{rl} & 0 & \frac{nt}{rl} \\ -\frac{na}{rl^2}(l-t) - \frac{nb}{rl^2}(l-t) & -\frac{1}{l} & -\frac{nat}{rl^2} & -\frac{nbt}{rl^2} & \frac{1}{l} \end{bmatrix} \begin{bmatrix} u_{rn}^i \\ u_{zn}^i \\ u_{\theta n}^i \\ u_{rn}^j \\ u_{tn}^j \\ u_{\theta n}^j \end{bmatrix} \quad (C8)$$

If Equation (C8) is evaluated at the center of the element, the average element strains are given by

$$\begin{bmatrix} \bar{\epsilon}_{tn} \\ \bar{\epsilon}_{\theta n} \\ \bar{\gamma}_{\theta tn} \end{bmatrix} = \begin{bmatrix} -\frac{a}{l^2} & -\frac{b}{l^2} & 0 & \frac{a}{l^2} & \frac{b}{l^2} & 0 \\ \frac{1}{2\bar{r}} & 0 & \frac{n}{2\bar{r}} & \frac{1}{2\bar{r}} & 0 & \frac{n}{2\bar{r}} \\ -\frac{na}{2\bar{r}} & -\frac{nb}{2\bar{r}l} & -\frac{1}{l} & -\frac{na}{2\bar{r}l} & -\frac{nb}{2\bar{r}l} & \frac{1}{l} \end{bmatrix} \begin{bmatrix} u_{rn}^i \\ u_{zn}^i \\ u_{\theta n}^i \\ u_{rn}^j \\ u_{zn}^j \\ u_{\theta n}^j \end{bmatrix} \quad (C9a)$$

where $\bar{r} = (r_i + r_j)/2$

or Equation (C9a) written in symbolic form

$$[\epsilon_n] = [G_n] [u_n] \quad (C9b)$$

From Hooke's law, the stresses within the element are given by

$$\begin{bmatrix} \sigma_{tn} \\ \sigma_{\theta n} \\ \tau_{\theta tn} \end{bmatrix} = \frac{E}{(1-\nu^2)} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix} \begin{bmatrix} \epsilon_{tn} \\ \epsilon_{\theta n} \\ \gamma_{\theta tn} \end{bmatrix} = \begin{bmatrix} \tau_1 \\ \tau_2 \\ 0 \end{bmatrix} \quad (10a)$$

where $\tau_1 = \tau_2 = \frac{1 + \nu}{1 - \nu^2} E \alpha_t T_n$

Equation (10a) expressed in symbolic form is

$$[\sigma_n] = [C] [\epsilon_n] + [\tau] \quad (10b)$$

From Equation (1.15), the nodal circle forces in terms of the nodal circle displacement for the harmonic n are given by

$$[S_n] = [k_n] [U_n] + [L_n] T_n \quad (C11)$$

where

$$[K_n] = \bar{r} A [G_n]^T [C] [G_n]$$

$$[L_n] = \bar{r} A [G_n]^T [\tau]$$

For the truncated cone element

$$A = h \cdot \ell$$

These forces are included into the overall equilibrium of the system for the harmonic n by the same technique used for the system of triangular rings. When n equals zero, these equations reduce to the axisymmetric case.

APPENDIX D

PROGRAM LISTING - ARBITRARY AXISYMMETRIC STRUCTURES

Report No. 5654-02 FS

CAXISY STRESS ANALYSIS OF AXISYMMETRIC SOLIDS

C
C
C

DIMENSION AND COMMON STATEMENTS

DIMENSION THDIS(6),THFRC(6)

DIMENSION NPNUM(340),XORD(340),YORD(340),

1DSX(340),DSY(340),XLOAD(340),YLOAD(340),NP(340,10),SXX(340,9),

2SXY(340,9),SYX(340,9),SYY(340,9),NAP(340)

DIMENSION NUME(550),NPI(550),NPJ(550),NPK(550),ET(550),XU(550),

1RO(550),COED(550),DT(550),THERM(550),AJ(550),BJ(550),AK(550),

2BK(550),SIGXX(550),SIGYY(550),SIGXY(550),SLOPE(340)

DIMENSION NPB(340),NPIX(340),LM(3),A(6,6),B(6,6),S(6,6)

COMMON SXX,SXY,SYX,SYY

EQUIVALENCE (SIGXX,RO,NPB), (SIGYY,COED,NPIX), (SIGXY,DT,SLOPE)

C
C
C

READ AND PRINT OF DATA

KINN=5

KOUT=6

150 READ INPUT TAPE KINN,100

WRITE OUTPUT TAPE KOUT,99

WRITE OUTPUT TAPE KOUT,100

READ INPUT TAPE KINN,1,NUMEL,NUMNP,NUMBC,NCPIN,NOPIN,NCYCM,TOLER,

1XFAC,T1

WRITE OUTPUT TAPE KOUT,101,NUMEL

WRITE OUTPUT TAPE KOUT,102,NUMNP

WRITE OUTPUT TAPE KOUT,103,NUMBC

WRITE OUTPUT TAPE KOUT,104,NCPIN

WRITE OUTPUT TAPE KOUT,105,NOPIN

WRITE OUTPUT TAPE KOUT,106,NCYCM

WRITE OUTPUT TAPE KOUT,107,TOLER

WRITE OUTPUT TAPE KOUT,108,XFAC

READ INPUT TAPE KINN,2,(NUME(N),NPI(N),NPJ(N),NPK(N),ET(N),RO(N),

1XU(N),COED(N),DT(N),N=1,NUMEL)

READ INPUT TAPE KINN,3,(NPNUM(M),XORD(M),YORD(M),XLOAD(M),YLOAD(M),

1,DSX(M),DSY(M),M=1,NUMNP)

IF (T1) 160,155,160

155 WRITE OUTPUT TAPE KOUT,110

WRITE OUTPUT TAPE KOUT,2,(NUME(N),NPI(N),NPJ(N),NPK(N),ET(N),RO(N),

1,XU(N),COED(N),DT(N),N=1,NUMEL)

WRITE OUTPUT TAPE KOUT,111

WRITE OUTPUT TAPE KOUT,109,(NPNUM(M),XORD(M),YORD(M),XLOAD(M),

1YLOAD(M),DSX(M),DSY(M),M=1,NUMNP)

C

AXISY001
AXISY002
AXISY003
AXISY004
AXISY005
AXISY006
AXISY007
AXISY008
AXISY009
AXISY010
AXISY011
AXISY012
AXISY013
AXISY014
AXISY015
AXISY016
AXISY017
AXISY018
AXISY019
AXISY020
AXISY021
AXISY022
AXISY023
AXISY024
AXISY025
AXISY026
AXISY027
AXISY028
AXISY029
AXISY030
AXISY031
AXISY032
AXISY033
AXISY034
AXISY035
AXISY036
AXISY037
AXISY038
AXISY039
AXISY040
AXISY041
AXISY042
AXISY043
AXISY044

C C

Report No. 5654-02 FS

INITIALIZATION

```

160 NCYCLE=0
NUMPT=NCPIIN
NUMOPT=NOPIN
DO 175 L=1,NUMNP
DO 170 M=1,9
SXX(L,M)=0.0
SXY(L,M)=0.0
SYX(L,M)=0.0
SYY(L,M)=0.0
170 NP(L,M)=0
NP(L,10)=0
175 NP(L,1)=L

```

C C C

FOR EACH ELEMENT OF THE SYSTEM

```

NTAG=0
DO 2000 N=1,NUMEL

```

C C C

1. INITIALIZATION

```

I=NPJ(N)
J=NPJ(N)
K=NPJ(N)
THICK=(XORD(I)+XORD(J)+XORD(K))/3.
AJ(N)=XORD(J)-XORD(I)
AK(N)=XORD(K)-XORD(I)
BJ(N)=YORD(J)-YORD(I)
BK(N)=YORD(K)-YORD(I)
176 AREA=(AJ(N)*BK(N)-BJ(N)*AK(N))/2.
IF (AREA) 701,701,177
701 WRITE OUTPUT TAPE KOUT,711,(N)
NTAG=1
177 COMM=(1.+XU(N))*(1.-2.*XU(N))
THERM(N)=--ET(N)*COED(N)*DT(N)/(1.-2.*XU(N))

```

C C C C C

2. COMPUTE ELEMENT STIFFNESS MATRIX

A. FORM MATRIX (A)

```

A(1,1)=BJ(N)-BK(N)
A(1,2)=0.0
A(1,3)=BK(N)

```

AXISY045
AXISY046
AXISY047
AXISY048
AXISY049
AXISY050
AXISY051
AXISY052
AXISY053
AXISY054
AXISY055
AXISY056
AXISY057
AXISY058
AXISY059
AXISY060
AXISY061
AXISY062
AXISY063
AXISY064
AXISY065
AXISY066
AXISY067
AXISY068
AXISY069
AXISY070
AXISY071
AXISY072
AXISY073
AXISY074
AXISY075
AXISY076
AXISY077
AXISY078
AXISY079
AXISY080
AXISY081
AXISY082
AXISY083
AXISY084
AXISY085
AXISY086
AXISY087
AXISY088

A(1,4)=0.0
 A(1,5)=-BJ(N)
 A(1,6)=0.0
 A(2,1)=0.0
 A(2,2)=AK(N)-AJ(N)
 A(2,3)=0.0
 A(2,4)=-AK(N)
 A(2,5)=0.0
 A(2,6)=AJ(N)
 A(3,1)=AK(N)-AJ(N)
 A(3,2)=BJ(N)-BK(N)
 A(3,3)=-AK(N)
 A(3,4)=BK(N)
 A(3,5)=AJ(N)
 A(3,6)=-BJ(N)
 C=.66666667*AREA
 A(4,1)=C/XORD(I)
 A(4,2)=0.0
 A(4,3)=C/XORD(J)
 A(4,4)=0.0
 A(4,5)=C/XORD(K)
 A(4,6)=0.0

B. FORM MATRIX (B)

COMM=.25*E(N)*THICK/(COMM*AREA)
 E=XU(N)*COMM
 D=(1.-XU(N))*COMM
 C=(.5-XU(N))*COMM
 B(1,1)=D
 B(1,2)=E
 B(1,3)=0.0
 B(1,4)=E
 B(2,1)=E
 B(2,2)=D
 B(2,3)=0.0
 B(2,4)=E
 B(3,1)=0.0
 B(3,2)=0.0
 B(3,3)=C
 B(3,4)=0.0
 B(4,1)=E
 B(4,2)=E
 B(4,3)=0.0

AXISY089
 AXISY090
 AXISY091
 AXISY092
 AXISY093
 AXISY094
 AXISY095
 AXISY096
 AXISY097
 AXISY098
 AXISY099
 AXISY100
 AXISY101
 AXISY102
 AXISY103
 AXISY104
 AXISY105
 AXISY106
 AXISY107
 AXISY108
 AXISY109
 AXISY110
 AXISY111
 AXISY112
 AXISY113
 AXISY114
 AXISY115
 AXISY116
 AXISY117
 AXISY118
 AXISY119
 AXISY120
 AXISY121
 AXISY122
 AXISY123
 AXISY124
 AXISY125
 AXISY126
 AXISY127
 AXISY128
 AXISY129
 AXISY130
 AXISY131
 AXISY132

$$B(4,4)=D$$

C. FORM ELEMENT STIFFNESS MATRIX (A)T*(B)†(A)

```

DO 182 J=1,6
DO 182 I=1,4
S(I,J)=0.0
DO 182 K=1,4
182 S(I,J)=S(I,J)+B(I,K)*A(K,J)
DO 183 J=1,6
DO 183 I=1,4
DO 183 B(J,I)=S(I,J)
183 B(J,I)=S(I,J)
DO 184 J=1,6
DO 184 I=1,6
S(I,J)=0.0
DO 184 K=1,4
184 S(I,J)=S(I,J)+B(I,K)*A(K,J)

```

3. ADD ELEMENT STIFFNESS TO TOTAL STIFFNESS

```

LM(1)=NPI(N)
LM(2)=NPJ(N)
LM(3)=NPK(N)
DO 200 L=1,3
DO 200 M=1,3
LX=LM(L)
MX=0

```

```

185 MX=MX+1
    IF(NP(LX,MX)-LM(M)) 190,195,190
190 IF(NP(LX,MX)) 185,195,185
195 NP(LX,MX)=LM(M)
    IF (MX-10) 196,702,702
702 WRITE OUTPUT TAPE KOUT,712,(LX)
    NTAG=1
196 SXX(LX,MX)=SXX(LX,MX)+S(2*L-1,2*M-1)
    SKY(LX,MX)=SKY(LX,MX)+S(2*L-1,2*M)
    SYX(LX,MX)=SYX(LX,MX)+S(2*L,2*M-1)
    SYY(LX,MX)=SYY(LX,MX)+S(2*L,2*M)
200

```

5. COMPUTE BODY FORCES

```
DL=AREA*THICK*RO(N)/3.  
I=NP I(N)  
J=NP J(N)
```

AXISY177
AXISY178
AXISY179
AXISY180
AXISY181
AXISY182
AXISY183
AXISY184
AXISY185
AXISY186
AXISY187
AXISY188
AXISY189
AXISY190
AXISY191
AXISY192
AXISY193
AXISY194
AXISY195
AXISY196
AXISY197
AXISY198
AXISY199
AXISY200
AXISY201
AXISY202
AXISY203
AXISY204
AXISY205
AXISY206
AXISY207
AXISY208
AXISY209
AXISY210
AXISY211
AXISY212
AXISY213
AXISY214
AXISY215
AXISY216
AXISY217
AXISY218
AXISY219
AXISY220

```

K=NPK(N)
C=COED(N)*DT(N)
THDIS(1)=C*XORD(I)
THDIS(2)=0.0
THDIS(3)=C*XORD(J)
THDIS(4)=C*BJ(N)
THDIS(5)=C*XORD(K)
THDIS(6)=C*BK(N)
DO 204 L=1,6
THFRC(L)=0.0
DO 204 M=1,6
204 THFRC(L)=THFRC(L)+S(L,M)*THDIS(M)
XLOAD(I)=XLOAD(I)+THFRC(1)
YLOAD(I)=YLOAD(I)+THFRC(2)-DL
XLOAD(J)=XLOAD(J)+THFRC(3)
YLOAD(J)=YLOAD(J)+THFRC(4)-DL
XLOAD(K)=XLOAD(K)+THFRC(5)
YLOAD(K)=YLOAD(K)+THFRC(6)-DL
2000 CONTINUE
C
C
C
COUNT OF ADJACENT NODAL POINTS
DO 206 M=1,NUMNP
MX=1
205 MX=MX+1
IF (NP(M,MX)) 206,206,205
206 NAP(M)=MX-1
C
C
C
INVERSION OF NODAL POINT STIFFNESS
DO 210 M=1,NUMNP
COMM=SXX(M,1)*SYY(M,1)-SXY(M,1)*SYX(M,1)
TEMP=SYY(M,1)/COMM
SYY(M,1)=SXX(M,1)/COMM
SXX(M,1)=TEMP
SXY(M,1)=-SXY(M,1)/COMM
210 SYX(M,1)=-SYX(M,1)/COMM
C
C
C
MODIFICATION OF BOUNDARY FLEXIBILITIES
WRITE OUTPUT TAPE KOUT,112
READ INPUT TAPE KINN,4,(NPB(L),SLOPE(L),L=1,NUMBC)
WRITE OUTPUT TAPE KOUT,4,(NPB(L),NFIX(L),SLOPE(L),L=1,NUMBC)
DO 240 L=1,NUMBC

```



```

WRITE OUTPUT TAPE KOUT,120,NCYCLE,SUM
305 IF (SUM-TOLER) 400,400,310
310 IF (NCYCM-NCYCLE) 400,400,315
315 IF (NCYCLE-NUMOPT) 244,320,320
320 NUMOPT=NUMOPT+NOPIN

PRINT OF NODAL POINT DISPLACEMENTS

400 WRITE OUTPUT TAPE KOUT,99
WRITE OUTPUT TAPE KOUT,100
WRITE OUTPUT TAPE KOUT,121
WRITE OUTPUT TAPE KOUT,122,(NPNUM(M),DSX(M),DSY(M),M=1,NUMNP)

CALCULATION AND PRINT OF ELEMENT STRESSES

WRITE OUTPUT TAPE KOUT,123
DO 420 N=1,NUMEL
  I=NPI(N)
  J=NPJ(N)
  K=NPK(N)

  1. COMPUTE ELEMENT STRAINS

  C=AJ(N)*BK(N)-BJ(N)*AK(N)
  EPX=((BJ(N)-BK(N))*DSX(I)+BK(N)*DSX(J)-BJ(N)*DSX(K))/C
  EPY=((AK(N)-AJ(N))*DSY(I)-AK(N)*DSY(J)+AJ(N)*DSY(K))/C
  EPZ=(DSX(I)+DSX(J)+DSX(K))/(XORD(I)+XORD(J)+XORD(K))
  GAM=((AK(N)-AJ(N))*DSX(I)-AK(N)*DSX(J)+AJ(N)*DSX(K)
    1 + (BJ(N)-BK(N))*DSY(I)+BK(N)*DSY(J)-BJ(N)*DSY(K))/C

  2. COMPUTE ELEMENT STRESSES

  E=ET(N)*XU(N)/((1.+XU(N))*(1.-2.*XU(N)))
  D=E*(1.-XU(N))/XU(N)
  C=E*(1.-2.*XU(N))/(2.*XU(N))
  X= D*EPX + E*EPY + E*EPZ +THERM(N)
  Y= E*EPX + D*EPY + E*EPZ +THERM(N)
  Z= E*EPX + E*EPY + D*EPZ +THERM(N)
  XY=C*GAM
  SIGXX(N)=X
  SIGYY(N)=Y
  SIGXY(N)=XY

  3.COMPUTE PRINCIPAL STRESSES

```

AXISY265
 AXISY266
 AXISY267
 AXISY268
 AXISY269
 AXISY270
 AXISY271
 AXISY272
 AXISY273
 AXISY274
 AXISY275
 AXISY276
 AXISY277
 AXISY278
 AXISY279
 AXISY280
 AXISY281
 AXISY282
 AXISY283
 AXISY284
 AXISY285
 AXISY286
 AXISY287
 AXISY288
 AXISY289
 AXISY290
 AXISY291
 AXISY292
 AXISY293
 AXISY294
 AXISY295
 AXISY296
 AXISY297
 AXISY298
 AXISY299
 AXISY300
 AXISY301
 AXISY302
 AXISY303
 AXISY304
 AXISY305
 AXISY306
 AXISY307
 AXISY308

C

Report No. 5654-02 FS

```

C
  C=(X+Y)/2.0
  R=SQRT(((Y-X)/2.0)**2+XY**2)
  XMAX=C+R
  XMIN=C-R
  PA=0.5*57.29578*ATANF ( 2.* XY/(Y-X))
  IF (2.*X-XMAX-XMIN) 405,420,420
405 IF (PA) 410,420,415
410 PA=PA+90.0
  GO TO 420
415 PA=PA-90.0
C
  4.PRINT OF ELEMENT STRESSES
C
C
  420 WRITE OUTPUT TAPE KOUT,125,(NUME(N),X,Y,Z,XY,XMAX,XMIN,PA)
C
  CALCULATION AND PRINT OF NODAL POINT STRESSES
C
  WRITE OUTPUT TAPE KOUT,823
  DO 900 M=1,NUMNP
    X=0.0
    Y=0.0
    XY=0.0
    SRX=0.0
    SRY=0.0
    E=0.0
    XX=0.0
    TEMP=0.0
    R=0.0
    DO 860 N=1,NUMEL
      I=NPI(N)
      J=NPJ(N)
      K=NPK(N)
      IF (M-I) 830,850,830
830 IF (M-J) 835,845,835
835 IF (M-K) 860,840,860
840 I=NPK(N)
      K=NPI(N)
      GO TO 850
845 I=NPJ(N)
      J=NPI(N)
850 A=ABSF(XORD(J)+XORD(K)-2.*XORD(I))
      B=ABSF(YORD(J)+YORD(K)-2.*YORD(I))
      RY=B/(A+B)
    END DO
  END DO
  AXISY309
  AXISY310
  AXISY311
  AXISY312
  AXISY313
  AXISY314
  AXISY315
  AXISY316
  AXISY317
  AXISY318
  AXISY319
  AXISY320
  AXISY321
  AXISY322
  AXISY323
  AXISY324
  AXISY325
  AXISY326
  AXISY327
  AXISY328
  AXISY329
  AXISY330
  AXISY331
  AXISY332
  AXISY333
  AXISY334
  AXISY335
  AXISY336
  AXISY337
  AXISY338
  AXISY339
  AXISY340
  AXISY341
  AXISY342
  AXISY343
  AXISY344
  AXISY345
  AXISY346
  AXISY347
  AXISY348
  AXISY349
  AXISY350
  AXISY351
  AXISY352

```



```

SRY=SRX+RY
Y=Y+SIGYY(N)*RY
RX=A/(A+B)
SRX=SRX+RX
X=X+SIGXX(N)*RX
R=R+1.0
XY=XY+SIGXY(N)
E=E+ET(N)
XX=XX+XU(N)
TEMP=TEMP+THERM(N)*{1.-2.XU(N)}
860 CONTINUE
X=X/SRX
Y=Y/SRY
XY=XY/R
E=E/R
XX=XX/R
TEMP=TEMP/R
Z=E*DSX(M)/XORD(M)+XX*(X+Y)+TEMP
C=(X+Y)/2.0
R=SQRTF((Y-X)/2.0)**2+XY**2)
XMAX=C+R
XMIN=C-R
PA=0.5*57.29578*ATANF ( 2.* XY/(Y-X))
IF (2.*X-XMAX-XMIN) 805,820,820
805 IF (PA) 810,820,815
810 PA=PA+90.0
GO TO 820
815 PA=PA-90.0
820 WRITE OUTPUT TAPE KOUT,125. (M,X,Y,Z,XY,XMAX,XMIN,PA)
900 CONTINUE

IF (SUM-TOLER) 440,440,430
430 IF (NCYCM-NCYCLE) 440,440,243

440 GO TO 150

      FORMAT STATEMENTS

      1 FORMAT (6I4,2E12.5,4I1)
      2 FORMAT (4I4,4E12.4,F8.4)
      3 FORMAT (1I4,4F8.0,2F12.8)
      4 FORMAT (2I4,1F8.3)
      5 FORMAT (3E15.8)
999 FORMAT (1H1)

```

```

100 FORMAT (72H BCD INFORMATION
1
101 FORMAT(29HNUMBER OF ELEMENTS
=114/)
102 FORMAT(29H NUMBER OF NODAL POINTS
=114/)
103 FORMAT(29H NUMBER OF BOUNDARY POINTS
=114/)
104 FORMAT(29H CYCLE PRINT INTERVAL
=114/)
105 FORMAT(29H OUTPUT INTERVAL OF RESULTS
=114/)
106 FORMAT(29H CYCLE LIMIT
=114/)
107 FORMAT(29H TOLERANCE LIMIT
=1E12.4/)
108 FORMAT(29H OVER RELAXATION FACTOR
=1F6.3)
109 FORMAT (118.4F12.1,2F12.8)
110 FORMAT (74H1EL. I J K E DENSITY POISSON
1 ALPHA DELTA T)
111 FORMAT (80H1 NP X-ORD Y-ORD X-LOAD Y-LOAD Y-LOAAXISY410
ID X-DISP Y-DISP)
112 FORMAT (20H BOUNDARY CONDITIONS)
119 FORMAT(34H0 CYCLE FORCE UNBALANCE)
120 FORMAT (1112.1E20.6)
121 FORMAT (42HONODAL POINT X-DISPLACEMENT Y-DISPLACEMENT)
122 FORMAT (1112.2E15.6)
123 FORMAT(120H1 ELEMENT X-STRESS Y-STRESS
ISS XY-STRESS MAX-STRESS MIN-STRESS
Z-STREAXISY417
DIRECTION)AXISY418
AXISY419
AXISY420
AXISY421
AXISY422
Z-STREAXISY423
DIRECTION)AXISY424
AXISY425
AXIS0426
124 FORMAT (1110.3F20.4,5X,3F15.2)
125 FORMAT (116.4F17.3,1X,3F15.2)
711 FORMAT (32H0ZERO OR NEGATIVE AREA, EL. NO.=114)
712 FORMAT (33H0OVER 8 N.P. ADJACENT TO N.P. NO.114)
823 FORMAT(120H1 N-POINT X-STRESS Y-STRESS
ISS XY-STRESS MAX-STRESS MIN-STRESS
END

```

APPENDIX E

PROGRAM LISTING - AXISYMMETRIC HEAT SHIELDS

Report No. 5654-02 FS

CASHS

ASHS0001
ASHS0002
ASHS0003
ASHS0004
ASHS0005
ASHS0006
ASHS0007
ASHS0008
ASHS0009
ASHS0010
ASHS0011
ASHS0012
ASHS0013
ASHS0014
ASHS0015
ASHS0016
ASHS0017
ASHS0018
ASHS0019
ASHS0020
ASHS0021
ASHS0022
ASHS0023
ASHS0024
ASHS0025
ASHS0026
ASHS0027
ASHS0028
ASHS0029
ASHS0030
ASHS0031
ASHS0032
ASHS0033
ASHS0034
ASHS0035
ASHS0036
ASHS0037
ASHS0038
ASHS0039
ASHS0040
ASHS0041
ASHS0042

DIMENSION

```

DIMENSION
1  XX(5),YY(5),S(10,10),P(10),X(40,10),Y(40,10),U(40,10),V(40,10),
1  SS(800,24),RR(800),KODE(40,10),HED(12),T(40,10),CC(5,3)
4  A(4,4),B(4,4)

```

COMMON YINN YOUT NMAX NMT NUMLD NUMBC TE TR

1 ES.XS.AS.TS.ESH.XSH.ASH.TSH.

1 HER-X-Y-T-U-V-CC-XX-YY-S-P-KODE-RR-SS

READ AND PRINT OF DATA

50 KINN=5

KOUT=6

CALL INPUT

MX=2#MAX

$$NS = NMAX \neq NMAX$$
$$MM = MMAX - 1$$

1-XAZNHN

MBAND=2*(MMAX+2)

NEQ=2*NS

FORMATION OF STIFFNESS ARRAY

DO 175 I=1, N=Q

DO 170 J=1,MEAND

$$170 \text{ SS}(I, J) = 0.0$$

175 $RR(1)=0.0$

DO 200 N = 1, N

DO 200 M=1-MM

DO 180 101.10

$P(1) = 0.0$

DO 180 J=1,10

$$180^\circ S(1,1)=0.0$$
$$X \times X(1) = X(N, \Sigma)$$
$$xx(2)=x(N \cdot M+1)$$
$$XX(\varepsilon) = X(N+1, M)$$
$$xx(A) = x(N+1, M+1)$$

ASHS0043
ASHS0044
ASHS0045
ASHS0046
ASHS0047
ASHS0048
ASHS0049
ASHS0050
ASHS0051
ASHS0052
ASHS0053
ASHS0054
ASHS0055
ASHS0056
ASHS0057
ASHS0058
ASHS0059
ASHS0060
ASHS0061
ASHS0062
ASHS0063
ASHS0064
ASHS0065
ASHS0066
ASHS0067
ASHS0068
ASHS0069
ASHS0070
ASHS0071
ASHS0072
ASHS0073
ASHS0074
ASHS0075
ASHS0076
ASHS0077
ASHS0078
ASHS0079
ASHS0080
ASHS0081
ASHS0082
ASHS0083
ASHS0084

```

YY(1)=Y(N,M)
YY(2)=Y(N,M+1)
YY(3)=Y(N+1,M)
YY(4)=Y(N+1,M+1)
NNN=MB-M
TT=.25*(T(N,M)+T(N,M+1)+T(N+1,M)+T(N+1,M+1))
CALL TRIST(1,4,2,TT,NNN)
CALL TRIST(1,3,4,TT,NNN)

DO 200 I=1,4
  II=((N-1)*MNX+M-1)*2+I
  KK=II+MX
  RR(II)=RR(II)+P(I)
  RR(KK)=RR(KK)+P(I+4)
  DO 190 J=1,4
    JJ=J-I+1
    SS(II,JJ)=SS(II,JJ)+S(I,J)
190  SS(KK,JJ)=SS(KK,JJ)+S(I+4,J+4)
    DO 200 J=1,4
      JJ=J-I+1+MX
      SS(II,JJ)=SS(II,JJ)+S(I,J+4)
200

STIFFNESS OF PLATE ELEMENTS

M=MB
DO 500 N=1,NN

  RI=X(N,M)
  ZI=Y(N,M)
  RJ=X(N+1,M)
  ZJ=Y(N+1,M)
  AA=ZJ-ZI
  BB=RJ-RI
  XL2=AA**2+BB**2
  XL=SQRTF(XL2)

  A(1,1)=1.0/(RI+RJ)
  A(1,2)=0.0
  A(1,3)=A(1,1)
  A(1,4)=0.0
  A(2,1)=-BB/XL2
  A(2,2)=-AA/XL2

```

```

C
A(2,3)=-A(2,1)
A(2,4)=-A(2,2)
B(1,1)=(ESH/(1.-XSH**2))*TSH*XL*(RI+RJ)/2.
B(1,2)=B(1,1)*XSH
B(2,1)=B(1,2)
B(2,2)=B(1,1)
C
DO 470 I=1,2
DO 470 J=1,4
S(I,J)=0.0
DO 470 K=1,2
470 S(I,J)=S(I,J)+B(I,K)*A(K,J)
C
DO 480 I=1,4
DO 480 J=1,4
B(I,J)=0.0
DO 480 K=1,2
480 B(I,J)=B(I,J)+A(K,I)*S(K,J)
C
AT=ASH*(T(N,M)+T(N+1,M))/2.
A(1)=AT*RI
A(2)=AT*ZI
A(3)=AT*RJ
A(4)=AT*ZJ
C
DO 485 I=1,4
P(I)=0.0
DO 485 K=1,4
485 P(I)=P(I)+B(I,K)*A(K)
C
DO 495 I=1,2
II=((N-1)*MMAX+MB-1)*2+I
KK=II+MX
RR(II)=RR(II)+P(I)
RR(KK)=RR(KK)+P(I+2)
DO 490 J=1,2
JJ=J-I+1
SS(II,JJ)=SS(II,JJ)+B(I,J)
490 SS(KK,JJ)=SS(KK,JJ)+B(I+2,J+2)
DO 495 J=1,2
JJ=J-I+1+MX

```

ASHS0085
 ASHS0086
 ASHS0087
 ASHS0088
 ASHS0089
 ASHS0090
 ASHS0091
 ASHS0092
 ASHS0093
 ASHS0094
 ASHS0095
 ASHS0096
 ASHS0097
 ASHS0098
 ASHS0099
 ASHS0100
 ASHS0101
 ASHS0102
 ASHS0103
 ASHS0104
 ASHS0105
 ASHS0106
 ASHS0107
 ASHS0108
 ASHS0109
 ASHS0110
 ASHS0111
 ASHS0112
 ASHS0113
 ASHS0114
 ASHS0115
 ASHS0116
 ASHS0117
 ASHS0118
 ASHS0119
 ASHS0120
 ASHS0121
 ASHS0122
 ASHS0123
 ASHS0124
 ASHS0125
 ASHS0126

```

495 SS(II,JJ)=SS(II,JJ)+B(I,J+2)
C
500 CONTINUE
C
READ IN SPECIFIED RADIAL AND AXIAL LOADS
C
IF(NUMLD) 505, 202, 505
C
505 WRITE OUTPUT TAPE KOUT, 2000
C
DO 550 I=1,NUMLD
READ INPUT TAPE KINN, 1001,N,M,PR,PZ
WRITE OUTPUT TAPE KOUT, 2001, N,M,PR,PZ
K = 2*( (N-1)*MMAX+M ) -1
RR(K) = RR(K) + PR
550 RR(K+1) = RR(K+1) + PZ
C
BOUNDARY CONDITIONS
C
202 DO 60 N=1,NMAX
DO 60 M=1,MMAX
60 CODE(N,M)=0
C
FIX EDGE
C
DO 205 M=1,MMAX
205 CODE(1,M)=1
C
READ IN ADDITIONAL BOUNDARY CONDITIONS
C
WRITE OUTPUT TAPE KOUT, 2002
C
DO 208 I=1,NUMBC
READ INPUT TAPE KINN,1000,NNN,MMM,KKK
WRITE OUTPUT TAPE KOUT, 1000, NNN,MMM,KKK
208 CODE(NNN,MMM)=KKK
C
KK=0
DO 250 N=1,NMAX
DO 250 M=1,MMAX
IF (CODE(N,M)) 210,250,210
210 LL=1

```

ASHS0127
 ASHS0128
 ASHS0129
 ASHS0130
 ASHS0131
 ASHS0132
 ASHS0133
 ASHS0134
 ASHS0135
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 ASHS0137
 ASHS0138
 ASHS0139
 ASHS0140
 ASHS0141
 ASHS0142
 ASHS0143
 ASHS0144
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 ASHS0146
 ASHS0147
 ASHS0148
 ASHS0149
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 ASHS0151
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 ASHS0157
 ASHS0158
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 ASHS0161
 ASHS0162
 ASHS0163
 ASHS0164
 ASHS0165
 ASHS0166
 ASHS0167
 ASHS0168

1000 FORMAT (3I5)
 1001 FORMAT (2I5, 2F10.3)
 2000 FORMAT (46H1 POINTS WITH SPECIFIED RADIAL AND AXIAL LOADS //
 113X 26HN M R-LOAD Z-LOAD)
 2001 FORMAT (9X 2I5, 2F10.3)
 2002 FORMAT (44H1 POINTS WITH ADDITIONAL BOUNDARY CONDITIONS //
 116H N M CODE)
 2003 FORMAT (16H1R-DISPLACEMENTS)
 2004 FORMAT (16H1Z-DISPLACEMENTS)
 END

ASHS0211
 ASHS0212
 ASHS0213
 ASHS0214
 ASHS0215
 ASHS0216
 ASHS0217
 ASHS0218
 ASHS0219
 ASHS0220

```

C C C
SUBROUTINE INPUT
COMMON AND DIMENSION STATEMENTS
DIMENSION HED(12),R(40,10),Z(40,10),T(40,10),CC(5,3),TA(40),
1 TT(50),A(50,5),B(5,5),D(5,3),EE(50,3),U(40,10),V(40,10)
2,KR(5)
COMMON KINN,KOUT,NMAX,MMAX,MB,NMT,NUMLD,NUMBC,TE,TR,
1 ES,XS,AS,TS,ESH,XSH,ASH,TSH,
1 HED,R,Z,T,U,V,CC,TA,TT,A,B,D,EE
READ AND PRINT INPUT DATA
READ INPUT TAPE KINN,1000,
1HED,NMAX,MMAX,MB,NMT,NUMLD,NUMBC,TE,TR,ES,XS,AS,TS,ESH,XSH,ASH,TSHASHS0236
WRITE OUTPUT TAPE KOUT,2000,
1HED,NMAX,MMAX,MB,NMT,NUMLD,NUMBC,TE,TR
CHECK THE SIZE OF NMAX, MMAX, NMT AND LOCATION OF MB. CHECK FOR
ADDITIONAL BOUNDARY CONDITIONS. IF ANY INPUT ERRORS EXIT.
DO 1 I=1,5
1 KR(I) = 0
IF (40-NMAX) 5, 10, 10
5 KR(1) = 1
10 IF (10-MMAX) 15, 20, 20
15 KR(2) = 2
20 IF (MMAX-MB) 25, 25, 30
25 KR(3) = 3
30 IF (50-NMT) 35, 40, 40
35 KR(4) = 4
40 IF (NUMBC) 45, 45, 50
45 KR(5) = 6
50 DO 65 I=1,5
IF(KR(I)) 60, 65, 60
60 WRITE OUTPUT TAPE KOUT, 2011, KR(I)
KR = 1
65 CONTINUE
IF(KR) 70, 75, 70
70 CALL EXIT
75 WRITE OUTPUT TAPE KOUT, 2006

```

```

WRITE OUTPUT TAPE KOUT,2008, ES,XS,AS,TS
WRITE OUTPUT TAPE KOUT,2007
WRITE OUTPUT TAPE KOUT,2008, ESH,XSH,ASH,TSH
READ INPUT TAPE KINN,1001,
1 (R(N,MB),Z(N,MB),T(N,MB),TA(N),N=1,NMAX)
WRITE OUTPUT TAPE KOUT,2001,
1 (R(N,MB),Z(N,MB),T(N,MB),TA(N),N=1,NMAX)

LEAST SQUARE EVALUATION OF MATERIAL PROPERTIES

READ INPUT TAPE KINN,1002,
1 (TT(I),EE(I,1),EE(I,2),EE(I,3),I=1,NMT)
WRITE OUTPUT TAPE KOUT, 2009
WRITE OUTPUT TAPE KOUT,2002,
1 (TT(I),EE(I,1),EE(I,2),EE(I,3),I=1,NMT)

DO 100 I=1,NMT
A(I,1)=1.0
A(I,2)=TT(I)
A(I,3)=TT(I)*TT(I)
A(I,4)=A(I,3)*TT(I)
100 A(I,5)=A(I,4)*TT(I)

CALL LEAST(A,EE,B,D,CC,NMT,5,3)

WRITE OUTPUT TAPE KOUT, 2010
WRITE OUTPUT TAPE KOUT,2002,
1 (TT(I),EE(I,1),EE(I,2),EE(I,3),I=1,NMT)

GENERATE MESH

MC = MB+1
MBB = MB-1
SHL = FLOATF(MBB)
ABL = FLOATF(MMAX-MB)
DO 200 N=1,NMAX

CHECK FOR END POINTS

NL = N-1
NH=N+1
IF(NL)160,150,160
150 SS = 0.
CX = 1.
GO TO 185
160 IF(N-NMAX)180,170,180

```


6 33H ADDITIONAL BOUNDARY CONDITIONS-- I3/
 7 33H SURFACE TEMPERATURE OF ABLATOR-- F6.0/
 8 33H ZERO STRESS TEMPERATURE----- F6.0)
 2001 FORMAT (15H1 R-ORDINATE 5X 10HZ-ORDINATE 5X 10HBOND TEMP. 3X
 1 17HABLATOR THICKNESS / (3F15.3,1F20.4))
 2002 FORMAT (15H0 TEMPERATURE 5X 10HMODULUS A 5X 9HMODULUS B 1X
 120H COEFF. OF EXPANSION / (3F15.0,1E20.5))
 2003 FORMAT (14H1 R-ORDINATES)
 2004 FORMAT (14H1 Z-ORDINATES)
 2005 FORMAT (14H1 TEMPERATURE)
 2006 FORMAT (37HOPROPERTIES OF SANDWICH CORE MATERIAL)
 2007 FORMAT (30HOPROPERTIES OF SANDWICH PLATES)
 2008 FORMAT
 1(28H MODULUS OF ELASTICITY----- F10.0/
 2 28H POISSONS RATIO----- F10.4/
 3 28H COEFFICIENT OF EXPANSION-- F10.8/
 4 28H THICKNESS----- F10.4)
 2009 FORMAT (25H1 MATERIAL PROPERTY DATA)
 2010 FORMAT (1H1 5X 55HLEAST SQUARE EVALUATION OF ABOVE MATERIAL PROPE
 1RTY DATA)
 2011 FORMAT (49H0 ERROR IN ABOVE INPUT DATA. CHECK VALUE ON LINE I3)
 END

ASHS0359
 ASHS0360
 ASHS0361
 ASHS0362
 ASHS0363
 ASHS0364
 ASHS0365
 ASHS0366
 ASHS0367
 ASHS0368
 ASHS0369
 ASHS0370
 ASHS0371
 ASHS0372
 ASHS0373
 ASHS0374
 ASHS0375
 ASHS0376
 ASHS0377
 ASHS0378
 ASHS0379
 ASHS0380

SUBROUTINE TRIST(IX,JX,KX,TT,NNN)

DIMENSION XX(5),YY(5),S(10,10),P(10),A(6,6),B(6,6),C(6,6),LM(3)
1 , HED(12),X(40,10),Y(40,10),T(40,10),CC(5,3),EE(6),U(40,10)
3 ,V(40,10)

COMMON KINN,KOUT,NMAX,MMAX,MB,NMT,NUMLD,NUMBC,TE,TR,
1 ES,XS,AS,TS,ESH,XSH,ASH,TSH,
1 HED,X,Y,T,U,V,CC,XX,YY,S,P,C

INITIALIZATION

AJ=XX(JX)-XX(IX)
AK=XX(KX)-XX(IX)
BJ=YY(JX)-YY(IX)
BK=YY(KX)-YY(IX)
TEMP=AJ*BK-BJ*AK
AREA=TEMP/2.
THICK=(XX(IX)+XX(JX)+XX(KX))/3.
DO 50 I=1,36
A(I)=0.0
50 B(I)=0.0

CALCULATE MATERIAL PROPERTIES

DO 60 I=1,3
60 EE(I)=CC(1,I)+TT*(CC(2,I)+TT*(CC(3,I)+TT*(CC(4,I)+TT*CC(5,I))))
IF (NNN) 70,70,80
70 EP=EE(1)
ALP=EE(3)
XNU=0.3155+0.000174*TT
GO TO 90
80 EP=ES
ALP=AS
XNU=XS
90 AT=ALP*(TT-TR)
COMM=EP/(1.-2.*XNU)
TE=COMM*AT
COMM=COMM/(1.+XNU)

FORM MATRIX (B)

ASHS0381
ASHS0382
ASHS0383
ASHS0384
ASHS0385
ASHS0386
ASHS0387
ASHS0388
ASHS0389
ASHS0390
ASHS0391
ASHS0392
ASHS0393
ASHS0394
ASHS0395
ASHS0396
ASHS0397
ASHS0398
ASHS0399
ASHS0400
ASHS0401
ASHS0402
ASHS0403
ASHS0404
ASHS0405
ASHS0406
ASHS0407
ASHS0408
ASHS0409
ASHS0410
ASHS0411
ASHS0412
ASHS0413
ASHS0414
ASHS0415
ASHS0416
ASHS0417
ASHS0418
ASHS0419
ASHS0420
ASHS0421
ASHS0422

```

E=XNU*COMM
D=(1.-XNU)*COMM
C=(.5-XNU)*COMM
B(1,1)=D
B(1,2)=E
B(1,4)=E
B(2,1)=E
B(2,2)=D
B(2,4)=E
B(3,3)=C
B(4,1)=E
B(4,2)=E
B(4,4)=D

```

```

FORM MATRIX (A)

```

```

A(1,1)=BJ-BK
A(1,3)=BK
A(1,5)=-BJ
A(2,2)=AK-AJ
A(2,4)=-AK
A(2,6)=AJ
A(3,1)=AK-AJ
A(3,2)=BJ-BK
A(3,3)=-AK
A(3,4)=BK
A(3,5)=AJ
A(3,6)=-BJ
C=.333333*TEMP/THICK

```

```

A(4,1)=C
A(4,3)=C
A(4,5)=C

```

```

DO 95 I=1,36
95 A(I)=A(I)/TEMP

```

```

FORM TRIANGULAR ELEMENT STIFFNESS MATRIX (A)T(B)(A)

```

```

COMM=THICK*AREA
DO 100 L=1,4
DO 100 M=1,6
C(L,M)=0.0
DO 100 N=1,4

```

```

ASHS0423
ASHS0424
ASHS0425
ASHS0426
ASHS0427
ASHS0428
ASHS0429
ASHS0430
ASHS0431
ASHS0432
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ASHS0434
ASHS0435
ASHS0436
ASHS0437
ASHS0438
ASHS0439
ASHS0440
ASHS0441
ASHS0442
ASHS0443
ASHS0444
ASHS0445
ASHS0446
ASHS0447
ASHS0448
ASHS0449
ASHS0450
ASHS0451
ASHS0452
ASHS0453
ASHS0454
ASHS0455
ASHS0456
ASHS0457
ASHS0458
ASHS0459
ASHS0460
ASHS0461
ASHS0462
ASHS0463
ASHS0464

```



```

SUBROUTINE STRESS
  DIMENSION T(40,10),EE(3),CC(5,3),HED(12),
  1 XX(5),YY(5),S(10,10),P(10),X(40,10),Y(40,10),U(40,10),V(40,10),
  2 A(9,9),B(9,2),C(6,6),D(9,9),E(9,2),SIGTT(40,10),SIGXM(40,10),
  3 SIGXX(40,10),SIGYY(40,10),SIGXY(40,10),SIGNN(40,10),SIGSS(40,10),
  4 SIGNS(40,10),BSIGS(40),BSIGT(40),SIG(4)
  COMMON KINN,KOUT,NMAX,MMAX,MB,NMT,NUMLD,NUMBC,TE,TR,
  1 ES,XS,AS,TS,IESH,XSH,ASH,TSH,
  1 HED,X,Y,T,U,V,CC,XX,YY,S,P,C
  1 A,B,D,E,NN,MM,SIGXX,SIGYY,SIGXY,SIGMX,SIGMN,SIGTT,BSIGS,BSIGT
  1 ,EE,SIGNN,SIGNS,SIGSS,SIGXM,SIG
  NN=NMAX-1
  MM=MMAX-1
  DO 200 N=1,NN
  DO 200 M=1,MM
    XX(1)=X(N,M)
    XX(2)=X(N,M+1)
    XX(3)=X(N+1,M)
    XX(4)=X(N+1,M+1)
    YY(1)=Y(N,M)
    YY(2)=Y(N,M+1)
    YY(3)=Y(N+1,M)
    YY(4)=Y(N+1,M+1)
    NNN=MB-M
    TT=.25*(T(N,M)+T(N,M+1)+T(N+1,M)+T(N+1,M+1))
    CALL TRIST(1,4,2,TT,NNN)
    P(1)=U(N,M)
    P(2)=V(N,M)
    P(3)=U(N+1,M+1)
    P(4)=V(N+1,M+1)
    P(5)=U(N,M+1)
    P(6)=V(N,M+1)
  DO 150 I=1,4

```

```

ASHS0548
ASHS0549
ASHS0550
ASHS0551
ASHS0552
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ASHS0556
ASHS0557
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ASHS0559
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ASHS0581
ASHS0582
ASHS0583
ASHS0584
ASHS0585
ASHS0586
ASHS0587
ASHS0588
ASHS0589

SIG(I)=0.0
DO 150 K=1,6
  150 SIG(I)=SIG(I)+C(I,K)*P(K)
C
CALL TRIST(1,3,4,TT,NNN)
C
P(1)=U(N,M)
P(2)=V(N,M)
P(3)=U(N+1,M)
P(4)=V(N+1,M)
P(5)=U(N+1,M+1)
P(6)=V(N+1,M+1)
C
DO 170 I=1,4
DO 160 K=1,6
  160 SIG(I)=SIG(I)+C(I,K)*P(K)
  170 SIG(I)=SIG(I)/2.0
C
SIGXX(N,M)=SIG(1)-TE
SIGYY(N,M)=SIG(2)-TE
SIGXY(N,M)=SIG(3)
SIGTT(N,M)=SIG(4)-TE
C
COST = .5*( X(N,MB) + X(N+1,MB) - X(N,1) - X(N+1,1) ) / TS
SINT = .5*( Y(N,MB) + Y(N+1,MB) - Y(N,1) - Y(N+1,1) ) / TS
COS2=COST**2
SIN2=SINT**2
TT=2.0*SIGXY(N,M)*SINT*COST
SIGNN(N,M)=SIGXX(N,M)*COS2+SIGYY(N,M)*SIN2+TT
SIGSS(N,M)=SIGXX(N,M)*SIN2+SIGYY(N,M)*COS2-TT
SIGNS(N,M)=(SIGYY(N,M)-SIGXX(N,M))*SINT*COST+SIGXY(N,M)*
  1 (COS2-SIN2)
200 CONTINUE
C
PRINT STRESSES
C
WRITE OUTPUT TAPE KOUT,1006
CALL PRINTM(SIGTT,NN,MM,40)
WRITE OUTPUT TAPE KOUT,1007
CALL PRINTM(SIGXX,NN,MM,40)
WRITE OUTPUT TAPE KOUT,1008
CALL PRINTM(SIGY,NN,MM,40)

```

WRITE OUTPUT TAPE KOUT,1009
CALL PRINTM(SIGXY,NN,MM,40)
WRITE OUTPUT TAPE KOUT,1011
CALL PRINTM(SIGSS,NN,MM,40)

C
C
C
CALCULATION OF STRESSES IN SANDWICH PLATES

ED=ESH/(1.-XSH**2)
EE=XSH*ED
MM=MB-1

DO 225 N=1,NN
DO 225 M=1,MB,MM
RI=X(N,M)
ZI=Y(N,M)
RJ=X(N+1,M)
ZJ=Y(N+1,M)
AA=ZJ-ZI
BB=RJ-RI
XL2=AA**2+BB**2

AT=.5*ASH*(T(N,M)+T(N+1,M))
EPT=(U(N,M)+U(N+1,M))/(RI+RJ)-AT
EPS=(BB*(U(N+1,M)-U(N,M))+AA*(V(N+1,M)-V(N,M)))/XL2--AT
SIGTT(N,M)=ED*EPT+EE*EPS
SIGSS(N,M)=EE*EPT+ED*EPS

225
C
C
C
WRITE OUTPUT TAPE KOUT, 1013,
1(N, SIGTT(N,1), SIGSS(N,1), SIGTT(N,MB), SIGSS(N,MB), N=1,NN)

C
C
C
CALCULATION OF STRESSES IN BOND LAYER

DO 300 N=1,NN
TT = 0.50*(T(N,MB)+T(N+1,MB))
DO 260 I=1,3
260 EE(I)=CC(1,I)+TT*(CC(2,I)+TT*(CC(3,I)+TT*(CC(4,I)+TT*CC(5,I))))
XNU = 0.3155+0.000174*TT
AA = Y(N+1,MB)-Y(N,MB)
BB = X(N+1,MB)-X(N,MB)
XL2 = AA**2+BB**2
EEE = XNU*SIGNN(N,MB)/EE(2) - EE(3)*TT
EPT = (U(N,MB)+U(N+1,MB))/(X(N+1,MB)+X(N,MB)) + EEE
EPS = (BB*(U(N+1,MB)-U(N,MB))+AA*(V(N+1,MB)-V(N,MB)))/XL2+EEE

```

      COMM = EE(2)/(1.-XNU**2)
      BSGS(N) = COMM*(EPS+XNU*EPT)
    300 BSGT(N) = COMM*(EPT+XNU*EPS)
      WRITE OUTPUT TAPE KOUT, 1014, (N, BSGS(N), BSGT(N), SIGNN(N,MB), ASHS0632
        1 SIGNS(N,MB), N=1,NN ) ASHS0633
C                                     ASHS0634
C                                     ASHS0635
C                                     ASHS0636
      250 RETURN ASHS0637
C                                     ASHS0638
C                                     ASHS0639
C                                     ASHS0640
C                                     ASHS0641
      1006 FORMAT (10H1 T-STRESS) ASHS0642
      1007 FORMAT (10H1 R-STRESS) ASHS0643
      1008 FORMAT (10H1 Z-STRESS) ASHS0644
      1009 FORMAT (10H1RZ-STRESS) ASHS0645
      1011 FORMAT (10H1 S-STRESS) ASHS0646
      1013 FORMAT ( 1H1 42X 27HSTRESSES IN SANDWICH PLATES / ASHS0647
        1          27X 11HLOWER PLATE 39X 11HUPPER PLATE / ASHS0648
        2          20X 5HMERID 15X 4HHOOP 25X 5HMERID 15X 4HHOOP / ASHS0649
        3          4X 1HN 14X 6HSTRESS 14X 6HSTRESS 24X 6HSTRESS 14X 6HSTRESS // ASHS0650
        4          ( 15, 2F20.0, 10X 2F20.0 ) ASHS0651
      1014 FORMAT (1H1 11X 22HSTRESSES IN BOND LAYER // ASHS0652
        145H   N       MERID     HOOP     NORMAL     SHEAR // ASHS0653
        2(15, 4F10.1) ) ASHS0654
C                                     ASHS0655
      END

```

SUBROUTINE LEAST(A,B,C,D,E,N,M,L)

DIMENSION A(50,5),B(50,3),C(5,5),D(5,3),E(5,3)

(C)=(A)*A AND (D)=(A)*B

DO 200 I=1,M

DO 100 J=1,M

C(I,J)=0.0

DO 100 K=1,N

100 C(I,J)=C(I,J)+A(K,I)*A(K,J)

DO 200 J=1,L

D(I,J)=0.0

DO 200 K=1,N

200 D(I,J)=D(I,J)+A(K,I)*B(K,J)

INVERT (C)

CALL INVERT(C,M,5,B,E)

(E)=(C)*(D)

DO 300 I=1,M

DO 300 J=1,L

E(I,J)=0.0

DO 300 K=1,M

300 E(I,J)=E(I,J)+C(I,K)*D(K,J)

CHECK RESULTS (B)=(A)*(E)

DO 400 I=1,N

DO 400 J=1,L

B(I,J)=0.0

DO 400 K=1,M

400 B(I,J)=B(I,J)+A(I,K)*E(K,J)

RETURN

END

ASHS0656
ASHS0657
ASHS0658
ASHS0659
ASHS0660
ASHS0661
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ASHS0664
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ASHS0680
ASHS0681
ASHS0682
ASHS0683
ASHS0684
ASHS0685
ASHS0686
ASHS0687
ASHS0688
ASHS0689
ASHS0690
ASHS0691
ASHS0692
ASHS0693
ASHS0694

```

C      SUBROUTINE SYMSOL (A,B,NN,MM)
C
C      DIMENSION A(800,24),B(800),C(24)
C
C      N = 0
C      100 N = N+1
C
C      REDUCE N TH EQUATION
C
C      1. DIVIDE RIGHT SIDE BY DIAGONAL ELEMENT
C
C      B(N) = B(N) / A(N,1)
C
C      2. CHECK FOR LAST EQUATION
C
C      IF(N-NN) 150,300,150
C
C      3. DIVIDE N TH EQUATION BY DIAGONAL ELEMENT
C
C      150 DO 200 K=2,MM
C      C(K) = A(N,K)
C      200 A(N,K) = A(N,K) / A(N,1)
C
C      4. REDUCE REMAINING EQUATIONS
C
C      DO 260 L=2,MM
C      I = N+L-1
C      IF(NN-I) 260,240,240
C      240 J=0
C      DO 250 K=L,MM
C      J=J+1
C      250 A(I,J) = A(I,J) - C(L) * A(N,K)
C      B(I) = B(I) - C(L) * B(N)
C      260 CONTINUE
C      GO TO 100
C
C      BACK SUBSTITUTION
C
C      300 N = N-1
C
C      1. CHECK FOR FIRST EQUATION

```

ASHS0825
ASHS0826
ASHS0827
ASHS0828
ASHS0829
ASHS0830
ASHS0831
ASHS0832
ASHS0833
ASHS0834
ASHS0835
ASHS0836
ASHS0837
ASHS0838

IF(N) 350,500,350
C
C
C
2. CALCULATE UNKNOWN B(N)
350 DO 400 K=2,MM
L = N+K-1
IF(NN-L) 400,370,370
370 B(N) = B(N) - A(N,K) * B(L)
400 CONTINUE
GO TO 300
C
500 RETURN
C
END

ASHS0695
ASHS0696
ASHS0697
ASHS0698
ASHS0699
ASHS0700
ASHS0701
ASHS0702
ASHS0703
ASHS0704
ASHS0705
ASHS0706
ASHS0707
ASHS0708
ASHS0709
ASHS0710
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ASHS0731
ASHS0732
ASHS0733
ASHS0734
ASHS0735
ASHS0736

SUBROUTINE INVERT(A,NN,N,M,C)
GENERAL MATRIX INVERSION SUBROUTINE
DIMENSION A(1),M(1),C(1)
DO 90 I=1,NN
90 M(I)=-1
DO 140 I=1,NN
LOCATE LARGEST ELEMENT
D=0.0
DO 112 L=1,NN
IF (M(L)) 100,100,112
100 J=L
DO 110 K=1,NN
IF (M(K)) 103,103,108
103 IF (ABSF(D)-ABSF(A(J))) 105,105,108
105 LD=L
KD=K
D=A(J)
108 J=J+N
110 CONTINUE
112 CONTINUE
INTERCHANGE ROWS
TEMP=-M(LD)
M(LD)=M(KD)
M(KD)=TEMP
L=LD
K=KD
DO 114 J=1,NN
C(J)=A(L)
A(L)=A(K)
A(K)=C(J)
L=L+N
114 K=K+N
DIVIDE COLUMN BY LARGEST ELEMENT

ASHS0737
 ASHS0738
 ASHS0739
 ASHS0740
 ASHS0741
 ASHS0742
 ASHS0743
 ASHS0744
 ASHS0745
 ASHS0746
 ASHS0747
 ASHS0748
 ASHS0749
 ASHS0750
 ASHS0751
 ASHS0752
 ASHS0753
 ASHS0754
 ASHS0755
 ASHS0756
 ASHS0757
 ASHS0758
 ASHS0759
 ASHS0760
 ASHS0761
 ASHS0762
 ASHS0763
 ASHS0764
 ASHS0765
 ASHS0766
 ASHS0767
 ASHS0768
 ASHS0769
 ASHS0770
 ASHS0771
 ASHS0772
 ASHS0773
 ASHS0774
 ASHS0775
 ASHS0776
 ASHS0777
 ASHS0778

```

C      NR=(KD-1)*N+1
      NH=NR+N-1
      DO 115 K=NR,NH
115   A(K)=A(K)/D
C
C      REDUCE REMAINING ROWS AND COLUMNS
C
      L=1
      DO 135 J=1,NN
      IF (J-KD) 130,125,130
125   L=L+N
      GO TO 135
130   DO 134 K=NR,NH
      A(L)=A(L)-C(J)*A(K)
134   L=L+1
135   CONTINUE
C
C      REDUCE ROW
C
      C(KD)=-1.0
      J=KD
      DO 140 K=1,NN
      A(J)=-C(K)/D
140   J=J+N
C
C      INTERCHANGE COLUMNS
C
      DO 200 I=1,NN
      L=0
150   L=L+1
      IF(M(L)-I) 150,160,150
160   K=(L-1)*N+1
      J=(I-1)*N+1
      M(L)=M(I)
      M(I)=I
      DO 200 L=1,NN
      TEMP=A(K)
      A(K)=A(J)
      A(J)=TEMP
      J=J+1
200   K=K+1
  
```

ASHS0779
ASHS0780
ASHS0781
ASHS0782

RETURN

END

C C

Report No. 5654-02 FS

Page E23

```

C
SUBROUTINE PRINTM (A,NR,NC,MAXR)
SUBROUTINE TO PRINT ANY ARRAY
DIMENSION A(1),NHED(10)
COMMON KINN,KOUT
C
DO 50 I=1,NC,10
  II=NC-I+1
  IF (II-10) 20,20,10
  10 II=10
  20 DO 30 J=1,II
  30 NHED(J)=I+J-1
C
  WRITE OUTPUT TAPE KOUT,120,(NHED(J),J=1,II)
C
DO 50 J=1,NR
  KL=J+(I-1)*MAXR
  KH=KL+(II-1)*MAXR
  50 WRITE OUTPUT TAPE KOUT,130,(J,(A(K),K=KL,KH,MAXR))
C
  RETURN
C
  120 FORMAT (8H0  N/M 10I11)
  130 FORMAT (15,3X,10F11.3)
  END

```

ASHS0839
 ASHS0840
 ASHS0841
 ASHS0842
 ASHS0843
 ASHS0844
 ASHS0845
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 ASHS0847
 ASHS0848
 ASHS0849
 ASHS0850
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 ASHS0859
 ASHS0860
 ASHS0861
 ASHS0862

APPENDIX F

PROGRAM LISTING - NON-AXISYMMETRIC HEAT SHIELDS

```

CNAHS      NON-AXISYMMETRIC ANALYSIS HEAT SHIELDS
C
      DIMENSION R(30,8),Z(30,8),T(30,8),TC(30,8),E(10,10),CC(10,10),
1 XX(5),YY(5),S(15,15),P(15),KODE(30,10),
2 SS(720,30),RR(720),NTAG(3),C(9,9),B(9,9),G(9,9)
      COMMON KINN,KOUT,N,M,L,NMAX,MMAX,MB,NMT,NH,NB,TE,TTT,ES,XS,AS,TS,
1 ESH,XSH,ASH,TSH,R,Z,T,TC,E,CC,XX,YY,S,P,C,B,G,KODE,RR,SS
C
50 KINN=5
   KOUT=6
   REWIND 20
C
   CALL INPUT
C
   MX=3*MMAX
   NS=NMAX*MMAX
   MM=MMAX-1
   NN=NMAX-1
   MBAND=3*(NMAX+2)
   NEQ=3*NS
   MB=MB-1
   MCC=MB+1
C
      READ IN ADDITIONAL BOUNDARY CONDITIONS
C
      DO 60 N=1,NMAX
      DO 60 M=1,MMAX
60 KODE(N,M)=0
      WRITE OUTPUT TAPE KOUT,2000
      DO 70 K=1,NB
      READ INPUT TAPE KINN, 1000, N,M,KK
      KODE(N,M)=KK
      WRITE OUTPUT TAPE KOUT,2001, N,M,KODE(N,M)
70 CONTINUE
C
      SOLVE STRUCTURE FOR EACH HARMONIC
C
      DO 500 L=1,NH
      WRITE OUTPUT TAPE KOUT,2006, L
C
      COMPUTE FOURIER COEFFICIENTS FOR HARMONIC L
C
      XL=0.0

```

```

TT=0.0
IF(L-1) 102,101,102
101 TT=TE
102 DO 105 N=1,NMAX
TC(N,MMAX)=TT
TC(N,MB)=E(1,L)+XL*(E(2,L)+XL*(E(3,L)+XL*(E(4,L)+XL*(E(5,L)
1 +XL*(E(6,L))))))
XL=XL+SQRTF((R(N+1,MB)-R(N,MB))**2+(Z(N+1,MB)-Z(N,MB))**2)
DO 104 M=1,MBB
104 TC(N,M)=TC(N,MB)
TXX=(TC(N,MMAX)-TC(N,MB))/FLOATF(MMAX-MB)**2
DO 105 M=MCC,MMAX
105 TC(N,M)=TC(N,MB)+TXX*FLOATF(M-MB)**2

IF(L-1) 106,106,108
106 DO 107 N=1,NMAX
DO 107 M=1,MMAX
107 TC(N,M)=TC(N,M)-TTT
108 CALL PRINTM(TC,NMAX,MMAX,30)

C
C
C
FORMATION OF STIFFNESS ARRAY
DO 175 I=1,NEQ
DO 170 J=1,MBAND
170 SS(I,J)=0.0
175 RR(I)=0.0

C
DO 200 N=1,NN
DO 200 M=1,MM
C
DO 180 I=1,15
P(I)=0.0
DO 180 J=1,15
180 S(I,J)=0.0

C
XX(1)=R(N,M)
XX(2)=R(N,M+1)
XX(3)=R(N+1,M)
XX(4)=R(N+1,M+1)
YY(1)=Z(N,M)
YY(2)=Z(N,M+1)
YY(3)=Z(N+1,M)
YY(4)=Z(N+1,M+1)

```

NAHS0045
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 NAHS0058
 NAHS0059
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 NAHS0065
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 NAHS0070
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 NAHS0123
 NAHS0124
 NAHS0125
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 NAHS0130
 NAHS0131
 NAHS0132

```

CALL STIFFT(1,4,2)
CALL STIFFT(1,3,4)

ADD QUADRILATERAL STIFFNESS TO TOTAL STIFFNESS

DO 200 I=1,6
  II=((N-1)*MMAX+M-1)*3+I
  KK=II+MX
  RR(II)=RR(II)+P(I)
  RR(KK)=RR(KK)+P(I+6)
  DO 190 J=1,6
    JJ=J-I+1
    SS(II,JJ)=SS(II,JJ)+S(I,J)
    190 SS(KK,JJ)=SS(KK,JJ)+S(I+6,J+6)
  DO 200 J=1,6
    JJ=J-I+1+MX
    200 SS(II,JJ)=SS(II,JJ)+S(I,J+6)

STIFFNESS OF PLATE ELEMENTS

MBB=MB-1
DO 600 N=1,NN
DO 600 M=1,MB,MBB

CALL STIFFP

DO 600 I=1,3
  II=((N-1)*MMAX+M-1)*3+I
  KK=II+MX
  RR(II)=RR(II)+P(I)
  RR(KK)=RR(KK)+P(I+3)
  DO 590 J=1,3
    JJ=J-I+1
    SS(II,JJ)=SS(II,JJ)+S(I,J)
    590 SS(KK,JJ)=SS(KK,JJ)+S(I+3,J+3)
  DO 600 J=1,3
    JJ=J-I+1+MX
    600 SS(II,JJ)=SS(II,JJ)+S(I,J+3)

SET BOUNDARY CONDITIONS ALONG AXIS FOR HARMONIC L

DO 208 M=1,MMAX
  IF(L-2) 205,206,207
  205 CODE(1,M)=4
  
```

NAHS0133
 NAHS0134
 NAHS0135
 NAHS0136
 NAHS0137
 NAHS0138
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 NAHS0171
 NAHS0172
 NAHS0173
 NAHS0174
 NAHS0175
 NAHS0176

```

GO TO 208
206 CODE(1,M)=3
GO TO 208
207 CODE(1,M)=7
208 CONTINUE

      MODIFY STIFFNESS MATRIX

      K=1
      DO 300 N=1,NMAX
      DO 300 M=1,MMAX
      NTAG(1)=0
      NTAG(2)=0
      NTAG(3)=0
      KK=CODE(N,M)
      IF(KK) 250,250,210
210 GO TO (230,220,245,215,225,240,235),KK
215 NTAG(1)=K
220 NTAG(2)=K+1
      GO TO 250
225 NTAG(3)=K+2
230 NTAG(1)=K
      GO TO 250
235 NTAG(1)=K
240 NTAG(2)=K+1
245 NTAG(3)=K+2
250 K=K+3
      DO 300 LL=1,3
      II=NTAG(LL)
      IF(II)300,300,260
260 DO 270 J=1,MBAND
      SS(II,J)=0.0
      III=II+1-J
      IF(III) 270,270,265
265 SS(III,J)=0.0
270 CONTINUE
      SS(II,1)=1.0
      RR(II)=0.0
      300 CONTINUE

      SOLVE FOR DISPLACEMENTS

      CALL SYMSOL(SS,RR,NEQ,MBAND)
  
```


NAHS0177
 NAHS0178
 NAHS0179
 NAHS0180
 NAHS0181
 NAHS0182
 NAHS0183
 NAHS0184
 NAHS0185
 NAHS0186
 NAHS0187
 NAHS0188
 NAHS0189
 NAHS0190
 NAHS0191

C CALL STRESS
 C 500 CONTINUE
 C CALL OUTPUT
 C FORMAT STATEMENTS
 C 1000 FORMAT (3I5)
 C 2000 FORMAT (20H1BOUNDARY CONDITIONS // 18H M CODE)
 C 2001 FORMAT (3I6)
 C 2006 FORMAT (3HIL=13)
 C END

NAHS0192	
NAHS0193	
NAHS0194	
NAHS0195	
NAHS0196	
NAHS0197	
NAHS0198	
NAHS0199	
NAHS0200	
NAHS0201	
NAHS0202	
NAHS0203	
NAHS0204	
NAHS0205	
NAHS0206	
NAHS0207	
NAHS0208	
NAHS0209	
NAHS0210	
NAHS0211	
NAHS0212	
NAHS0213	
NAHS0214	
NAHS0215	
NAHS0216	
NAHS0217	
NAHS0218	
NAHS0219	
NAHS0220	
NAHS0221	
NAHS0222	
NAHS0223	
NAHS0224	
NAHS0225	
NAHS0226	
NAHS0227	
NAHS0228	
NAHS0229	
NAHS0230	
NAHS0231	
NAHS0232	
NAHS0233	
NAHS0234	
NAHS0235	


```

SUBROUTINE INPUT
COMMON AND DIMENSION STATEMENTS
DIMENSION R(30,8),Z(30,8),T(30,8),TC(30,8),E(10,10),CC(10,10),
1 RR(10),ZZ(10),TA(30),TT(50),A(50,10),B(10,10),D(10,10),
2 EE(50,10),HED(12)
COMMON KINN,KOUT,N,M,L,NMAX,MMAX,MB,NMT,NH,NB,TE,TTT,ES,XS,AS,TS,
1 ESH,XSH,ASH,TSH,R,Z,T,TC,E,CC,RR,ZZ,TA,TT,A,B,D,EE,HED
READ AND PRINT INPUT DATA
READ INPUT TAPE KINN,1000.
1 HED,NMAX,MMAX,MB,NMT,NH,NB,TE,TTT,ES,XS,AS,TS,ESH,XSH,ASH,TSH
WRITE OUTPUT TAPE KOUT,2000. HED,NMAX,MMAX,MB,NMT,NH,NB,TE,TTT
WRITE OUTPUT TAPE KOUT,2006
WRITE OUTPUT TAPE KOUT,2008. ES,XS,AS,TS
WRITE OUTPUT TAPE KOUT,2007
WRITE OUTPUT TAPE KOUT,2008. ESH,XSH,ASH,TSH
READ INPUT TAPE KINN,1001.
1 (R(N,MB),Z(N,MB),T(N,MB),TA(N),N=1,NMAX)
WRITE OUTPUT TAPE KOUT,2001.
1 (R(N,MB),Z(N,MB),T(N,MB),TA(N),N=1,NMAX)
LEAST SQUARE EVALUATION OF MATERIAL PROPERTIES
READ INPUT TAPE KINN,1002.
1 (TT(I),EE(I,1),EE(I,2),EE(I,3),I=1,NMT)
WRITE OUTPUT TAPE KOUT, 2011
WRITE OUTPUT TAPE KOUT,2002.
1 (TT(I),EE(I,1),EE(I,2),EE(I,3),I=1,NMT)
DO 50 I=1,NMT
A(I,1)=1.0
A(I,2)=TT(I)
A(I,3)=TT(I)*TT(I)
A(I,4)=A(I,3)*TT(I)
50 A(I,5)=A(I,4)*TT(I)
CALL LEAST(A,EE,B,D,CC,NMT,5,3)
WRITE OUTPUT TAPE KOUT, 2012
WRITE OUTPUT TAPE KOUT,2002.

```

1 (TT(I),EE(I,1),EE(I,2),EE(I,3),I=1,NMT)

GENERATE MESH

MC = MB+1
 MBB = MB-1
 SHL = FLOATF(MBB)
 ABL = FLOATF(MMAX-MB)
 DO 200 N=1,NMAX

CHECK FOR END POINTS

NL = N-1
 NNH=N+1
 IF(NL)160,150,160

150 SS = 0.
 CX = 1.

GO TO 185
 160 IF(N-NMAX)180,170,180
 170 NNH=N

COMPUTE SIN AND COS AT EACH POINT ON BOND LINE

180 XX = R(NNH,MB) - R(NL,MB)
 YY = Z(NNH,MB) - Z(NL,MB)
 ZZ = SQRTF(XX**2 + YY**2)
 SS = YY/ZZ
 CX = XX/ZZ

COMPUTE COORDINATES OF POINTS IN CONSTANT THICKNESS SHELL

185 DO 190 M=1,MBB
 TT = TS*FLOATF(MB-M)/SHL
 T(N,M) = T(N,MB)
 R(N,M) = R(N,MB) + TT*SS
 190 Z(N,M) = Z(N,MB) - TT*CX

COMPUTE COORDINATES OF POINTS IN VARIABLE THICKNESS ABLATER

DO 200 M=MC,MMAX
 TT = TA(N)*FLOATF(M-MB)/ABL
 T(N,M) = T(N,MB) + (TE - T(N,MB)) * (TT/TA(N))**2
 R(N,M) = R(N,MB) - TT*SS
 200 Z(N,M) = Z(N,MB) + TT*CX

NAHS0280
NAHS0281
NAHS0282
NAHS0283
NAHS0284
NAHS0285
NAHS0286
NAHS0287
NAHS0288
NAHS0289
NAHS0290
NAHS0291
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NAHS0293
NAHS0294
NAHS0295
NAHS0296
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NAHS0298
NAHS0299
NAHS0300
NAHS0301
NAHS0302
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NAHS0319
NAHS0320
NAHS0321
NAHS0322
NAHS0323

DUMP OUT R-ORDINATE TABLE AND Z-ORDINATE TABLE

WRITE OUTPUT TAPE KOUT,2003
CALL PRINTM (R,NMAX,MMAX,30)
WRITE OUTPUT TAPE KOUT,2004
CALL PRINTM (Z,NMAX,MMAX,30)
WRITE OUTPUT TAPE KOUT,2005
CALL PRINTM (T,NMAX,MMAX,30)

READ IN THREE-DIMENSIONAL TEMPERATURE DISTRIBUTION

TT(1)=0.0
DO 240 I=2,19
240 TT(I)=TT(I-1)+10.
READ INPUT TAPE KINN, 1003,((RR(J),J=1,9),(ZZ(J),J=1,9))
WRITE OUTPUT TAPE KOUT,2009,((RR(J),J=1,9),(ZZ(J),J=1,9))
READ INPUT TAPE KINN, 1003,((EE(I),J=1,9),I=1,19)
WRITE OUTPUT TAPE KOUT, 2013
WRITE OUTPUT TAPE KOUT,2010,(TT(I),(EE(I),J=1,9),I=1,19)

DO 250 I=1,19
XX=0.17453*FLOATF(I-1)
DO 250 J=1,NH
YY=XX*FLOATF(J-1)
250 A(I,J)=COSF(YY)

DO 270 I=1,9
DO 260 J=1,NH
E(J,I)=.5*(EE(1,I)*A(1,J)+EE(19,I)*A(19,J))
DO 255 K=2,18
255 E(J,I)=E(J,I)+EE(K,I)*A(K,J)
260 E(J,I)=E(J,I)/9.
270 E(1,I)=E(1,I)/2.

DO 280 I=1,19
DO 280 J=1,9
EE(I,J)=0.0
DO 280 K=1,NH
280 EE(I,J)=EE(I,J)+A(I,K)*E(K,J)

WRITE OUTPUT TAPE KOUT, 2014
WRITE OUTPUT TAPE KOUT,2010,(TT(I),(EE(I),J=1,9),I=1,19)

DETERMINE FOURIER COEFFICIENTS AS A FUNCTION OF SPACE

C

Report No. 5654-02 FS

```

XL=0.0
DO 300 I=1,9
  A(I,1)=1.0
  A(I,2)=XL
  A(I,3)=XL**2
  A(I,4)=A(I,3)*XL
  A(I,5)=A(I,4)*XL
  A(I,6)=A(I,5)*XL
  XL=XL+SQRTF((RR(I+1)-RR(I))**2+(ZZ(I+1)-ZZ(I))**2)
DO 300 J=1,NH
  EE(I,J)=E(J,I)
300

```

C

```

CALL LEAST(A,EE,B,D,E,9,6,NH)

```

C

```

RETURN

```

C

```

FORMAT STATEMENTS

```

C

```

1000 FORMAT (12A6/ 6I5,2F10.2/ 4F10.2/4F10.2)
1001 FORMAT (4F10.2)
1002 FORMAT (4F10.2)
1003 FORMAT (9F8.0)
2000 FORMAT ( 1H1 12A6/

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1 33HNUMBER OF POINTS ALONG LENGTH--- I3/
2 33H NUMBER OF POINTS THRU THICKNESS- I3/
3 33H LOCATION OF BOND LINE----- I3/
4 33H NUMBER OF PROPERTY CARDS----- I3/
5 33H NUMBER OF HARMONICS----- I4/
6 33H NUMBER OF BOUNDARY CONDITIONS--- I4/
7 33H SURFACE TEMPERATURE OF ABLATOR-- F6.0/
8 33H ZERO STRESS TEMPERATURE----- F6.0)

```

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2001 FORMAT (15H1 R-ORDINATE 5X 10HZ-ORDINATE 5X 10HBOND TEMP. 3X
1 17HABLATOR THICKNESS / (3F15.3,1F20.4))
2002 FORMAT (15H0 TEMPERATURE 5X 10HMODULUS A 5X 9HMODULUS B 1X
120H COEFF. OF EXPANSION / (3F15.0,1E20.5))

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2003 FORMAT (14H1 R-ORDINATES )

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2004 FORMAT (14H1 Z-ORDINATES )

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2005 FORMAT (14H1 TEMPERATURE )

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2006 FORMAT (37H0PROPERTIES OF SANDWICH CORE MATERIAL )

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2007 FORMAT (30H0PROPERTIES OF SANDWICH PLATES )

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2008 FORMAT

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1(28H MODULUS OF ELASTICITY----- F10.0/

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2 28H POISSONS RATIO----- F10.4/
 3 28H COEFFICIENT OF EXPANSION-- F10.8/
 4 28H THICKNESS----- F10.4)
 2009 FORMAT (5H1 R=9F12.4/5H Z=9F12.4)
 2010 FORMAT (/(F5.0,9F12.1))
 2011 FORMAT (20H1 MATERIAL PROPERTIES)
 2012 FORMAT (38H1 LEAST SQUARE EVALUATION OF ABOVE DATA)
 2013 FORMAT (36H0 ANGLE BOND LINE TEMPERATURES)
 2014 FORMAT (1H010X,47H FOURIER SERIES EXPANSION OF ABOVE TEMPERATURES)
 END

NAHS0368
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 NAHS0375
 NAHS0376
 NAHS0377

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C
SUBROUTINE STIFFT(IX,JX,KX)
    DIMENSION R(30,8),Z(30,8),T(30,8),TC(30,8),E(10,10),CC(10,10),
    1 XX(5),YY(5),S(15,15),P(15),G(9,9),B(9,9),C(9,9),LM(3),EE(9)
    COMMON KINN,KOUT,N,M,L,NMAX,MMAX,MB,NMT,NH,NB,TE,TTT,ES,XS,AS,TS,
    1 ESH,XSH,ASH,TSH,R,Z,T,TC,E,CC,XX,YY,S,P,C,B,G

C
    INITIALIZATION
    II=IX
    JJ=JX
    KK=KX
    DO 50 I=1,81
    G(I)=0.0
    50 B(I)=0.0
    AJ=XX(JJ)-XX(II)
    AK=XX(KK)-XX(II)
    BJ=YY(JJ)-YY(II)
    BK=YY(KK)-YY(II)
    AREA=(AJ*BK-BJ*AK)/2.
    RBAR=(XX(II)+XX(JJ)+XX(KK))/3.

C
    CALCULATE MATERIAL PROPERTIES
    TT=(T(N,M)+T(N,M+1)+T(N+1,M)+T(N+1,M+1))/4.
    DO 60 I=1,3
    60 EE(I)=CC(1,I)+TT*(CC(2,I)+TT*(CC(3,I)+TT*(CC(4,I)+TT*CC(5,I))))
    IF(MB-M) 70,70,80
    70 EP=EE(1)
    ALP=EE(3)
    XNU=0.3155+0.000174*TT
    GO TO 90
    80 EP=ES
    ALP=AS
    XNU=XS
    90 CONTINUE

C
    FORM MATRIX (B)
    COMM=EP/((1.+XNU)*(1.-2.*XNU))
    CX=(.5-XNU)*COMM
    DX=(1.-XNU)*COMM
    EX=XNU*COMM
    B(1,1)=DX

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 NAHS0421

B(1,2)=EX
B(1,3)=EX
B(2,1)=EX
B(2,2)=DX
B(2,3)=EX
B(3,1)=EX
B(3,2)=EX
B(3,3)=DX
B(4,4)=CX
B(5,5)=CX
B(6,6)=CX

FORM G MATRIX

D=XX(JJ)*(YY(KK)-YY(II))+XX(II)*(YY(JJ)-YY(KK))
1 +XX(KK)*(YY(II)-YY(JJ))
XN=L-1

G(1,1)=(YY(JJ)-YY(KK))/D
G(1,4)=(YY(KK)-YY(II))/D
G(1,7)=(YY(II)-YY(JJ))/D
G(2,1)=1./'(XX(II)+XX(JJ)+XX(KK))

G(2,4)=G(2,1)

G(2,7)=G(2,1)

G(2,2)=XN#G(2,1)

G(2,5)=XN#G(2,4)

G(2,8)=XN#G(2,7)

G(3,3)=(XX(KK)-XX(JJ))/D

G(3,6)=(XX(II)-XX(KK))/D

G(3,9)=(XX(JJ)-XX(II))/D

G(4,1)=-G(2,2)

G(4,4)=-G(2,5)

G(4,7)=-G(2,8)

G(4,2)=G(1,1)-G(2,1)

G(4,5)=G(1,4)-G(2,4)

G(4,8)=G(1,7)-G(2,7)

G(5,1)=G(3,3)

G(5,4)=G(3,6)

G(5,7)=G(3,9)

G(5,3)=G(1,1)

G(5,6)=G(1,4)

G(5,9)=G(1,7)

G(6,2)=G(3,3)

G(6,5)=G(3,6)

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RETURN

END

C C

Report No. 5654-02 FS

Page F14

SUBROUTINE STIFFP

DIMENSION R(30,8),Z(30,8),T(30,8),TC(30,8),E(10,10),CC(10,10),
 1 XX(5),YY(5),S(15,15),P(15),G(9,9),B(9,9),C(9,9)
 COMMON KINN,KOUT,N,M,L,NMAX,MMAX,MB,NMT,NH,NB,TE,TTT,ES,XS,AS,TS,
 1 ESH,XSH,ASH,TSH,R,Z,T,TC,E,CC,XX,YY,S,P,C,B,G

FORMATION OF G MATRIX

XN=L-1
 A=R(N+1,M)-R(N,M)
 B=Z(N+1,M)-Z(N,M)
 XL2=A**2+E**2
 XL=SQRTF(XL2)
 RBAR=R(N+1,M)+R(N,M)

G(1,1)=-A/XL2
 G(1,2)=0.0
 G(1,3)=-B/XL2
 G(1,4)=-G(1,1)
 G(1,5)=0.0
 G(1,6)=-G(1,3)
 G(2,1)=1./RBAR
 G(2,2)=XN*G(2,1)
 G(2,3)=0.0
 G(2,4)=G(2,1)
 G(2,5)=G(2,2)
 G(2,6)=0.0
 G(3,1)=-XN*A/RBAR/XL
 G(3,2)=1./XL
 G(3,3)=-XN*B/RBAR/XL
 G(3,4)=G(3,1)
 G(3,5)=-G(3,2)
 G(3,6)=G(3,3)

FORMATION OF B MATRIX

B(1,1)=ESH/(1.-XSH**2)
 B(1,2)=XSH*B(1,1)
 B(1,3)=0.0
 B(2,1)=B(1,2)
 B(2,2)=B(1,1)
 B(2,3)=0.0
 B(3,1)=0.0

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      B(3,2)=0.0
      B(3,3)=.5*ESH/(1.+XSH)
C
C
C
      FORMATION OF PLATE STIFFNESS MATRIX (G)T*(B)*(G)
      DO 150 I=1,3
      DO 150 J=1,6
      C(I,J)=0.0
      DO 150 K=1,3
      150 C(I,J)=C(I,J)+B(I,K)*G(K,J)
C
      COMM=RBAR*XL*TS/2.
      DO 165 I=1,6
      DO 165 J=1,6
      S(I,J)=0.0
      DO 160 K=1,3
      160 S(I,J)=S(I,J)+G(K,I)*C(K,J)
      165 S(I,J)=S(I,J)*COMM
C
      COMPUTE TEMPERATURE LOADS
C
C
      TEM=(TC(N,M)+TC(N+1,M))/2.
      TTT=TEM*ESH*ASH*(1.+XSH)/(1.-XSH**2)
      TEM=TTT*COMM
      DO 170 I=1,6
      170 P(I)=(G(1,I)+G(2,I))*TEM
C
      RETURN
C
      END

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 NAHS0587

SUBROUTINE STRESS

DIMENSION R(30,8),Z(30,8),T(30,8),TC(30,8),E(10,10),CC(10,10),
 1 XX(5),YY(5),S(15,15),P(15),U(30,8),V(30,8),W(30,8),KODE(30,10),
 2 C(9,9),RR(720),SIG(6,30,8),PSIG(3,30,2),B(9,9),G(9,9)
 COMMON KINN,KOUT,N,M,L,NMAX,MMAX,MB,NMT,NH,NB,TE,TTT,ES,XS,AS,TS,
 1 ESH,XSH,ASH,TSH,R,Z,T,TC,E,CC,XX,YY,S,P,C,B,G,KODE,RR,U,V,W,SIG,
 3 PSIG

SEPARATE DISPLACEMENTS FOR HARMONIC L

K=1
 DO 400 N=1,NMAX
 DO 400 M=1,MMAX
 U(N,M)=RR(K)
 V(N,M)=RR(K+1)
 W(N,M)=RR(K+2)
 400 K=K+3

CALCULATION OF STRESSES FOR HARMONIC L

NN=NMAX-1
 MM=MMAX-1
 DO 100 N=1,NN
 N=N
 DO 100 M=1,MM
 M=M
 XX(1)=R(N,M)
 XX(2)=R(N,M+1)
 XX(3)=R(N+1,M)
 XX(4)=R(N+1,M+1)
 YY(1)=Z(N,M)
 YY(2)=Z(N,M+1)
 YY(3)=Z(N+1,M)
 YY(4)=Z(N+1,M+1)

CALL STIFFT(1,4,2)

P(1)=U(N,M)
 P(2)=V(N,M)
 P(3)=W(N,M)
 P(4)=U(N+1,M+1)
 P(5)=V(N+1,M+1)
 P(6)=W(N+1,M+1)
 P(7)=U(N,M+1)

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DO 200 I=1,3
  PSIG(I,N,K)=0.0
DO 200 J=1,6
  200 PSIG(I,N,K)=PSIG(I,N,K)+C(I,J)*P(J)
  PSIG(I,N,K)=PSIG(I,N,K)-TTT
  210 PSIG(2,N,K)=PSIG(2,N,K)-TTT

WRITE TAPE 20, ((( PSIG(I,N,K),I=1,3),K=1,2),N=1,NN)

RETURN

END
  
```

Report No. 5654-02 ES

C SUBROUTINE OUTPUT
 DIMENSION U(30,8),V(30,8),W(30,8),XU(30,8),XV(30,8),XW(30,8),
 1 SIG(6,30,8),XSIG(6,30,8),PSIG(3,30,2),XPSIG(3,30,2)
 COMMON KINN,KOUT,N,M,L,NMAX,MMAX,MB,NMT,NH,NB,TE,TTI,ES,XS,AS,TS,
 1 U,V,W,XU,XV,XW,SIG,XSIG,PSIG,XPSIG
 C NN=NMAX-1
 C MM=MMAX-1
 C 50 READ INPUT TAPE KINN,1000,THETA
 C DO 110 N=1,NMAX
 DO 100 M=1,MMAX
 U(N,M)=0.0
 V(N,M)=0.0
 W(N,M)=0.0
 DO 100 I=1,6
 100 SIG(I,N,M)=0.0
 DO 110 K=1,2
 DO 110 I=1,3
 110 PSIG(I,N,K)=0.0
 C
 C REWIND 20
 DO 500 LL=1,NH
 TT=FLOATF(LL-1)*THETA*.017453289
 COSINE=COSF(TT)
 SINE=SINF(TT)
 C
 C READ TAPE 20,((XU(N,M),XW(N,M),XV(N,M),XSIG(I,N,M),I=1,6),
 1 N=1,NMAX),M=1,MMAX)
 C
 C DO 200 N=1,NMAX
 DO 200 M=1,MMAX
 U(N,M)=U(N,M)+XU(N,M)*COSINE
 W(N,M)=W(N,M)+XW(N,M)*COSINE
 V(N,M)=V(N,M)+XV(N,M)*SINE
 DO 150 I=1,3
 150 SIG(I,N,M)=SIG(I,N,M)+XSIG(I,N,M)*COSINE
 SIG(4,N,M)=SIG(4,N,M)+XSIG(4,N,M)*SINE
 SIG(5,N,M)=SIG(5,N,M)+XSIG(5,N,M)*COSINE
 200 SIG(6,N,M)=SIG(6,N,M)+XSIG(6,N,M)*SINE
 C
 C READ TAPE 20, ((XPSIG(I,N,K),I=1,3),K=1,2),N=1,NN)


```

C
DO 300 N=1,NN
DO 300 K=1,2
  PSIG(1,N,K)=PSIG(1,N,K)+XPSIG(1,N,K)*COSINE
  PSIG(2,N,K)=PSIG(2,N,K)+XPSIG(2,N,K)*COSINE
  300 PSIG(3,N,K)=PSIG(3,N,K)+XPSIG(3,N,K)*SINE
C
  500 CONTINUE
C
  OUTPUT DISPLACEMENTS AND STRESSES
C
  WRITE OUTPUT TAPE KOUT,2003, THETA
  CALL PRINTM(U,NMAX,MMAX,30)
  WRITE OUTPUT TAPE KOUT,2004, THETA
  CALL PRINTM(W,NMAX,MMAX,30)
  WRITE OUTPUT TAPE KOUT,2005, THETA
  CALL PRINTM(V,NMAX,MMAX,30)
C
  DO 600 M=1,MM
  WRITE OUTPUT TAPE KOUT, 2007, THETA
  600 WRITE OUTPUT TAPE KOUT,2000, (N,M,(SIG(I,N,M),I=1,6),N=1,NN)
C
  WRITE OUTPUT TAPE KOUT, 2008, THETA
  WRITE OUTPUT TAPE KOUT,2006,(N,((PSIG(I,N,K),I=1,3),K=1,2),N=1,NN)
C
  GO TO 50
C
  1000 FORMAT (F5.0)
  2000 FORMAT (12H0 N M 10X,2HRR 10X,2HTT 10X,2HZZ 10X,2HRT
    1 10X,2HRZ 10X,2HTZ / (2I6,6F12.2))
  2003 FORMAT (20H1R-DISPLACEMENTS AT F4.0,8H DEGREES)
  2004 FORMAT (20H1Z-DISPLACEMENTS AT F4.0,8H DEGREES)
  2005 FORMAT (20H1T-DISPLACEMENTS AT F4.0,8H DEGREES)
  2006 FORMAT (15,6F18.1)
  2007 FORMAT (28H1AVERAGE ELEMENT STRESSES AT F5.0,8H DEGREES)
  2008 FORMAT (44H1AVERAGE STRESSES IN SANDWICH FACE PLATES ATF5.0,8H DEGN
    1 REES//22X, 12HBOTTOM PLATE 44X, 9HTOP PLATE /5H N 2(54H MERN
    1 1D. STRESS HOOP STRESS IN-PLANE SHEAR ))
C
  END

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SUBROUTINE INVERT(A,NN,N,M,C)
      GENERAL MATRIX INVERSION SUBROUTINE

      DIMENSION A(1),M(1),C(1)

      CHECK FOR 1 X 1 MATRIX

      IF(NN-1) 300,70,80
70  A(1)=1./A(1)
      GO TO 300

      C

      80 DO 90 I=1,NN
      90 M(I)=-I

      C

      DO 140 I=1,NN

      LOCATE LARGEST ELEMENT

      D=0.0
      DO 112 L=1,NN
      IF (M(L)) 100,100,112
      100 J=L

      DO 110 K=1,NN
      IF (M(K)) 103,103,108
      103 IF (ABSF(D)-ABSF(A(J))) 105,105,108
      105 LD=L
      KD=K
      D=A(J)

      108 J=J+N
      110 CONTINUE
      112 CONTINUE

      C

      INTERCHANGE ROWS

      C

      TEMP=-M(LD)
      M(LD)=M(KD)
      M(KD)=TEMP
      L=LD
      K=KD

      DO 114 J=1,NN
      C(J)=A(L)
      A(L)=A(K)
      A(K)=C(J)
  
```

NAHS0855
 NAHS0856
 NAHS0857
 NAHS0858
 NAHS0859
 NAHS0860
 NAHS0861
 NAHS0862
 NAHS0863
 NAHS0864
 NAHS0865
 NAHS0866
 NAHS0867
 NAHS0868
 NAHS0869
 NAHS0870
 NAHS0871
 NAHS0872
 NAHS0873
 NAHS0874
 NAHS0875
 NAHS0876
 NAHS0877
 NAHS0878
 NAHS0879
 NAHS0880
 NAHS0881
 NAHS0882
 NAHS0883
 NAHS0884
 NAHS0885
 NAHS0886
 NAHS0887
 NAHS0888
 NAHS0889
 NAHS0890
 NAHS0891
 NAHS0892
 NAHS0893
 NAHS0894
 NAHS0895
 NAHS0896
 NAHS0897
 NAHS0898

```

L=L+N
114 K=K+N
      C
      C
      C
      DIVIDE COLUMN BY LARGEST ELEMENT
      NR=(KD-1)*N+1
      NH=NR+N-1
      DO 115 K=NR,NH
115  A(K)=A(K)/D
      C
      C
      C
      REDUCE REMAINING ROWS AND COLUMNS
      L=1
      DO 135 J=1,NN
      IF (J-KD) 130,125,130
125  L=L+N
      GO TO 135
130  DO 134 K=NR,NH
      A(L)=A(L)-C(J)*A(K)
134  L=L+1
135  CONTINUE
      C
      C
      C
      REDUCE ROW
      C(KD)=-1.0
      J=KD
      DO 140 K=1,NN
      A(J)=-C(K)/D
140  J=J+N
      C
      C
      C
      INTERCHANGE COLUMNS
      DO 200 I=1,NN
      L=0
150  L=L+1
      IF(M(L)-I) 150,160,150
160  K=(L-1)*N+1
      J=(I-1)*N+1
      M(L)=M(I)
      M(I)=I
      DO 200 L=1,NN
      TEMP=A(K)
      A(K)=A(J)
      A(J)=TEMP
  
```

NAHS0899
NAHS0900
NAHS0901
NAHS0902
NAHS0903
NAHS0904

J=J+1
200 K=K+1
C
300 RETURN
C
END

Report No. 5654-02 FS

Page F25

```

C      SUBROUTINE SYMSOL (A,B,NN,MM)
C
C      DIMENSION A(720,30),B(720),C(30)
C
C      N = 0
C      100 N = N+1
C
C      REDUCE N TH EQUATION
C
C      1. DIVIDE RIGHT SIDE BY DIAGONAL ELEMENT
C
C      B(N) = B(N) / A(N,1)
C
C      2. CHECK FOR LAST EQUATION
C
C      IF(N-NN) 150,300,150
C
C      3. DIVIDE N TH EQUATION BY DIAGONAL ELEMENT
C
C      150 DO 200 K=2,MM
C      C(K) = A(N,K)
C      200 A(N,K) = A(N,K) / A(N,1)
C
C      4. REDUCE REMAINING EQUATIONS
C
C      DO 260 L=2,MM
C      I = N+L-1
C      IF(NN-I) 260,240,240
C      240 J=0
C      DO 250 K=L,MM
C      J=J+1
C      250 A(I,J) = A(I,J) - C(L) * A(N,K)
C      B(I) = B(I) - C(L) * B(N)
C      260 CONTINUE
C      GO TO 100
C
C      BACK SUBSTITUTION
C
C      300 N = N-1
C
C      1. CHECK FOR FIRST EQUATION
C
C      IF(N) 350,500,350
C
C

```

```

C      2. CALCULATE UNKNOWN B(N)
C
350 DO 400 K=2,MM
    L = N+K-1
    IF(NN-L) 400,370,370
370 B(N) = B(N) - A(N,K) * B(L)
400 CONTINUE
    GO TO 300
C
500 RETURN
C
    END

```

```

NAHS0949
NAHS0950
NAHS0951
NAHS0952
NAHS0953
NAHS0954
NAHS0955
NAHS0956
NAHS0957
NAHS0958
NAHS0959
NAHS0960

```

```

SUBROUTINE PRINTM (A,NR,NC,MAXR)
DIMENSION A(1),NHED(10)
COMMON KINN,KOUT

C
DO 50 I=1,NC,10
  II=NC-I+1
  IF (II-10) 20,20,10
    10 II=10
    20 DO 30 J=1,II
      30 NHED(J)=I+J-1
C
  WRITE OUTPUT TAPE KOUT,120,(NHED(J),J=1,II)
C
DO 50 J=1,NR
  KL=J+(I-1)*MAXR
  KH=KL+(II-1)*MAXR
  50 WRITE OUTPUT TAPE KOUT,130,(J,(A(K),K=KL,KH,MAXR))
C
  RETURN
C
  120 FORMAT (8H0 N/M 10I11)
  130 FORMAT (15.3X,10F11.3)
  END

```

NAHS0961
 NAHS0962
 NAHS0963
 NAHS0964
 NAHS0965
 NAHS0966
 NAHS0967
 NAHS0968
 NAHS0969
 NAHS0970
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 NAHS0972
 NAHS0973
 NAHS0974
 NAHS0975
 NAHS0976
 NAHS0977
 NAHS0978
 NAHS0979
 NAHS0980
 NAHS0981
 NAHS0982
 NAHS0983

APPENDIX G

SUMMARY OF EFFORTS TO SOLVE THE THERMAL STRAIN PROBLEM BY THE OVER-RELAXATION AND DIRECT INTEGRATION OF FINITE-DIFFERENCE FORMULATION

I. ACCOMPLISHMENTS

During the initial phases of this contract, finite-difference techniques were employed in an effort to solve the governing differential equations for this problem. The following work was accomplished:

- A. Derivation of basic equations suitable to the given geometry
- B. Derivation of the finite-differences model of the equations
- C. Development of a thin-shell model for the thin layers (bond, face-plates) and its finite-differences equivalent
- D. Attempt to establish proper boundary conditions for the finite-difference model
- E. Programing of the finite-difference model(s) and attempts to solve the equations by (1) the over-relaxation approach (in several versions) and (2) direct matrix, inversions.
- F. Investigation of alternative methods and recommendations.

A summary of the results in each of the above areas is presented in this appendix.

II. DERIVATION OF BASIC EQUATIONS SUITABLE TO GIVEN GEOMETRY

A complete set of equilibrium equations in terms of displacements in spherical and toroidal coordinates was derived and is given below. A singular point exists at $\phi = 0$. The equations for this point are also presented.

For orthogonal curvilinear coordinates $(\alpha_1, \alpha_2, \alpha_3)$, the element of arc ds is defined by

$$ds^2 = \sum_{i=1}^3 g_{ii} d\alpha_i^2 \quad (G1)$$

where g_{ii} represents the metric coefficients

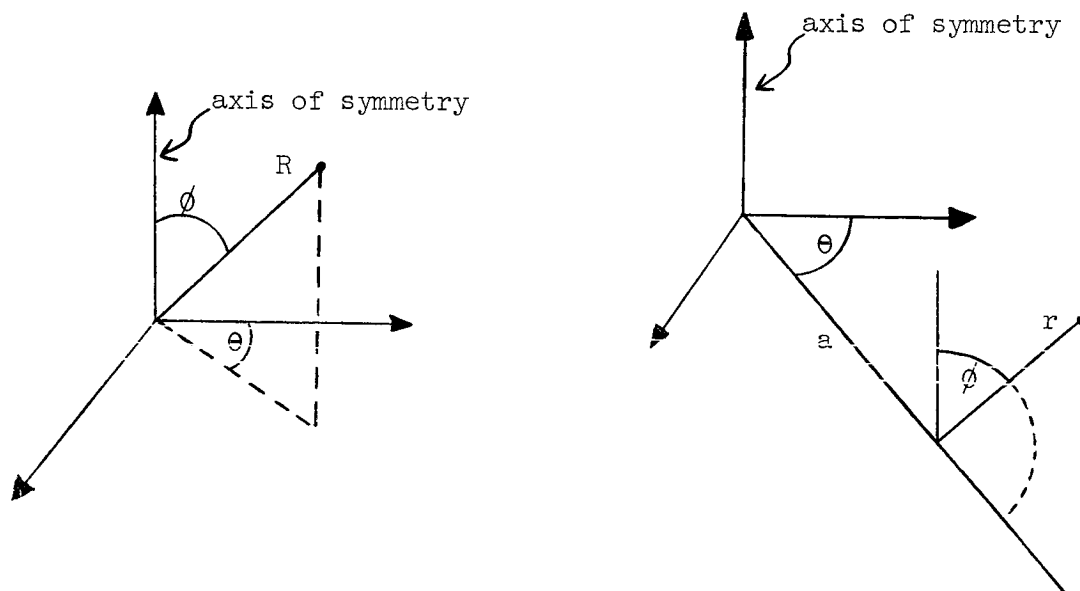


Fig. G1 - Coordinate Axes

	<u>Spherical Coordinates</u>	<u>Toroidal Coordinates</u>
α_1	R	r
α_2	ϕ	ϕ
α_3	θ	θ
g_{11}	1	1
g_{22}	R^2	r^2
g_{33}	$R^2 \sin^2 \phi$	$(a + r \sin \phi)^2$
$g \equiv \sqrt{g_{11}g_{22}g_{33}}$	$R^2 \sin \phi$	$r(a + r \sin \phi)$

Note: Toroidal coordinates reduce to spherical coordinates in the limit as $a \rightarrow 0$.

Equations of equilibrium with zero body force take the form shown below.

$$\sum_{j=1}^3 \left[\frac{\partial}{\partial \alpha_j} \left(\frac{g_{ii} \tau_{ij}}{\sqrt{g_{ii} g_{jj}}} \right) - \frac{1}{2} \cdot \frac{g_{jj}}{g_{jj}} \frac{\partial g_{jj}}{\partial \alpha_i} \right] = 0 \quad (G2)$$

where $g \equiv \sqrt{g_{11} g_{22} g_{33}}$ and τ_{ii} and τ_{ij} are normal and shear components of stress, respectively.

After substituting the respective components of α_i and g_{ii} in Equation (G2) and performing the indicated differentiations and summations, there are obtained the following equilibrium equations in terms of stresses for each coordinate system:

A. SPHERICAL COORDINATES

$$\frac{\partial \tau_{RR}}{\partial R} + \frac{1}{R} \frac{\partial \tau_{R\theta}}{\partial \theta} + \frac{1}{R \sin \theta} \frac{\partial \tau_{R\phi}}{\partial \phi} + \frac{2\tau_{RR} - \tau_{\theta\theta} - \tau_{\phi\phi} + \tau_{R\theta} \cot \theta}{R} = 0 \quad (G3)$$

$$\frac{\partial \tau_{R\theta}}{\partial R} + \frac{1}{R} \frac{\partial \tau_{\theta\theta}}{\partial \theta} + \frac{1}{R \sin \theta} \frac{\partial \tau_{\theta\phi}}{\partial \phi} + \frac{3\tau_{R\theta} + (\tau_{\theta\theta} - \tau_{\phi\phi}) \cot \theta}{R} = 0 \quad (G4)$$

$$\frac{\partial \tau_{R\phi}}{\partial R} + \frac{1}{R} \frac{\partial \tau_{\phi\phi}}{\partial \phi} + \frac{1}{R \sin \theta} \frac{\partial \tau_{\phi\theta}}{\partial \theta} + \frac{3\tau_{R\phi} + 2\tau_{\phi\theta} \cot \theta}{R} = 0 \quad (G5)$$

B. TOROIDAL COORDINATES

$$\begin{aligned} \frac{\partial \tau_{rr}}{\partial r} + \frac{1}{r} \frac{\partial \tau_{r\theta}}{\partial \theta} + \frac{1}{(a + r \sin \theta)} \frac{\partial \tau_{r\phi}}{\partial \phi} \\ + \frac{(a + 2r \sin \theta) \tau_{rr} - (a + r \sin \theta) \tau_{\theta\theta} - \tau_{\phi\phi} r \sin \theta + \tau_{r\theta} r \cos \theta}{r(a + r \sin \theta)} = 0 \end{aligned} \quad (G6)$$

$$\begin{aligned} \frac{\partial \tau_{r\theta}}{\partial r} + \frac{1}{r} \frac{\partial \tau_{\theta\theta}}{\partial \theta} + \frac{1}{a + r \sin \theta} \frac{\partial \tau_{\theta\phi}}{\partial \phi} \\ + \frac{(2a + 3r \sin \theta) \tau_{r\theta} + (\tau_{\theta\theta} - \tau_{\phi\phi}) r \cos \theta}{r(a + r \sin \theta)} = 0 \end{aligned} \quad (G7)$$

$$\frac{\partial \tau_{r\theta}}{\partial r} + \frac{1}{r} \frac{\partial \tau_{\phi\theta}}{\partial \phi} + \frac{1}{(a + r \sin \phi)} \frac{\partial \tau_{\theta\theta}}{\partial \theta} + \frac{(a + 3r \sin \phi) \tau_{r\theta} + 2 \tau_{\phi\theta} r \cos \phi}{r(a + r \sin \phi)} = 0 \quad (G8)$$

C. HOOKE'S LAW INCLUDING TEMPERATURE TERMS

$$\tau_{ii} = \lambda \theta + 2\mu e_{ii} - (3\lambda + 2\mu) \int_{T_0}^T \alpha(T) dT \quad (G9)$$

$$\tau_{ij} = 2\mu e_{ij} \quad (G10)$$

where

$$\theta = e_{11} + e_{22} + e_{33}$$

and λ and μ are the Lamé constants defined in terms of Poisson's ratio ν and Young's modulus E according to

$$\left. \begin{aligned} \lambda &= \frac{\nu E}{(1 + \nu)(1 - 2\nu)} \\ \mu &= \frac{E}{2(1 + \nu)} \end{aligned} \right\} \quad (G11)$$

D. STRAIN - DISPLACEMENT RELATIONS

$$e_{ii} = \frac{\partial}{\partial \alpha_i} \frac{u_i}{\sqrt{g_{ii}}} + \frac{1}{2g_{ii}} \sum_{k=1}^3 \frac{\partial g_{ii}}{\partial \alpha_k} \frac{u_k}{\sqrt{g_{kk}}} \quad (G12)$$

$$e_{ij} = \frac{1}{2\sqrt{g_{ii}g_{jj}}} \left[g_{ii} \frac{\partial}{\partial \alpha_j} \left(\frac{u_i}{\sqrt{g_{ii}}} \right) + g_{jj} \frac{\partial}{\partial \alpha_i} \left(\frac{u_j}{\sqrt{g_{jj}}} \right) \right], \quad i \neq j \quad (G13)$$

Let u , v , and w be components of displacement in the three principal directions r or R , ϕ and θ . Then substitution of these displacements in Equations (G12) and (G13), with the metric coefficients of Page G2, yields the strain-displacement relations for the coordinate systems shown in Sections II,E and II,F.

E. SPHERICAL COORDINATES

$$\left. \begin{aligned}
 e_{RR} &= \frac{\partial u}{\partial R} \\
 e_{\phi\phi} &= \frac{1}{R} \frac{\partial v}{\partial \phi} + \frac{u}{R} \\
 e_{\theta\theta} &= \frac{1}{R \sin \phi} \frac{\partial w}{\partial \theta} + \frac{u}{R} + \frac{v \cot \phi}{R} \\
 e_{R\phi} &= \frac{1}{2} \left(\frac{1}{R} \frac{\partial u}{\partial \phi} - \frac{v}{R} + \frac{\partial v}{\partial R} \right) \\
 e_{\phi\theta} &= \frac{1}{2} \left(\frac{1}{R} \frac{\partial w}{\partial \phi} - \frac{w \cot \phi}{R} + \frac{1}{R \sin \phi} \frac{\partial v}{\partial \theta} \right) \\
 e_{R\theta} &= \frac{1}{2} \left(\frac{1}{R \sin \phi} \frac{\partial u}{\partial \theta} - \frac{w}{R} + \frac{\partial w}{\partial R} \right)
 \end{aligned} \right\} \quad (G14)$$

F. TOROIDAL COORDINATES

$$\begin{aligned}
 e_{rr} &= \frac{\partial u}{\partial r} \\
 e_{\phi\phi} &= \frac{1}{r} \frac{\partial v}{\partial \phi} + \frac{u}{r} \\
 e_{\theta\theta} &= \frac{1}{a + r \sin \phi} \frac{\partial w}{\partial \theta} + \frac{u \sin \phi}{a + r \sin \phi} + \frac{v \cos \phi}{a + r \sin \phi} \\
 e_{r\phi} &= \frac{1}{2} \left(\frac{1}{r} \frac{\partial u}{\partial \phi} + \frac{\partial v}{\partial r} - \frac{v}{r} \right)
 \end{aligned} \quad (G15)$$

$$e_{\phi\theta} = \frac{1}{2} \left(\frac{1}{a + r \sin \phi} \frac{\partial v}{\partial \theta} + \frac{1}{r} \frac{\partial w}{\partial \phi} - \frac{w \cos \phi}{a + r \sin \phi} \right) \quad (G15)$$

$$e_{r\theta} = \frac{1}{2} \left(\frac{1}{a + r \sin \phi} \frac{\partial u}{\partial \theta} + \frac{\partial w}{\partial r} - \frac{w \sin \phi}{a + r \sin \phi} \right)$$

G. EQUILIBRIUM EQUATIONS IN TERMS OF DISPLACEMENTS

Expressing the stresses in terms of displacement using Hooke's law [Equations (G9) and (G10)] with the strain-displacement relations [Equations (G14) and (G15)], the equilibrium equations [Equations (G3) through (G8)] may be written in terms of displacements in the form

$$\begin{aligned} & A_k \frac{\partial^2 u}{\partial \alpha_1^2} + B_k \frac{\partial^2 u}{\partial \alpha_2^2} + C_k \frac{\partial^2 u}{\partial \alpha_3^2} + D_k \frac{\partial^2 u}{\partial \alpha_1 \partial \alpha_2} + E_k \frac{\partial^2 u}{\partial \alpha_2 \partial \alpha_3} \\ & + F_k \frac{\partial^2 u}{\partial \alpha_1 \partial \alpha_3} + G_k \frac{\partial u}{\partial \alpha_1} + H_k \frac{\partial u}{\partial \alpha_2} + I_k \frac{\partial u}{\partial \alpha_3} + J_k u \\ & + \bar{A}_k \frac{\partial^2 v}{\partial \alpha_1^2} + \bar{B}_k \frac{\partial^2 v}{\partial \alpha_2^2} + \bar{C}_k \frac{\partial^2 v}{\partial \alpha_3^2} + \bar{D}_k \frac{\partial^2 v}{\partial \alpha_1 \partial \alpha_2} + \bar{E}_k \frac{\partial^2 v}{\partial \alpha_2 \partial \alpha_3} \\ & + \bar{F}_k \frac{\partial^2 v}{\partial \alpha_1 \partial \alpha_3} + \bar{G}_k \frac{\partial v}{\partial \alpha_1} + \bar{H}_k \frac{\partial v}{\partial \alpha_2} + \bar{I}_k \frac{\partial v}{\partial \alpha_3} + \bar{J}_k v \\ & + \bar{\bar{A}}_k \frac{\partial^2 w}{\partial \alpha_1^2} + \bar{\bar{B}}_k \frac{\partial^2 w}{\partial \alpha_2^2} + \bar{\bar{C}}_k \frac{\partial^2 w}{\partial \alpha_3^2} + \bar{\bar{D}}_k \frac{\partial^2 w}{\partial \alpha_1 \partial \alpha_2} + \bar{\bar{E}}_k \frac{\partial^2 w}{\partial \alpha_2 \partial \alpha_3} \\ & + \bar{\bar{F}}_k \frac{\partial^2 w}{\partial \alpha_1 \partial \alpha_3} + \bar{\bar{G}}_k \frac{\partial w}{\partial \alpha_1} + \bar{\bar{H}}_k \frac{\partial w}{\partial \alpha_2} + \bar{\bar{I}}_k \frac{\partial w}{\partial \alpha_3} + \bar{\bar{J}}_k w \\ & = \frac{(3\lambda + 2\mu) \alpha(T)}{\sqrt{g_{kk}}} \frac{\partial T}{\partial \alpha_k}, \quad k = 1, 2, 3 \end{aligned} \quad (G16)$$

TABLE G1
COEFFICIENTS OF EQUILIBRIUM EQUATIONS
SPHERICAL COORDINATES
($\phi \neq 0$)

	k = 1	k = 2	k = 3
A_k	$\lambda + 2\mu$	0	0
B_k	μ/R^2	0	0
C_k	$\mu/(R^2 \sin^2 \phi)$	0	0
D_k	0	$(\lambda + \mu)/R$	0
E_k	0	0	0
F_k	0	0	$(\lambda + \mu)/(R \sin \phi)$
G_k	$2(\lambda + 2\mu)/R$	0	0
H_k	$\mu \cot \phi/R^2$	$2(\lambda + 2\mu)/R^2$	0
I_k	0	0	$2(\lambda + 2\mu)/(R^2 \sin \phi)$
J_k	$-2(\lambda + 2\mu)/R^2$	0	0
\bar{A}_k	0	μ	0
\bar{B}_k	0	$(\lambda + 2\mu)/R^2$	0
\bar{C}_k	0	$\mu/(R^2 \sin^2 \phi)$	0
\bar{D}_k	$(\lambda + \mu)/R$	0	0
\bar{E}_k	0	0	$(\lambda + \mu)/(R^2 \sin \phi)$
\bar{F}_k	0	0	0
\bar{G}_k	$(\lambda + \mu) \cot \phi/R$	$2\mu/R$	0
\bar{H}_k	$-(\lambda + 3\mu)/R^2$	$(\lambda + 2\mu) \cot \phi/R^2$	0
\bar{I}_k	0	0	$(\lambda + 3\mu) \cot \phi/(R^2 \sin \phi)$
\bar{J}_k	$-(\lambda + 3\mu) \cot \phi/R^2$	$-(\lambda + 2\mu)/(R^2 \sin^2 \phi)$	0
$\bar{\bar{A}}_k$	0	0	μ
$\bar{\bar{B}}_k$	0	0	μ/R^2

TABLE G1 (cont.)

COEFFICIENTS OF EQUILIBRIUM EQUATIONS

SPHERICAL COORDINATES

$(\phi \neq 0)$

	k = 1	k = 2	k = 3
\bar{C}_k	0	0	$(\lambda + 2\mu)/(R^2 \sin^2 \phi)$
\bar{D}_k	0	0	0
\bar{E}_k	0	$(\lambda + \mu)/(R^2 \sin \phi)$	0
\bar{F}_k	$(\lambda + \mu)/(R \sin \phi)$	0	0
\bar{G}_k	0	0	$2\mu/R$
\bar{H}_k	0	0	$\mu \cot \phi / R^2$
\bar{I}_k	$-(\lambda + 3\mu)/(R^2 \sin \phi)$	$-(\lambda + 3\mu) \cot \phi / (R^2 \sin \phi)$	0
\bar{J}_k	0	0	$-\mu/(R^2 \sin^2 \phi)$

TABLE G2

COEFFICIENTS OF EQUILIBRIUM EQUATIONS

TOROIDAL COORDINATES

	k = 1	k = 2	k = 3
A_k	$\lambda + 2\mu$	0	0
B_k	μ/r^2	0	0
C_k	$\mu/(a+r\sin\phi)^2$	0	0
D_k	0	$(\lambda + \mu)/r$	0
E_k	0	0	0
F_k	0	0	$(\lambda + \mu)/(a+r\sin\phi)$
G_k	$(\lambda + 2\mu)(a + 2r\sin\phi)/[r(a+r\sin\phi)]$	0	0
H_k	$\mu \cos \phi / [r(a+r\sin\phi)]$	$\frac{2(\lambda + 2\mu)r\sin\phi + (\lambda + 3\mu)a}{r^2(a+r\sin\phi)}$	0

TABLE G2 (cont.)

COEFFICIENTS OF EQUILIBRIUM EQUATIONS
TOROIDAL COORDINATES

	k = 1	k = 2	k = 3
I_k	0	0	$\frac{2(\lambda+2\mu)r\sin\phi+(\lambda+\mu)a}{r(a+r\sin\phi)^2}$
J_k	$-(\lambda+2\mu)\left[1/r^2+\sin^2\phi/(a+r\sin\phi)^2\right]$	$(\lambda+2\mu)a\cos\phi/[r(a+r\sin\phi)^2]$	0
\bar{A}_k	0	μ	0
\bar{B}_k	0	$(\lambda+2\mu)/r^2$	0
\bar{C}_k	0	$\mu/(a+r\sin\phi)^2$	0
\bar{D}_k	$(\lambda+\mu)/r$	0	0
\bar{E}_k	0	0	$(\lambda+\mu)/[r(a+r\sin\phi)]$
\bar{F}_k	0	0	0
\bar{G}_k	$(\lambda+\mu)\cos\phi/(a+r\sin\phi)$	$\mu(a+2r\sin\phi)/[r(a+r\sin\phi)]$	0
\bar{H}_k	$-(\lambda+3\mu)/r^2$	$(\lambda+2\mu)\cos\phi/[r(a+r\sin\phi)]$	0
\bar{I}_k	0	0	$(\lambda+3\mu)\cos\phi/(a+r\sin\phi)^2$
\bar{J}_k	$-\frac{(\lambda+3\mu)r\sin\phi\cos\phi+\mu a\cos\phi}{r(a+r\sin\phi)^2}$	$-\frac{(\lambda+2\mu)r^2+\mu a^2+(\lambda+3\mu)a r\sin\phi}{r^2(a+r\sin\phi)^2}$	0
$\bar{\bar{A}}_k$	0	0	μ
$\bar{\bar{B}}_k$	0	0	μ/r^2
$\bar{\bar{C}}_k$	0	0	$(\lambda+2\mu)/(a+r\sin\phi)^2$
$\bar{\bar{D}}_k$	0	0	0
$\bar{\bar{E}}_k$	0	$(\lambda+\mu)/[r(a+r\sin\phi)]$	0
$\bar{\bar{F}}_k$	$(\lambda+\mu)/(a+r\sin\phi)$	0	0
$\bar{\bar{G}}_k$	0	0	$\mu(a+2r\sin\phi)/[r(a+r\sin\phi)]$
$\bar{\bar{H}}_k$	0	0	$\mu\cos\phi/[r(a+r\sin\phi)]$
$\bar{\bar{I}}_k$	$-(\lambda+3\mu)\sin\phi/(a+r\sin\phi)^2$	$-(\lambda+3\mu)\cos\phi/(a+r\sin\phi)^2$	0
$\bar{\bar{J}}_k$	0	0	$-\mu/(a+r\sin\phi)^2$

TABLE G3

COEFFICIENTS OF EQUILIBRIUM EQUATIONS
POLAR COORDINATES [FOR APPROXIMATE TWO-DIMENSIONAL LOCAL
SOLUTION FOR ANY ($\theta = \text{constant}$) CROSS-SECTIONAL PLANE]

	k = 1	k = 2
A_k	$\lambda + 2\mu$	0
B_k	μ/R^2	0
D_k	0	$(\lambda + \mu)/R$
G_k	$(\lambda + 2\mu)/R + \frac{\partial}{\partial T} (\lambda + 2\mu) \frac{\partial T}{\partial R}$	$\frac{1}{R} \left(\frac{\partial \lambda}{\partial T} \right) \left(\frac{\partial T}{\partial \phi} \right)$
H_k	$\frac{1}{R^2} \left(\frac{\partial \mu}{\partial T} \right) \left(\frac{\partial T}{\partial \phi} \right)$	$\frac{1}{R} \left(\frac{\partial \mu}{\partial T} \right) \left(\frac{\partial T}{\partial R} \right) + (\lambda + 3\mu)/R^2$
J_k	$\frac{1}{R} \left(\frac{\partial \lambda}{\partial T} \right) \left(\frac{\partial T}{\partial R} \right) - (\lambda + 2\mu)/R^2$	$\left(\frac{1}{R^2} \right) \left(\frac{\partial T}{\partial T} \right) (\lambda + 2\mu) \frac{\partial T}{\partial \phi}$
\bar{A}_k	0	μ
\bar{B}_k	0	$(\lambda + 2\mu)/R^2$
\bar{D}_k	$(\lambda + \mu)/R$	0
\bar{G}_k	$\frac{1}{R} \left(\frac{\partial \mu}{\partial T} \right) \left(\frac{\partial T}{\partial \phi} \right)$	$\left(\frac{\partial \mu}{\partial T} \right) \left(\frac{\partial T}{\partial R} \right) + \mu/R$
\bar{H}_k	$\frac{1}{R} \left(\frac{\partial \lambda}{\partial T} \right) \left(\frac{\partial T}{\partial R} \right) - \frac{(\lambda + 3\mu)}{R^2}$	$\left(\frac{1}{R^2} \right) \left(\frac{\partial T}{\partial T} \right) (\lambda + 2\mu) \frac{\partial T}{\partial \phi}$
\bar{J}_k	$-\frac{1}{R^2} \left(\frac{\partial \mu}{\partial T} \right) \left(\frac{\partial T}{\partial \phi} \right)$	$-\frac{1}{R} \left(\frac{\partial \mu}{\partial T} \right) \left(\frac{\partial T}{\partial R} \right) - \mu/R^2$

Note: Temperature-dependent material property derivative terms are also included. Only applicable coefficients are listed. By replacing R with r, the above coefficients are applicable in the torus cross-section region.

H. EQUATIONS FOR STRESSES IN TERMS OF DISPLACEMENTS

From Hooke's law, Equations (G9) and (G10)

$$\tau_{ij} = 2\mu e_{ij} + \delta_{ij} \left[\lambda \theta - (3\lambda + 2\mu) \int_{T_0}^T \alpha(T) dT \right] \quad (G17)$$

where δ_{ij} is the Kronecker delta defined by

$$\begin{aligned} \delta_{ij} &= 1, i = j \\ &= 0, i \neq j \end{aligned}$$

and

$$\theta \equiv e_{11} + e_{22} + e_{33}$$

Writing the strains in terms of displacements from either Equation (G14) or (G15) and shortening the nomenclature by defining the stresses

$$\tau_1 = \tau_{rr} \text{ or } \tau_{RR}$$

$$\tau_2 = \tau_{\phi\phi}$$

$$\tau_3 = \tau_{\theta\theta}$$

$$\tau_4 = \tau_{r\phi} \text{ or } \tau_{R\phi}$$

$$\tau_5 = \tau_{\phi\theta}$$

$$\tau_6 = \tau_{r\theta} \text{ or } \tau_{R\theta}$$

Equation (G17) may be written in terms of displacements according to

$$\begin{aligned} \tau_l + \Delta_l (3\lambda + 2\mu) \int_{T_0}^T \alpha(T) dT &= \alpha_l^u r + \beta_l^u \phi + \gamma_l^u \theta + \delta_l^u \\ &+ \bar{\alpha}_l^v r + \bar{\beta}_l^v \phi + \bar{\gamma}_l^v \theta + \bar{\delta}_l^v \\ &+ \bar{\bar{\alpha}}_l^w r + \bar{\bar{\beta}}_l^w \phi + \bar{\bar{\gamma}}_l^w \theta + \bar{\bar{\delta}}_l^w \end{aligned} \quad (G18)$$

where

$$\Delta_l = 1 \quad \text{if } l = 1, 2, 3$$

$$= 0 \quad \text{if } l = 4, 5, 6$$

TABLE G4
COEFFICIENTS OF STRESS EQUATIONS
SPHERICAL COORDINATES
($\phi \neq 0$)

l	1	2	3	4	5	6
α_l	$\lambda + 2\mu$	λ	λ	0	0	0
β_l	0	0	0	μ/R	0	0
γ_l	0	0	0	0	0	$\mu/(R \sin \phi)$
δ_l	$2\lambda/R$	$2(\lambda + \mu)/R$	$2(\lambda + \mu)/R$	0	0	0
$\bar{\alpha}_l$	0	0	0	μ	0	0
$\bar{\beta}_l$	λ/R	$(\lambda + 2\mu)/R$	λ/R	0	0	0
$\bar{\gamma}_l$	0	0	0	0	$\mu/(R \sin \phi)$	0
$\bar{\delta}_l$	$\lambda \cot \phi/R$	$\lambda \cot \phi/R$	$(\lambda + 2\mu) \cot \phi/R$	$-\mu/R$	0	0
$\bar{\bar{\alpha}}_l$	0	0	0	0	0	μ
$\bar{\bar{\beta}}_l$	0	0	0	0	μ/R	0
$\bar{\bar{\gamma}}_l$	$\lambda/(R \sin \phi)$	$\lambda/(R \sin \phi)$	$(\lambda + 2\mu)/(R \sin \phi)$	0	0	0
$\bar{\bar{\delta}}_l$	0	0	0	0	$-\mu \cot \phi/R$	$-\mu/R$

TABLE G5
COEFFICIENTS OF STRESS EQUATIONS
TOROIDAL COORDINATES

l	1	2	3	4	5	6
α_l	$\lambda + 2\mu$	λ	λ	0	0	0
β_l	0	0	0	μ/R	0	0
γ_l	0	0	0	0	0	$\mu/(a+r\sin\phi)$
δ_l	$\frac{\lambda(a+2r\sin\phi)}{r(a+r\sin\phi)}$	$\frac{\lambda+2\mu}{r} + \frac{\lambda\sin\phi}{a+r\sin\phi}$	$\frac{\lambda}{r} + \frac{(\lambda+2\mu)\sin\phi}{a+r\sin\phi}$	0	0	0
$\bar{\alpha}_l$	0	0	0	μ	0	0
$\bar{\beta}_l$	λ/r	$(\lambda+2\mu)/r$	λ/r	0	0	0
$\bar{\gamma}_l$	0	0	0	0	$\mu/(a+r\sin\phi)$	0
$\bar{\delta}_l$	$\frac{\lambda\cos\phi}{a+r\sin\phi}$	$\frac{\lambda\cos\phi}{a+r\sin\phi}$	$\frac{(\lambda+2\mu)\cos\phi}{a+r\sin\phi}$	$-\mu/r$	0	0
$\bar{\bar{\alpha}}_l$	0	0	0	0	0	μ
$\bar{\bar{\beta}}_l$	0	0	0	0	μ/r	0
$\bar{\bar{\gamma}}_l$	$\lambda/(a+r\sin\phi)$	$\frac{\lambda}{a+r\sin\phi}$	$\frac{\lambda+2\mu}{a+r\sin\phi}$	0	0	0
$\bar{\bar{\delta}}_l$	0	0	0	0	$-\frac{\mu\cos\phi}{a+r\sin\phi}$	$-\frac{\mu\sin\phi}{a+r\sin\phi}$

TABLE G6
COEFFICIENTS OF EQUILIBRIUM EQUATIONS
POLAR COORDINATES [FOR APPROXIMATE TWO-DIMENSIONAL LOCAL
SOLUTION FOR ANY ($\theta = \text{constant}$) CROSS-SECTIONAL PLANE]

	1	2	4
α_l	$\lambda + 2\mu$	λ	0
β_l	0	0	μ/R
δ_l	λ/R	$(\lambda + 2\mu)/R$	0
$\bar{\alpha}_l$	0	0	μ
$\bar{\beta}_l$	λ/R	$(\lambda + 2\mu)/R$	0
$\bar{\delta}_l$	0	0	$-\mu/R$

Note: Only applicable values of l and coefficients are listed.
By replacing R with r , the above coefficients are applicable in
the torus cross-section region.

I. EQUATIONS AT THE AXIS OF SYMMETRY

Certain of the coefficients in the displacement equilibrium and stress equations become singular at the axis of symmetry ($\phi = 0$). For the nonaxially symmetric case the axis of symmetry has no special physical significance and this point can be avoided. For the axially symmetric case, however, the axis of symmetry is generally quite important and the singular coefficients may be evaluated by the use of L'Hôpital's rule. For example, the coefficient H_1 in the displacement equilibrium equations in spherical coordinates is $\mu \cot \phi / R^2$ which becomes infinite as ϕ approaches zero. From Equation (G16), this term multiplies the displacement component $\frac{\partial u}{\partial \phi}$. The conditions for axial symmetry are

$$w(R, \phi, \theta) = \frac{\partial f}{\partial \theta} = 0 \quad (G19)$$

where f is any function of R, ϕ, θ . From this it can be shown that

$$v = \frac{\partial u}{\partial \phi} = \frac{\partial^2 v}{\partial \phi^2} = 0 \text{ at } \phi = 0. \quad (G20)$$

Hence, since $\frac{\partial u}{\partial \phi}$ approaches zero while H_1 approaches infinity, L'Hôpital's rule is applicable to the product

$$\frac{\mu \cot \phi}{R^2} \frac{\partial u}{\partial \phi}$$

as $\phi \rightarrow 0$. Taking the limit, there is obtained

$$\begin{aligned} \lim_{\phi \rightarrow 0} \frac{\mu \cot \phi}{R^2} \cdot \frac{\partial u}{\partial \phi} &= \frac{\mu}{R^2} \lim_{\phi \rightarrow 0} \frac{\frac{\partial u}{\partial \phi} \cos \phi}{\sin \phi} \\ &= \frac{\mu}{R^2} \lim_{\phi \rightarrow 0} \frac{\frac{\partial^2 u}{\partial \phi^2} \cos \phi - \frac{\partial u}{\partial \phi} \sin \phi}{\cos \phi} \\ &= \frac{\mu}{R^2} \cdot \frac{\partial^2 u}{\partial \phi^2} \end{aligned}$$

Hence, for this case, the coefficient H_1 becomes zero and the coefficient B_1 which multiplies $\frac{\partial^2 u}{\partial \phi^2}$ is increased by μ/R^2 . Applying this limiting process to all the singular terms, the following sets of coefficients are obtained:

TABLE G7
COEFFICIENTS OF EQUILIBRIUM EQUATIONS ON AXIS
OF SYMMETRY ($\phi = 0$) FOR AXIALLY SYMMETRIC CASE
SPHERICAL COORDINATES

k	1	2	3	k	1	2	3	k	1	2	3
A_k	$\lambda+2\mu$	0	0	\bar{A}_k	0	0	0	$\bar{\bar{A}}_k$	0	0	0
B_k	$2\mu/R^2$	0	0	\bar{B}_k	0	0	0	$\bar{\bar{B}}_k$	0	0	0
C_k	0	0	0	\bar{C}_k	0	0	0	$\bar{\bar{C}}_k$	0	0	0
D_k	0	0	0	\bar{D}_k	$2(\lambda+\mu)/R$	0	0	$\bar{\bar{D}}_k$	0	0	0
E_k	0	0	0	\bar{E}_k	0	0	0	$\bar{\bar{E}}_k$	0	0	0
F_k	0	0	0	\bar{F}_k	0	0	0	$\bar{\bar{F}}_k$	0	0	0
G_k	$2(\lambda+2\mu)/R$	0	0	\bar{G}_k	0	0	0	$\bar{\bar{G}}_k$	0	0	0
H_k	0	0	0	\bar{H}_k	$-2(\lambda+3\mu)/R^2$	0	0	$\bar{\bar{H}}_k$	0	0	0
I_k	0	0	0	\bar{I}_k	0	0	0	$\bar{\bar{I}}_k$	0	0	0
J_k	$-2(\lambda+2\mu)/R^2$	0	0	\bar{J}_k	0	0	0	$\bar{\bar{J}}_k$	0	0	0

TABLE G8

COEFFICIENTS OF STRESS EQUATIONS ON AXIS OF
SYMMETRY ($\phi = 0$) FOR AXIALLY SYMMETRIC CASE
SPHERICAL COORDINATES

l	1	2	3	4	5	6
α_l	$\lambda + 2\mu$	λ	λ	0	0	0
β_l	0	0	0	μ/R	0	0
γ_l	0	0	0	0	0	0
δ_l	$2\lambda/R$	$2(\lambda + \mu)/R$	$2(\lambda + \mu)/R$	0	0	0
$\bar{\alpha}_l$	0	0	0	μ	0	0
$\bar{\beta}_l$	$2\lambda/R$	$2(\lambda + \mu)/R$	$2(\lambda + \mu)/R$	0	0	0
$\bar{\gamma}_l$	0	0	0	0	0	0
$\bar{\delta}_l$	0	0	0	$-\mu/R$	0	0
$\bar{\bar{\alpha}}_l$	0	0	0	0	0	0
$\bar{\bar{\beta}}_l$	0	0	0	0	0	0
$\bar{\bar{\gamma}}_l$	0	0	0	0	0	0
$\bar{\bar{\delta}}_l$	0	0	0	0	0	0

J. TEMPERATURE DEPENDENCE OF ELASTIC CONSTANTS

If, in addition to the coefficient of thermal expansion, the elastic constants are strongly dependent on temperature, then additional terms must be included in the displacement equilibrium equations to account for the special derivatives of these constants. For example, in differentiating the stress component τ_{ii} with respect to coordinate α_i [from Equation (G9)] there is obtained

$$\left. \begin{aligned}
\frac{\partial \tau_{ii}}{\partial \alpha_i} &= \lambda \frac{\partial \theta}{\partial \alpha_i} + \theta \frac{\partial \lambda}{\partial \alpha_i} + 2\mu \frac{\partial e_{ii}}{\partial \alpha_i} + 2 e_{ii} \frac{\partial \mu}{\partial \alpha_i} \\
&- (3\lambda + 2\mu) \alpha(T) \frac{\partial T}{\partial \alpha_i} - \frac{\partial}{\partial \alpha_i} - (3\lambda + 2\mu) \int_{T_0}^T \alpha(T) dT \\
&= \theta \left(\frac{\partial \lambda}{\partial T} \right) \left(\frac{\partial T}{\partial \alpha_i} \right) + 2 e_{ii} \left(\frac{\partial \mu}{\partial T} \right) \left(\frac{\partial T}{\partial \alpha_i} \right) - \frac{\partial}{\partial T} (3\lambda + 2\mu) \frac{\partial T}{\partial \alpha_i} \int_{T_0}^T \alpha(T) dT \\
&+ \lambda \frac{\partial \theta}{\partial \alpha_i} + 2\mu \frac{\partial e_{ii}}{\partial \alpha_i} - (3\lambda + 2\mu) \alpha(T) \frac{\partial T}{\partial \alpha_i}
\end{aligned} \right\} \quad (G21)$$

where the first three terms to the right of the equal sign have not been accounted for in the coefficients of Equation (G16). Representing the additional terms by primed quantities, Equation (G16) becomes

$$\begin{aligned}
(A_k + A'_k) \frac{\partial^2 u}{\partial \alpha_1^2} + (B_k + B'_k) \frac{\partial^2 u}{\partial \alpha_2^2} + \dots &= \frac{(3\lambda + 2\mu) \alpha(T)}{\sqrt{g_{kk}}} \frac{\partial T}{\partial \alpha_k} \\
&+ \frac{1}{\sqrt{g_{kk}}} \frac{\partial}{\partial T} (3\lambda + 2\mu) \frac{\partial T}{\partial \alpha_k} \int_{T_0}^T \alpha(T) dT, \quad k = 1, 2, 3
\end{aligned} \quad (G22)$$

The coefficients A'_k, B'_k, \dots are tabulated below for spherical and toroidal coordinates, and for the special point in spherical coordinates on the axis of symmetry for the case of axial symmetry.

TABLE G9

ADDITIONAL TERMS IN COEFFICIENTS OF EQUILIBRIUM EQUATIONS
FROM TEMPERATURE DEPENDENCE OF ELASTIC CONSTANTS
SPHERICAL COORDINATES

	k = 1	k = 2	k = 3
A'_k	0	0	0
B'_k	0	0	0
C'_k	0	0	0
D'_k	0	0	0
E'_k	0	0	0
F'_k	0	0	0
G'_k	$\frac{\partial}{\partial T} (\lambda + 2\mu) \frac{\partial T}{\partial R}$	$\frac{1}{R} \left(\frac{\partial \lambda}{\partial T} \right) \left(\frac{\partial T}{\partial \phi} \right)$	$\frac{1}{R \sin \phi} \left(\frac{\partial \lambda}{\partial T} \right) \frac{\partial T}{\partial \theta}$
H'_k	$\frac{1}{R^2} \left(\frac{\partial \mu}{\partial T} \right) \left(\frac{\partial T}{\partial \phi} \right)$	$\frac{1}{R} \left(\frac{\partial \mu}{\partial T} \right) \left(\frac{\partial T}{\partial R} \right)$	0
I'_k	$\frac{1}{R^2 \sin^2 \phi} \frac{\partial \mu}{\partial T} \frac{\partial T}{\partial \theta}$	0	$\frac{1}{R \sin \phi} \left(\frac{\partial \mu}{\partial T} \right) \frac{\partial T}{\partial R}$
J'_k	$\frac{2}{R} \left(\frac{\partial \lambda}{\partial T} \right) \left(\frac{\partial T}{\partial R} \right)$	$\frac{2}{R^2} \left(\frac{\partial \mu}{\partial T} \right) (\lambda + \mu) \frac{\partial T}{\partial \phi}$	$\frac{2}{R^2 \sin \phi} \left(\frac{\partial \mu}{\partial T} \right) (\lambda + \mu) \frac{\partial T}{\partial \theta}$
\bar{A}'_k	0	0	0
\bar{B}'_k	0	0	0
\bar{C}'_k	0	0	0
\bar{D}'_k	0	0	0
\bar{E}'_k	0	0	0
\bar{F}'_k	0	0	0
\bar{G}'_k	$\frac{1}{R} \left(\frac{\partial \mu}{\partial T} \right) \left(\frac{\partial T}{\partial \phi} \right)$	$\frac{\partial \mu}{\partial T} \frac{\partial T}{\partial R}$	0

TABLE G9 (cont.)

ADDITIONAL TERMS IN COEFFICIENTS OF EQUILIBRIUM EQUATIONS
FROM TEMPERATURE DEPENDENCE OF ELASTIC CONSTANTS
SPHERICAL COORDINATES

	k = 1	k = 2	k = 3
\bar{H}'_k	$\frac{1}{R} \left(\frac{\partial \lambda}{\partial T} \right) \left(\frac{\partial T}{\partial R} \right)$	$\frac{1}{R^2} \left(\frac{\partial}{\partial T} \right) (\lambda + 2\mu) \frac{\partial T}{\partial \phi}$	$\frac{1}{R^2 \sin \phi} \left(\frac{\partial \lambda}{\partial T} \right) \frac{\partial T}{\partial \theta}$
\bar{I}'_k	0	$\frac{1}{R^2 \sin^2 \phi} \left(\frac{\partial \mu}{\partial T} \right) \frac{\partial T}{\partial \theta}$	$\frac{1}{R^2 \sin \phi} \left(\frac{\partial \mu}{\partial T} \right) \frac{\partial T}{\partial \phi}$
\bar{J}'_k	$\frac{\partial \lambda}{\partial T} \left(\frac{\partial T}{\partial R} \right) \left(\frac{\cot \phi}{R} \right) - \frac{1}{R^2} \left(\frac{\partial \mu}{\partial T} \right) \frac{\partial T}{\partial \phi}$	$\frac{\cot \phi}{R^2} \left(\frac{\partial \lambda}{\partial T} \right) \frac{\partial T}{\partial \phi} - \frac{1}{R} \left(\frac{\partial \mu}{\partial T} \right) \frac{\partial T}{\partial R}$	$\frac{\cot \phi}{R^2 \sin \phi} \left(\frac{\partial}{\partial T} \right) (\lambda + 2\mu) \frac{\partial T}{\partial \theta}$
\bar{A}'_k	0	0	0
\bar{B}'_k	0	0	0
\bar{C}'_k	0	0	0
\bar{D}'_k	0	0	0
\bar{E}'_k	0	0	0
\bar{F}'_k	0	0	0
\bar{G}'_k	$\frac{1}{R \sin \phi} \left(\frac{\partial \mu}{\partial T} \right) \frac{\partial T}{\partial \theta}$	0	$\frac{\partial \mu}{\partial T} \frac{\partial T}{\partial R}$
\bar{H}'_k	0	$\frac{1}{R^2 \sin \phi} \left(\frac{\partial \mu}{\partial T} \right) \frac{\partial T}{\partial \theta}$	$\frac{1}{R^2} \left(\frac{\partial \mu}{\partial T} \right) \frac{\partial T}{\partial \phi}$
\bar{I}'_k	$\frac{1}{R \sin \phi} \left(\frac{\partial \lambda}{\partial T} \right) \frac{\partial T}{\partial R}$	$\frac{1}{R^2 \sin \phi} \left(\frac{\partial \lambda}{\partial T} \right) \frac{\partial T}{\partial \phi}$	$\frac{1}{R^2 \sin^2 \phi} \left(\frac{\partial}{\partial T} \right) (\lambda + 2\mu) \frac{\partial T}{\partial \theta}$
\bar{J}'_k	$-\frac{1}{R^2 \sin \phi} \left(\frac{\partial \mu}{\partial T} \right) \frac{\partial T}{\partial \theta}$	$-\frac{\cot \phi}{R^2 \sin \phi} \left(\frac{\partial \mu}{\partial T} \right) \frac{\partial T}{\partial \theta}$	$-\frac{1}{R} \left(\frac{\partial \mu}{\partial T} \right) \frac{\partial T}{\partial R} - \frac{\cot \phi}{R^2} \left(\frac{\partial \mu}{\partial T} \right) \frac{\partial T}{\partial \phi}$

TABLE G10

ADDITIONAL TERMS IN COEFFICIENTS OF EQUILIBRIUM EQUATIONS
FROM TEMPERATURE DEPENDENCE OF ELASTIC CONSTANTS
TOROIDAL COORDINATES

	k = 1	k = 2	k = 3
A'_k	0	0	0
B'_k	0	0	0
C'_k	0	0	0
D'_k	0	0	0
E'_k	0	0	0
F'_k	0	0	0
G'_k	$\frac{\partial}{\partial T}(\lambda+2\mu)\frac{\partial T}{\partial r}$	$\frac{1}{r}\left(\frac{\partial \lambda}{\partial T}\right)\frac{\partial T}{\partial \phi}$	$\frac{1}{(a+r\sin\phi)}\left(\frac{\partial \lambda}{\partial T}\right)\frac{\partial T}{\partial \theta}$
H'_k	$\frac{1}{r^2}\left(\frac{\partial \mu}{\partial T}\right)\frac{\partial T}{\partial \phi}$	$\frac{1}{r}\left(\frac{\partial \mu}{\partial T}\right)\frac{\partial T}{\partial r}$	0
I'_k	$\frac{1}{(a+r\sin\phi)^2}\left(\frac{\partial \mu}{\partial T}\right)\frac{\partial T}{\partial \theta}$	0	$\frac{1}{(a+r\sin\phi)}\left(\frac{\partial \mu}{\partial T}\right)\frac{\partial T}{\partial r}$
J'_k	$\left[\frac{1}{r} + \frac{\sin\phi}{(a+r\sin\phi)}\right]\left(\frac{\partial \lambda}{\partial T}\right)\frac{\partial T}{\partial r}$	$\frac{1}{r^2}\left(\frac{\partial}{\partial T}\right)(\lambda+2\mu)\frac{\partial T}{\partial \phi}$ $+ \frac{\sin\phi}{r(a+r\sin\phi)}\left(\frac{\partial \lambda}{\partial T}\right)\frac{\partial T}{\partial \phi}$	$\frac{\sin\phi}{(a+r\sin\phi)^2}\left(\frac{\partial}{\partial T}\right)(\lambda+2\mu)\frac{\partial T}{\partial \theta}$ $+ \frac{\frac{\partial \lambda}{\partial T} \frac{\partial T}{\partial \theta}}{r(a+r\sin\phi)}$
\bar{A}'_k	0	0	0
\bar{B}'_k	0	0	0
\bar{C}'_k	0	0	0
\bar{D}'_k	0	0	0
\bar{E}'_k	0	0	0
\bar{F}'_k	0	0	0
\bar{G}'_k	$\frac{1}{r}\left(\frac{\partial \mu}{\partial T}\right)\frac{\partial T}{\partial \phi}$	$\frac{\partial \mu}{\partial T} \frac{\partial T}{\partial r}$	0

TABLE G10 (cont.)

ADDITIONAL TERMS IN COEFFICIENTS OF EQUILIBRIUM EQUATIONS
FROM TEMPERATURE DEPENDENCE OF ELASTIC CONSTANTS
TOROIDAL COORDINATES

	k = 1	k = 2	k = 3
\bar{H}'_k	$\frac{1}{r} \left(\frac{\partial \lambda}{\partial T} \right) \frac{\partial T}{\partial r}$	$\frac{1}{r^2} \left(\frac{\partial}{\partial T} \right) (\lambda + 2\mu) \frac{\partial T}{\partial \phi}$	$\frac{1}{r(a + r \sin \phi)} \left(\frac{\partial \lambda}{\partial T} \right) \frac{\partial T}{\partial \theta}$
\bar{I}'_k	0	$\frac{1}{(a + r \sin \phi)^2} \left(\frac{\partial \mu}{\partial T} \right) \frac{\partial T}{\partial \theta}$	$\frac{1}{r(a + r \sin \phi)} \left(\frac{\partial \mu}{\partial T} \right) \frac{\partial T}{\partial \phi}$
\bar{J}'_k	$\frac{\cos \phi}{(a + r \sin \phi)} \left(\frac{\partial \lambda}{\partial T} \right) \frac{\partial T}{\partial r} - \frac{1}{r^2} \left(\frac{\partial \mu}{\partial T} \right) \frac{\partial T}{\partial \phi}$	$\frac{\cos \phi}{r(a + r \sin \phi)} \left(\frac{\partial \lambda}{\partial T} \right) \frac{\partial T}{\partial \phi} - \frac{1}{r} \left(\frac{\partial \mu}{\partial T} \right) \frac{\partial T}{\partial r}$	$\frac{\cos \phi}{(a + r \sin \phi)^2} \left(\frac{\partial}{\partial T} \right) (\lambda + 2\mu) \frac{\partial T}{\partial \theta}$
\bar{A}'_k	0	0	0
\bar{B}'_k	0	0	0
\bar{C}'_k	0	0	0
\bar{D}'_k	0	0	0
\bar{E}'_k	0	0	0
\bar{F}'_k	0	0	0
\bar{G}'_k	$\frac{1}{(a + r \sin \phi)} \left(\frac{\partial \mu}{\partial T} \right) \frac{\partial T}{\partial \theta}$	0	$\frac{\partial \mu}{\partial T} \frac{\partial T}{\partial r}$
\bar{H}'_k	0	$\frac{1}{r(a + r \sin \phi)} \left(\frac{\partial \mu}{\partial T} \right) \frac{\partial T}{\partial \theta}$	$\frac{1}{r^2} \left(\frac{\partial \mu}{\partial T} \right) \frac{\partial T}{\partial \phi}$
\bar{I}'_k	$\frac{1}{(a + r \sin \phi)} \left(\frac{\partial \lambda}{\partial T} \right) \frac{\partial T}{\partial r}$	$\frac{1}{r(a + r \sin \phi)} \left(\frac{\partial \lambda}{\partial T} \right) \frac{\partial T}{\partial \phi}$	$\frac{1}{(a + r \sin \phi)^2} \left(\frac{\partial}{\partial T} \right) (\lambda + 2\mu) \frac{\partial T}{\partial \theta}$
\bar{J}'_k	$-\frac{\sin \phi}{(a + r \sin \phi)^2} \left(\frac{\partial \mu}{\partial T} \right) \frac{\partial T}{\partial \theta}$	$-\frac{\cos \phi}{(a + r \sin \phi)^2} \left(\frac{\partial \mu}{\partial T} \right) \frac{\partial T}{\partial \phi}$	$-\frac{\frac{\partial \mu}{\partial T}}{(a + r \sin \phi)} \left[\frac{\partial T}{\partial r} (\sin \phi) + \frac{\partial T}{\partial \phi} \left(\frac{\cos \phi}{r} \right) \right]$

K. AXIS OF SYMMETRY WITH AXIAL SYMMETRY

The only non-zero terms in the coefficients of Table G9 on the axis of symmetry in the axially symmetric case are the following:

$$G'_1 = \frac{\partial}{\partial T} (\lambda + 2\mu) \frac{\partial T}{\partial R}$$

$$J'_1 = \frac{2}{R} \left(\frac{\partial \lambda}{\partial T} \right) \frac{\partial T}{\partial R}$$

$$\bar{H}'_1 = \frac{2}{R} \left(\frac{\partial \lambda}{\partial R} \right) \frac{\partial T}{\partial R}$$

The integral term in Equation (G22) is also non-zero for the equilibrium equation corresponding to $k = 1$.

L. EQUATIONS FOR THE SINGULAR POINT, $\phi = 0$, FOR THE NON-AXISYMMETRIC CASE

The components of displacement u, v, w in the R, ϕ, θ directions, respectively, have the following properties at $\phi = 0$:

$$\left. \begin{aligned} \frac{\partial u}{\partial \theta} &= 0 & \frac{\partial^3 u}{\partial \theta^2 \partial \phi} &= - \frac{\partial u}{\partial \phi} \\ \frac{\partial w}{\partial \theta} &= v & \frac{\partial^3 w}{\partial \theta^2 \partial \phi} &= - 2 \frac{\partial^2 v}{\partial \theta \partial \phi} \\ \frac{\partial v}{\partial \theta} &= w & \frac{\partial^3 v}{\partial \theta^2 \partial \phi} &= 2 \frac{\partial^2 w}{\partial \theta \partial \phi} \\ \frac{\partial \beta}{\partial \theta} &= 0 & \beta &= \int_{T_0}^T \alpha(T) \partial T \end{aligned} \right\} \quad (G23)$$

These results are derived beginning on page G26 of this appendix.

M. EVALUATION OF STRAINS

From Equation (G23) the singular terms in the strain-displacement equations can be evaluated. Thus we find

$$e_{\theta\theta} = \frac{1}{R \sin \phi} \frac{\partial w}{\partial \theta} + \frac{u}{R} + \frac{v \cot \phi}{R} \xrightarrow{\phi \rightarrow 0} \frac{1}{R} \left(\frac{\partial w}{\partial \theta} + v \right) + \frac{u}{R}$$

and from Equation (G23) we note that the bracketed part is $\begin{pmatrix} 0 \\ 0 \end{pmatrix}$ and thus we have the result

$$e_{\theta\theta} = \frac{1}{R} \left(\frac{\partial^2 w}{\partial \theta \partial \phi} + \frac{\partial v}{\partial \phi} + u \right) (\phi = 0) \quad (G24)$$

Similarly we find

$$e_{\phi\theta} = \frac{1}{2R} \left(\frac{\partial w}{\partial \phi} - w \cot \phi + \frac{1}{\sin \phi} \frac{\partial v}{\partial \theta} \right)$$

and again through Equation (G23) we obtain

$$\left. \begin{aligned} e_{\phi\theta} &= \frac{1}{2R} \left(\frac{\partial w}{\partial \phi} - \frac{\partial w}{\partial \phi} + \frac{\partial^2 v}{\partial \theta \partial \phi} \right) (\phi = 0) \\ e_{\phi\theta} &= \frac{1}{2R} \frac{\partial^2 v}{\partial \theta \partial \phi} (\phi = 0) \end{aligned} \right\} \quad (G25)$$

Finally,

$$\left. \begin{aligned} e_{R\theta} &= \frac{1}{2R} \left(\frac{1}{\sin \phi} \frac{\partial u}{\partial \theta} - w + R \frac{\partial u}{\partial R} \right) \\ e_{R\theta} &= \frac{1}{2R} \left(\frac{\partial^2 u}{\partial \theta \partial \phi} - w + R \frac{\partial u}{\partial R} \right) (\phi = 0) \end{aligned} \right\} \quad (G26)$$

N. EQUATIONS OF EQUILIBRIUM

The equations of equilibrium

$$\frac{\partial \tau_{RR}}{\partial R} + \frac{1}{R} \frac{\partial \tau_{R\phi}}{\partial \phi} + \frac{1}{R \sin \phi} \frac{\partial \tau_{R\theta}}{\partial \theta} + \frac{2\tau_{RR} - \tau_{\phi\phi} - \tau_{\theta\theta} + \tau_{R\phi} \cot \phi}{R} = 0 \quad (G27)$$

$$\frac{\partial \tau_{R\phi}}{\partial R} + \frac{1}{R} \frac{\partial \tau_{\phi\phi}}{\partial \phi} + \frac{1}{R \sin \phi} \frac{\partial \tau_{\phi\phi}}{\partial \theta} + \frac{3\tau_{R\phi} + (\tau_{\phi\phi} - \tau_{\theta\theta}) \cot \phi}{R} = 0 \quad (G28)$$

$$\frac{\partial \tau_{R\theta}}{\partial R} + \frac{1}{R} \frac{\partial \tau_{\phi\theta}}{\partial \phi} + \frac{1}{R \sin \phi} \frac{\partial \tau_{\theta\theta}}{\partial \theta} + \frac{3\tau_{R\theta} + 2\tau_{\phi\theta} \cot \phi}{R} = 0 \quad (G29)$$

have the indeterminate parts indicated below.

From Equation (G27), we have

$$\frac{1}{R \sin \phi} \frac{\partial \tau_{R\theta}}{\partial \theta} + \frac{\tau_{R\phi} \cot \phi}{R} \quad (G30)$$

Putting in the values of $\tau_{R\theta}$ and $\tau_{R\phi}$ in terms of displacements, we get

$$\frac{\partial \tau_{R\theta}}{\partial \theta} + \tau_{R\phi} = \frac{\mu}{R} \left[\frac{1}{\sin \phi} \frac{\partial^2 u}{\partial \theta^2} + \frac{\partial u}{\partial \phi} - \left(\frac{\partial w}{\partial \theta} + v \right) + \frac{\partial}{\partial R} \left(\frac{\partial w}{\partial \theta} + v \right) \right] \quad (G31)$$

From Equation (G23) we find that the expression (G31) is zero at $\phi = 0$. Thus we may write, for Equation (G30)

$$\frac{1}{R} \left(\frac{\partial^2 \tau_{R\theta}}{\partial \theta \partial \phi} + \frac{\partial \tau_{R\phi}}{\partial \phi} \right)$$

which is the evaluation of the indeterminate portion of Equation (G27). From Equation (G28), the indeterminate part is

$$\frac{1}{R \sin \phi} \frac{\partial \tau_{\phi\theta}}{\partial \theta} + \frac{\tau_{\theta\phi} - \tau_{\theta\theta}}{R} \cot \phi \quad (G32)$$

In terms of the displacements

$$\begin{aligned} \frac{\partial \tau_{\theta\phi}}{\partial \theta} + \tau_{\phi\phi} - \tau_{\theta\theta} &= \frac{\mu}{R} \left[\frac{\partial^2 w}{\partial \theta \partial \phi} + 2 \frac{\partial v}{\partial \phi} + \frac{1}{\sin \phi} \frac{\partial}{\partial \theta} \left(\frac{\partial v}{\partial \theta} - w \right) \right. \\ &\quad \left. - \frac{2}{\sin \phi} \left(\frac{\partial w}{\partial \theta} + v \right) \right] \end{aligned}$$

From Equation (G23), the terms in parenthesis are zero. Thus we have

$$\frac{\mu}{R} \left[\frac{\partial^2 w}{\partial \theta \partial \phi} + \frac{\partial^3 v}{\partial \theta^2 \partial \phi} - \frac{\partial^2 w}{\partial \phi \partial \theta} - 2 \frac{\partial^2 w}{\partial v \partial \phi} \right] = \frac{\mu}{R} \left(\frac{\partial^3 v}{\partial \theta^2 \partial \phi} - 2 \frac{\partial^2 w}{\partial \theta \partial \phi} \right)$$

which is zero by Equation (G23). Therefore we may write Equation (G32) as

$$\frac{1}{R} \frac{\partial^2 \tau_{\phi\theta}}{\partial \theta \partial \phi} + \frac{1}{R} \left(\frac{\partial \tau_{\phi\phi}}{\partial \phi} - \frac{\partial \tau_{\theta\theta}}{\partial \theta} \right)$$

which is the evaluation of the indeterminate part of Equation (G28).

From Equation (G29), the indeterminate parts are

$$\frac{1}{R \sin \phi} \frac{\partial \tau_{\theta\theta}}{\partial \theta} + 2 \frac{\tau_{\phi\theta}}{R} \cot \phi \quad (G33)$$

and

$$\frac{1}{R} \frac{\partial \tau_{\theta\theta}}{\partial \theta} \quad (G34)$$

For Equation (33) we have (at $\phi \rightarrow 0$)

$$\begin{aligned} \frac{\partial \tau_{\theta\theta}}{\partial \theta} + 2 \tau_{\theta\phi} &= \frac{2\mu}{R} \frac{\partial w}{\partial \phi} + \frac{2(\lambda + \mu)}{R} \frac{\partial u}{\partial \theta} + \lambda \frac{\partial^2 u}{\partial R \partial \theta} + \frac{\lambda}{R} \frac{\partial^2 v}{\partial \theta \partial \phi} - \frac{\partial \beta(T)}{\partial \theta} \\ &+ \frac{2\mu}{R \sin \phi} \left(\frac{\partial v}{\partial \theta} - w \right) - \frac{(\lambda + 2\mu)}{R \sin \phi} \frac{\partial}{\partial \theta} \left(\frac{\partial w}{\partial \theta} + v \right) \end{aligned}$$

It can be seen from Equation (G23) that the terms with $\frac{1}{\sin \phi}$ have the form $\frac{0}{0}$ and that the terms in u and β are zero. Thus we can write

$$\begin{aligned} \frac{\partial \tau_{\theta\theta}}{\partial \theta} + 2 \tau_{\theta\phi} &= \frac{1}{R} \left(2\mu \frac{\partial^2 v}{\partial \theta \partial \phi} + (\lambda + 2\mu) \frac{\partial^3 w}{\partial \theta^2 \partial \phi} + (\lambda + 2\mu) \frac{\partial^2 v}{\partial \theta \partial \phi} + \lambda \frac{\partial^2 v}{\partial \theta \partial \phi} \right) \\ &= \frac{\lambda + 2\mu}{R} \left(\frac{\partial^3 w}{\partial \theta^2 \partial \phi} + 2 \frac{\partial^2 v}{\partial \theta \partial \phi} \right) = 0 \quad \text{by Equation (G23)} \end{aligned}$$

Thus we may write Equation (G32) as

$$\frac{1}{R} \left(\frac{\partial^2 \tau_{\theta\theta}}{\partial \theta \partial \phi} + 2 \frac{\partial \tau_{\theta\phi}}{\partial \phi} \right)$$

For Equation (G34) we expand the functions in powers of ϕ thus:

$$w = w_0 + w_1 \phi + w_2 \phi^2 + \dots$$

Then we may write (for $\phi \rightarrow 0$)

$$\begin{aligned} \frac{\partial \tau_{\theta\phi}}{\partial \phi} &= \frac{\partial}{\partial \phi} \left[w_1 + 2w_2\phi - \frac{w_0 + w_1\phi + w_2\phi^2}{\phi} + \left(\frac{\partial v_0}{\partial \theta} + \frac{\partial v_1}{\partial \theta} \phi + \frac{\partial v_2}{\partial \theta} \phi^2 \right) / \phi \right] \\ &= 2w_2 + \frac{w_0 - \frac{\partial v_0}{\partial \theta}}{\phi^2} - w_2 + \frac{\partial v_2}{\partial \theta} \end{aligned}$$

And since $w_0 = \frac{\partial v_0}{\partial \theta}$ from Equation (G23), we have

$$\left. \begin{aligned} \frac{\partial \tau_{\theta\phi}}{\partial \phi} &= w_2 + \frac{\partial v_2}{\partial \theta} \quad (\phi = 0) \\ &= \frac{1}{2} \left(\frac{\partial^2 w}{\partial \phi^2} + \frac{\partial^3 v}{\partial \theta \partial \phi^2} \right) \quad (\phi = 0) \end{aligned} \right\} \quad (G35)$$

Thus, the one term, $\frac{\partial \tau_{\theta\phi}}{\partial \phi}$, in the equilibrium equations is not evaluated by simple differentiation with respect to ϕ .*

0. RELATIONS BETWEEN DISPLACEMENTS AT $\phi = 0$

To obtain the equations given in Equation (G23), we take the gradient and the Laplacian of the displacement vector, i.e.,

* See note on page G28.

$$\nabla \overline{\xi} = \nabla (\hat{R}u + \hat{\phi}v + \hat{\theta}w)$$

and

$$\nabla^2 \overline{\xi} = \nabla^2 (\hat{R}u + \hat{\phi}v + \hat{\theta}w)$$

Expanding in powers of ϕ , viz,

$$u = u_0 + u_1 \phi + u_2 \phi^2 + \dots$$

and letting $\phi \rightarrow 0$ after obtaining $\nabla \overline{\xi}$ and $\nabla^2 \overline{\xi}$, we find the following terms involving $\frac{1}{\phi}$ as a factor:

$$\hat{\theta} \hat{\theta} \left(\frac{\partial w_0}{\partial \theta} + v_0 \right)$$

$$\hat{\theta} \hat{\phi} \left(\frac{\partial v_0}{\partial \theta} - w_0 \right)$$

$$\hat{\theta} \hat{R} \left(\frac{\partial u_0}{\partial \theta} \right)$$

$$\hat{R} \left(\frac{\partial^2 u_1}{\partial \theta^2} + u_1 \right)$$

$$\hat{\phi} \left(\frac{\partial^2 v_1}{\partial \theta^2} - 2 \frac{\partial w_1}{\partial \theta} \right)$$

$$\hat{\theta} \left(\frac{\partial^2 w_1}{\partial \theta^2} + 2 \frac{\partial v_1}{\partial \theta} \right)$$

Since $\nabla \overline{\xi}$ and $\nabla^2 \overline{\xi}$ must be finite when $\phi = 0$, the above expressions (having $\frac{1}{\phi}$ as a multiplier) must all vanish. Thus we have

$$\frac{\partial w_0}{\partial \theta} = -v_0$$

$$\frac{\partial v_0}{\partial \theta} = w_0$$

$$\frac{\partial u_c}{\partial \theta} = 0$$

$$\frac{\partial^2 w_1}{\partial \theta^2} = -2 \frac{\partial v_1}{\partial \theta}$$

$$\frac{\partial^2 v_1}{\partial \theta^2} = 2 \frac{\partial w_1}{\partial \theta}$$

$$\frac{\partial^2 u_1}{\partial \theta^2} = -u_1$$

The equations in (G23) are equivalent to these when $\phi = 0$. The equation

$$\frac{\partial \beta}{\partial \theta} = 0 \quad (\phi = 0)$$

where

$$\beta = \int_{T_0}^T \alpha(T) dT$$

can be seen from the fact that at $\phi = 0$, there is only one point for all θ , so that β cannot vary with θ . (The same argument could have been applied to the quantity u . It could not be applied to w or v since these are functions which have extension in θ .)

III. DERIVATION OF THE FINITE-DIFFERENCE MODEL OF THE EQUATIONS

The finite-difference model of the basic equations is presented below. The difference analogs to the partial differential equations are constructed on a grid network as shown in Figure G2, for which α_1 - constant lines are ordered by the subscript i , α_2 = constant lines by the subscript j , α_3 = constant lines by the subscript k , and the intersection of grid lines (nodes) by the triple subscript i, j, k .

Note: The further evaluation of Equation (G35) may reduce the expression to a simple derivation process, but such a possibility will not be investigated since the expression has already been made determinate.

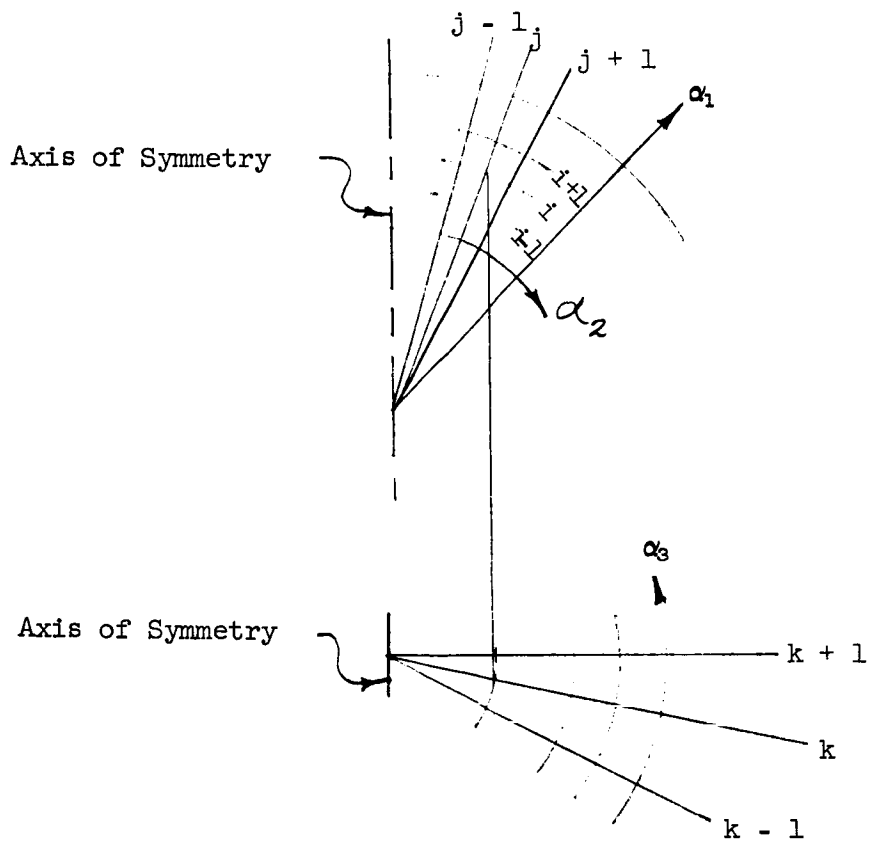


Fig. G2 - Grid Notation for Finite-Difference Formulation

For the general case, the grid spacing will be irregular and the increments in the vicinity of a node will be designated by the following notations:

$h_{11} = (\alpha_1)_{i+1} - (\alpha_1)_i$	$h_{21} = (\alpha_2)_{i+1} - (\alpha_2)_i$	$h_{31} = (\alpha_3)_{i+1} - (\alpha_3)_i$
$h_{12} = (\alpha_1)_{i+2} - (\alpha_1)_i$	$h_{22} = (\alpha_2)_{i+2} - (\alpha_2)_i$	$h_{32} = (\alpha_3)_{i+2} - (\alpha_3)_i$
$h_{13} = (\alpha_1)_i - (\alpha_1)_{i-1}$	$h_{23} = (\alpha_2)_i - (\alpha_2)_{i-1}$	$h_{33} = (\alpha_3)_i - (\alpha_3)_{i-1}$
$h_{14} = (\alpha_1)_i - (\alpha_1)_{i-2}$	$h_{24} = (\alpha_2)_i - (\alpha_2)_{i-2}$	$h_{34} = (\alpha_3)_i - (\alpha_3)_{i-2}$

Let $f(\alpha_1, \alpha_2, \alpha_3)$ be any function of the coordinates such that it and its partial derivatives (up to any order required in the analysis) are continuous, and expand the function about the point i, j, k . Using a new coordinate system with origin at i, j, k and with ξ_1, ξ_2, ξ_3 directed along $\alpha_1, \alpha_2, \alpha_3$, respectively, the function $f(\xi_1, \xi_2, \xi_3)$ is written

$$\begin{aligned} f(\xi_1, \xi_2, \xi_3) = & f_{i,j,k} + B_1 \xi_1 + B_2 \xi_2 + B_3 \xi_3 + B_4 \xi_1 \xi_2 + B_5 \xi_2 \xi_3 \\ & + B_6 \xi_3 \xi_1 + B_7 \xi_1^2 + B_8 \xi_2^2 + B_9 \xi_3^2 + B_{10} \xi_1 \xi_2 \xi_3 \\ & + B_{11} \xi_1 \xi_2^2 + B_{12} \xi_1 \xi_3^2 + B_{13} \xi_1^2 \xi_2 + B_{14} \xi_2^2 \xi_3 + \dots \quad (G36) \end{aligned}$$

The first and second derivatives of $f(\alpha_1, \alpha_2, \alpha_3)$ with respect to $\alpha_1, \alpha_2, \alpha_3$ are obtained from Equation (G36) according to

$$\left. \begin{aligned} \frac{\partial f}{\partial \alpha_1} \Big|_{i,j,k} &= \frac{\partial f}{\partial \xi_1} \Big|_{(0,0,0)} = B_1, \quad \frac{\partial^2 f}{\partial \alpha_1 \partial \alpha_2} \Big|_{i,j,k} = B_4, \quad \frac{\partial^2 f}{\partial \alpha_1^2} \Big|_{i,j,k} = 2 B_7 \\ \frac{\partial f}{\partial \alpha_2} \Big|_{i,j,k} &= \frac{\partial f}{\partial \xi_2} \Big|_{(0,0,0)} = B_2, \quad \frac{\partial^2 f}{\partial \alpha_2 \partial \alpha_3} \Big|_{i,j,k} = B_5, \quad \frac{\partial^2 f}{\partial \alpha_2^2} \Big|_{i,j,k} = 2 B_8 \\ \frac{\partial f}{\partial \alpha_3} \Big|_{i,j,k} &= \frac{\partial f}{\partial \xi_3} \Big|_{(0,0,0)} = B_3, \quad \frac{\partial^2 f}{\partial \alpha_3 \partial \alpha_1} \Big|_{i,j,k} = B_6, \quad \frac{\partial^2 f}{\partial \alpha_3^2} \Big|_{i,j,k} = 2 B_9 \end{aligned} \right\} \quad (G37)$$

By considering the values of $f(\xi_1, \xi_2, \xi_3)$ at the 12 nodes adjacent to i, j, k , the constants B_i are evaluated in terms of the function at these nodes and the grid spacings as shown in Figure G3.

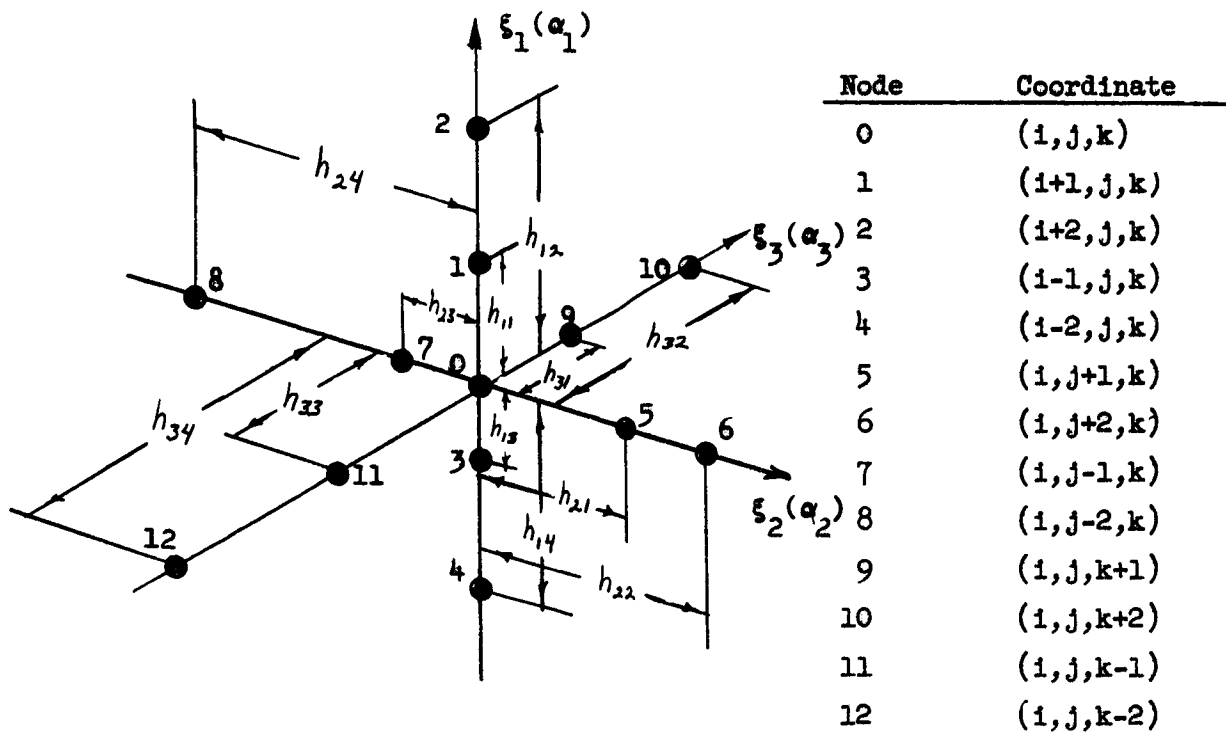


Fig. G3 - Coordinates of Irregular Mesh Intervals

Note that the grid spacing increments h_{ij} do not, in general, have the dimensions of length but have the dimensions of α_1 , α_2 , and α_3 .

At points 1 and 3, Equation (G36) becomes

$$\left. \begin{aligned} f(h_{11}, 0, 0) &= f_{i,j,k} + B_1 h_{11} + B_7 h_{11}^2 \\ f(-h_{13}, 0, 0) &= f_{i,j,k} - B_1 h_{13} + B_7 h_{13}^2 \end{aligned} \right\} \quad (G38)$$

where terms of higher order are deleted. Solving for B_1 and B_7 from Equation (G38) gives, for the first and second irregular central derivative with respect to α_1 ,

$$\left. \begin{aligned} \frac{\partial f}{\partial \alpha_1} \Big|_{i,j,k} &= \frac{h_{13}^2 f_{i+1,j,k} + (h_{11}^2 - h_{13}^2) f_{i,j,k} - h_{11}^2 f_{i-1,j,k}}{h_{11} h_{13} (h_{11} + h_{13})} \\ \frac{\partial^2 f}{\partial \alpha_1^2} \Big|_{i,j,k} &= 2 \left[\frac{h_{13} f_{i+1,j,k} - (h_{11} + h_{13}) f_{i,j,k} + h_{11} f_{i-1,j,k}}{h_{11} h_{13} (h_{11} + h_{13})} \right] \end{aligned} \right\} \quad (G39)$$

Substituting $h_{11} = h_{13} = h_1$ into Equation (G39) gives for the first and second angular central derivatives with respect to α_1

$$\left. \begin{aligned} \frac{\partial f}{\partial \alpha_1} \Big|_{i,j,k} &= \frac{f_{i+1,j,k} - f_{i-1,j,k}}{2h_1} \\ \frac{\partial^2 f}{\partial \alpha_1^2} \Big|_{i,j,k} &= \frac{f_{i+1,j,k} - 2f_{i,j,k} + f_{i-1,j,k}}{h_1^2} \end{aligned} \right\} \quad (G40)$$

By a similar procedure, the following first and second regular and irregular central derivatives are obtained with respect to the coordinates α_2 and α_3 :

First Regular Central Derivatives ($h_2 = h_{21} = h_{23}$, $h_3 = h_{31} = h_{33}$)

$$\frac{\partial f}{\partial \alpha_2} \Big|_{i,j,k} = \frac{f_{i,j+1,k} - f_{i,j-1,k}}{2h_2} \quad (G41)$$

$$\frac{\partial f}{\partial \alpha_3} \Big|_{i,j,k} = \frac{f_{i,j,k+1} - f_{i,j,k-1}}{2h_3} \quad (G42)$$

First Irregular Central Derivatives

$$\frac{\partial f}{\partial \alpha_2} \Big|_{i,j,k} = \frac{h_{23}^2 f_{i,j+1,k} + (h_{21}^2 - h_{23}^2) f_{i,j,k} - h_{21}^2 f_{i,j-1,k}}{h_{21} h_{23} (h_{21} + h_{23})} \quad (G43)$$

$$\frac{\partial f}{\partial \alpha_3} \Big|_{i,j,k} = \frac{h_{33}^2 f_{i,j,k+1} + (h_{31}^2 - h_{33}^2) f_{i,j,k} - h_{31}^2 f_{i,j,k-1}}{h_{31} h_{33} (h_{31} + h_{33})} \quad (G44)$$

Second Regular Central Derivatives ($h_2 = h_{21} = h_{23}$, $h_3 = h_{31} = h_{33}$)

$$\frac{\partial^2 f}{\partial \alpha_2^2} \Big|_{i,j,k} = \frac{f_{i,j+1,k} - 2f_{i,j,k} + f_{i,j-1,k}}{h_2^2} \quad (G45)$$

$$\left. \frac{\partial^2 f}{\partial \alpha_3^2} \right|_{i,j,k} = \frac{f_{i,j,k+1} - 2f_{i,j,k} + f_{i,j,k-1}}{h_3^2} \quad (G46)$$

Second Irregular Central Derivatives

$$\left. \frac{\partial^2 f}{\partial \alpha_2^2} \right|_{i,j,k} = \frac{2 \left[h_{23} f_{i,j+1,k} - (h_{21} + h_{23}) f_{i,j,k} + h_{21} f_{i,j-1,k} \right]}{h_{21} h_{23} (h_{21} + h_{23})} \quad (G47)$$

$$\left. \frac{\partial^2 f}{\partial \alpha_3^2} \right|_{i,j,k} = \frac{2 \left[h_{33} f_{i,j,k+1} + (h_{31} + h_{33}) f_{i,j,k} + h_{31} f_{i,j,k-1} \right]}{h_{31} h_{33} (h_{31} + h_{33})} \quad (G48)$$

Forward and Backward Derivatives

By applying the same procedure as above with respect to two nodes located either forward or backward from the origin (i,j,k) , the first and second regular and irregular derivatives are obtained in terms of the function $f(\alpha_1, \alpha_2, \alpha_3)$ evaluated at these nodes. The results are summarized below for the three coordinate directions:

First Irregular Forward Derivatives

$$\left. \frac{\partial f}{\partial \alpha_1} \right|_{i,j,k} = \frac{-(h_{12}^2 - h_{11}^2) f_{i,j,k} + h_{12}^2 f_{i+1,j,k} - h_{11}^2 f_{i+2,j,k}}{h_{11} h_{12} (h_{12} - h_{11})} \quad (G49)$$

$$\left. \frac{\partial f}{\partial \alpha_2} \right|_{i,j,k} = \frac{-(h_{22}^2 - h_{21}^2) f_{i,j,k} + h_{22}^2 f_{i,j+1,k} - h_{21}^2 f_{i,j+2,k}}{h_{21} h_{22} (h_{22} - h_{21})} \quad (G50)$$

$$\left. \frac{\partial f}{\partial \alpha_3} \right|_{i,j,k} = \frac{-(h_{32}^2 - h_{31}^2) f_{i,j,k} + h_{32}^2 f_{i,j,k+1} - h_{31}^2 f_{i,j,k+2}}{h_{31} h_{32} (h_{32} - h_{31})} \quad (G51)$$

First Regular Forward Derivatives

For equal grid spacings in each of the three coordinate directions, defined according to

$$\left. \begin{aligned} h_{11} &= h_{12}/2 \equiv h_1 \\ h_{21} &= h_{22}/2 \equiv h_2 \\ h_{31} &= h_{32}/2 \equiv h_3 \end{aligned} \right\} \quad (G52)$$

Equations (G49) through (G51) reduce to

$$\left. \frac{\partial f}{\partial \alpha_1} \right|_{i,j,k} = \frac{-3f_{i,j,k} + 4f_{i+1,j,k} - f_{i+2,j,k}}{2h_1} \quad (G53)$$

$$\left. \frac{\partial f}{\partial \alpha_2} \right|_{i,j,k} = \frac{-3f_{i,j,k} + 4f_{i,j+1,k} - f_{i,j+2,k}}{2h_2} \quad (G54)$$

$$\left. \frac{\partial f}{\partial \alpha_3} \right|_{i,j,k} = \frac{-3f_{i,j,k} + 4f_{i,j,k+1} - f_{i,j,k+2}}{2h_3} \quad (G55)$$

Second Irregular Forward Derivatives

$$\left. \frac{\partial^2 f}{\partial \alpha_1^2} \right|_{i,j,k} = 2 \left[\frac{-h_{12}f_{i+1,j,k} + (h_{12} - h_{11})f_{i,j,k} + h_{11}f_{i+2,j,k}}{h_{11}h_{12}(h_{12} - h_{11})} \right] \quad (G56)$$

$$\left. \frac{\partial^2 f}{\partial \alpha_2^2} \right|_{i,j,k} = 2 \left[\frac{-h_{22}f_{i,j+1,k} + (h_{22} - h_{21})f_{i,j,k} + h_{21}f_{i,j+2,k}}{h_{21}h_{22}(h_{22} - h_{21})} \right] \quad (G57)$$

$$\left. \frac{\partial^2 f}{\partial \alpha_3^2} \right|_{i,j,k} = 2 \left[\frac{-h_{32}f_{i,j,k+1} + (h_{32} - h_{31})f_{i,j,k} + h_{31}f_{i,j,k+2}}{h_{31}h_{32}(h_{32} - h_{31})} \right] \quad (G58)$$

Second Regular Forward Derivatives

With equal grid spacing, according to Equation (G52), Equations (G56) through (G58) reduce to

$$\left. \frac{\partial^2 f}{\partial \alpha_1^2} \right|_{i,j,k} = \frac{-2f_{i+1,j,k} + f_{i,j,k} + f_{i+2,j,k}}{h_1^2} \quad (G59)$$

$$\left. \frac{\partial^2 f}{\partial \alpha_2^2} \right|_{i,j,k} = \frac{-2f_{i,j+1,k} + f_{i,j,k} + f_{i,j+2,k}}{h_2^2} \quad (G60)$$

$$\left. \frac{\partial^2 f}{\partial \alpha_3^2} \right|_{i,j,k} = \frac{-2f_{i,j,k+1} + f_{i,j,k} + f_{i,j,k+2}}{h_3^2} \quad (G61)$$

First Irregular Backward Derivatives

$$\left. \frac{\partial f}{\partial \alpha_1} \right|_{i,j,k} = \frac{h_{13}^2 f_{i-2,j,k} + (h_{14}^2 - h_{13}^2) f_{i,j,k} - h_{14}^2 f_{i-1,j,k}}{h_{13} h_{14} (h_{14} - h_{13})} \quad (G62)$$

$$\left. \frac{\partial f}{\partial \alpha_2} \right|_{i,j,k} = \frac{h_{23}^2 f_{i,j-2,k} + (h_{24}^2 - h_{23}^2) f_{i,j,k} - h_{24}^2 f_{i,j-1,k}}{h_{23} h_{24} (h_{24} - h_{23})} \quad (G63)$$

$$\left. \frac{\partial f}{\partial \alpha_3} \right|_{i,j,k} = \frac{h_{33}^2 f_{i,j,k-2} + (h_{34}^2 - h_{33}^2) f_{i,j,k} - h_{34}^2 f_{i,j,k-1}}{h_{33} h_{34} (h_{34} - h_{33})} \quad (G64)$$

First Regular Backward Derivatives ($h_{13} = h_{14}/2 \equiv h_1$, etc.)

$$\left. \frac{\partial f}{\partial \alpha_1} \right|_{i,j,k} = \frac{f_{i-2,j,k} + 3f_{i,j,k} - 4f_{i-1,j,k}}{2h_1} \quad (G65)$$

$$\left. \frac{\partial f}{\partial \alpha_2} \right|_{i,j,k} = \frac{f_{i,j-2,k} + 3f_{i,j,k} - 4f_{i,j-1,k}}{2h_2} \quad (G66)$$

$$\left. \frac{\partial f}{\partial \alpha_3} \right|_{i,j,k} = \frac{f_{i,j,k-2} + 3f_{i,j,k} - 4f_{i,j,k-1}}{2h_3} \quad (G67)$$

Second Irregular Backward Derivatives

$$\left. \frac{\partial^2 f}{\partial \alpha_1^2} \right|_{i,j,k} = 2 \left[\frac{h_{13}^2 f_{i-2,j,k} + (h_{14}^2 - h_{13}^2) f_{i,j,k} - h_{14}^2 f_{i-1,j,k}}{h_{13} h_{14} (h_{14} - h_{13})} \right] \quad (G68)$$

$$\left. \frac{\partial^2 f}{\partial \alpha_2^2} \right|_{i,j,k} = 2 \left[\frac{h_{23} f_{i,j-2,k} + (h_{24} - h_{23}) f_{i,j,k} - h_{24} f_{i,j-1,k}}{h_{23} h_{24} (h_{24} - h_{23})} \right] \quad (G69)$$

$$\left. \frac{\partial^2 f}{\partial \alpha_3^2} \right|_{i,j,k} = 2 \left[\frac{h_{33} f_{i,j,k-2} + (h_{34} - h_{33}) f_{i,j,k} - h_{34} f_{i,j,k-1}}{h_{33} h_{34} (h_{34} - h_{33})} \right] \quad (G70)$$

Second Regular Backward Derivatives

$$\left. \frac{\partial^2 f}{\partial \alpha_1^2} \right|_{i,j,k} = \frac{f_{i-2,j,k} + f_{i,j,k} - 2f_{i-1,j,k}}{h_1^2} \quad (G71)$$

$$\left. \frac{\partial^2 f}{\partial \alpha_2^2} \right|_{i,j,k} = \frac{f_{1,j-2,k} + f_{i,j,k} - 2f_{i,j-1,k}}{h_2^2} \quad (G72)$$

$$\left. \frac{\partial^2 f}{\partial \alpha_3^2} \right|_{i,j,k} = \frac{f_{i,j,k-2} + f_{i,j,k} - 2f_{i,j,k-1}}{h_3^2} \quad (G73)$$

Mixed Derivatives

It can be shown from Equation (G36) that mixed derivatives require values of the function at any six nodes in the vicinity of the point under consideration. Figure G4 shows various combinations of mixed derivatives with respect to the coordinate axes α_1 and α_2 . It is noted that the mixed central derivatives involve the four corner nodes as well as two adjacent nodes in either of the two coordinate directions. The various combinations shown in Figure G4 are summarized below for the coordinate directions α_1 and α_2 :

Second Mixed Irregular Central Derivative with Respect to α_1 and α_2

$$\begin{aligned} \text{a) } \left. \frac{\partial^2 f}{\partial \alpha_1 \partial \alpha_2} \right|_{i,j,k} &= \frac{1}{h_{21} h_{23} (h_{11} + h_{13}) (h_{21} + h_{23})} \left[h_{23}^2 (f_{i+1,j+1,k} \right. \\ &\quad - f_{i-1,j+1,k}) - (h_{23}^2 - h_{21}^2) (f_{i+1,j,k} - f_{i-1,j,k}) \\ &\quad \left. - h_{21}^2 (f_{i+1,j-1,k} - f_{i-1,j-1,k}) \right] \end{aligned} \quad (G74)$$

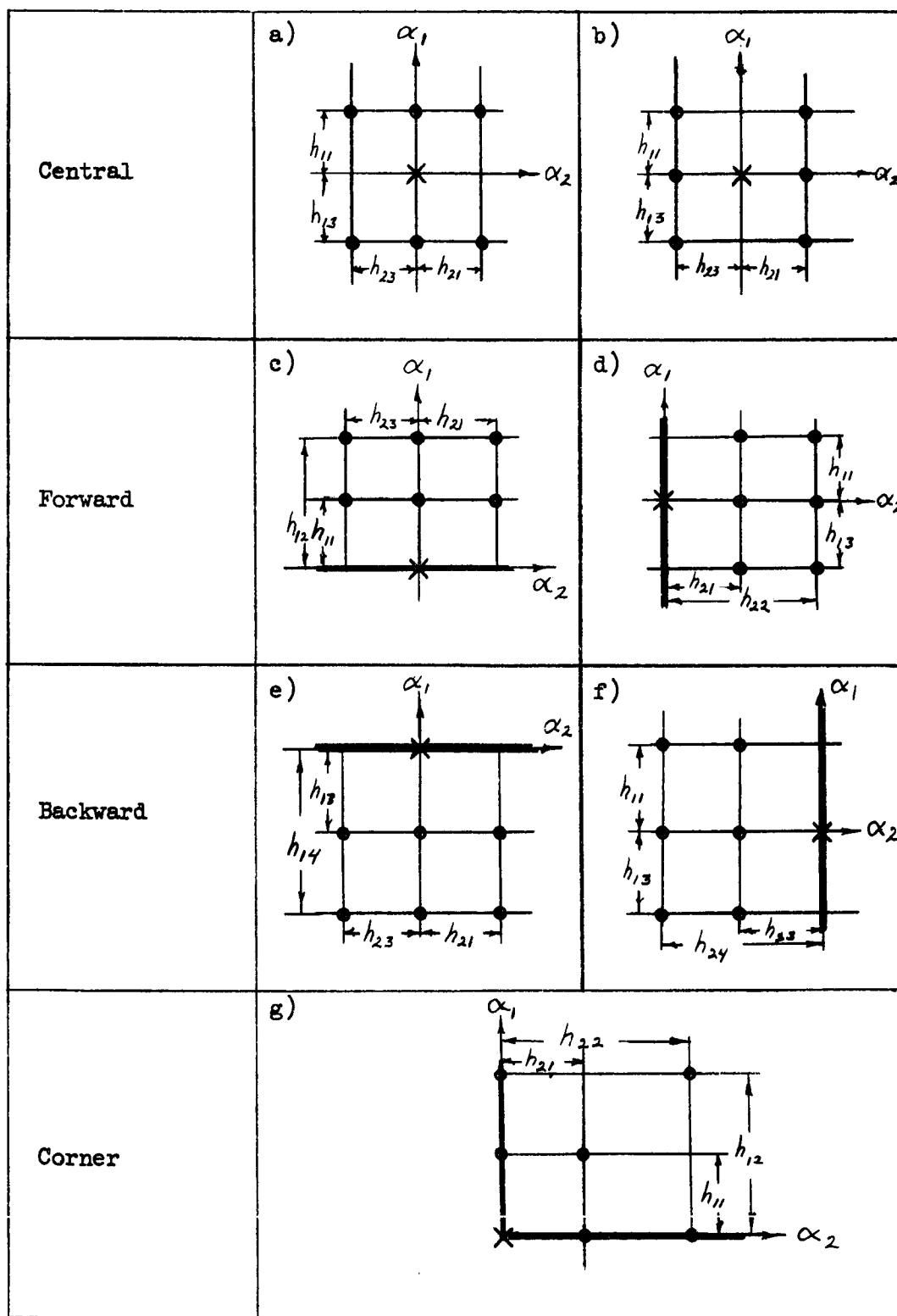


Fig. G4 - Irregular Mesh Intervals for Mixed Central, Forward Backward, and Corner Derivatives

$$\begin{aligned}
b) \quad \frac{\partial^2 f}{\partial \alpha_1 \partial \alpha_2} \bigg|_{i,j,k} &= \frac{1}{h_{11} h_{13} (h_{11} + h_{13}) (h_{21} + h_{23})} \left[h_{13}^2 (f_{i+1,j+1,k} \right. \\
&\quad - f_{i+1,j-1,k}) - (h_{13}^2 - h_{11}^2) (f_{i,j+1,k} - f_{i,j-1,k}) \\
&\quad \left. - h_{11}^2 (f_{i-1,j+1,k} - f_{i-1,j-1,k}) \right] \quad (G75)
\end{aligned}$$

Second Mixed Irregular Forward Derivative with Respect to α_1 and α_2

$$\begin{aligned}
c) \quad \frac{\partial^2 f}{\partial \alpha_1 \partial \alpha_2} \bigg|_{i,j,k} &= \frac{1}{h_{21} h_{23} (h_{11} - h_{12}) (h_{21} + h_{23})} \left[h_{23}^2 (f_{i+1,j+1,k} \right. \\
&\quad - f_{i+1,j,k} - f_{i+2,j+1,k} + f_{i+2,j,k}) - h_{21}^2 (f_{i+1,j-1,k} \\
&\quad \left. - f_{i+1,j,k} - f_{i+2,j-1,k} + f_{i+2,j,k}) \right] \quad (G76)
\end{aligned}$$

$$\begin{aligned}
d) \quad \frac{\partial^2 f}{\partial \alpha_1 \partial \alpha_2} \bigg|_{i,j,k} &= \frac{1}{h_{11} h_{13} (h_{21} - h_{22}) (h_{11} + h_{13})} \left[h_{13}^2 (f_{i+1,j+1,k} \right. \\
&\quad - f_{i,j+1,k} - f_{i+1,j+2,k} + f_{i,j+2,k}) - h_{11}^2 (f_{i-1,j+1,k} \\
&\quad \left. - f_{i,j+1,k} - f_{i-1,j+2,k} + f_{i,j+2,k}) \right] \quad (G77)
\end{aligned}$$

Second Mixed Irregular Backward Derivative with Respect to α_1 and α_2

$$\begin{aligned}
e) \quad \frac{\partial^2 f}{\partial \alpha_1 \partial \alpha_2} \bigg|_{i,j,k} &= \frac{1}{h_{21} h_{23} (h_{13} - h_{14}) (h_{21} + h_{23})} \left[h_{23}^2 (f_{i-1,j+1,k} \right. \\
&\quad - f_{i-2,j+1,k} + f_{i-2,j,k} - f_{i-1,j,k}) - h_{21}^2 (f_{i-1,j-1,k} \\
&\quad \left. - f_{i-1,j,k} - f_{i-2,j-1,k} + f_{i-2,j,k}) \right]
\end{aligned}$$

$$\begin{aligned}
 f) \quad \frac{\partial^2 f}{\partial \alpha_1 \partial \alpha_2} \Big|_{i,j,k} &= \frac{-1}{h_{11} h_{13} (h_{23} - h_{24}) (h_{11} + h_{13})} \left[h_{13}^2 (f_{i+1,j-1,k} \right. \\
 &\quad - f_{i+1,j-2,k} + f_{i,j-2,k} - f_{i,j-1,k}) - h_{11}^2 (f_{i-1,j-1,k} \\
 &\quad \left. - f_{i,j-1,k} - f_{i-1,j-2,k} + f_{i,j-2,k}) \right] \quad (G79)
 \end{aligned}$$

Second Mixed Irregular Corner Derivative with Respect to α_1 and α_2

$$\begin{aligned}
 g) \quad \frac{\partial^2 f}{\partial \alpha_1 \partial \alpha_2} &= \frac{1}{h_{11} h_{12} h_{21} h_{22} (h_{22} - h_{21})} \left[h_{12} h_{22}^2 (f_{i+1,j+1,k} - f_{i+1,j,k} \right. \\
 &\quad - f_{i,j+1,k} + f_{i,j,k}) - h_{11} h_{21}^2 (f_{i+2,j+2,k} - f_{i+2,j,k} \\
 &\quad \left. - f_{i,j+2,k} + f_{i,j,k}) \right] \quad (G80)
 \end{aligned}$$

Second Mixed Regular Derivatives

All of the above results can be reduced to regular derivatives with respect to either α_1 , α_2 or both coordinates by making the substitutions

$$h_{11} = h_{13} = \frac{h_{12}}{2} = \frac{h_{14}}{2} \equiv h_1 \quad (G81)$$

$$h_{21} = h_{23} = \frac{h_{22}}{2} = \frac{h_{24}}{2} \equiv h_2 \quad (G82)$$

$$h_1 = h_2 = h \quad (G83)$$

The various derivatives are summarized below for the case in which all grid spacings are equal (i.e., $h_1 = h_2$).

Second Mixed Regular Central Derivative with Respect to α_1 and α_2

$$a), b) \quad \left. \frac{\partial^2 f}{\partial \alpha_1 \partial \alpha_2} \right|_{i,j,k} = \frac{1}{4 h^2} \left(f_{i+1,j+1,k} - f_{i+1,j-1,k} - f_{i-1,j+1,k} + f_{i-1,j-1,k} \right) \quad (G84)$$

Second Mixed Regular Forward Derivative with Respect to α_1 and α_2

$$c) \quad \left. \frac{\partial^2 f}{\partial \alpha_1 \partial \alpha_2} \right|_{i,j,k} = \frac{-1}{2 h^2} \left(f_{i+1,j+1,k} - f_{i+2,j+1,k} - f_{i+1,j-1,k} + f_{i+2,j-1,k} \right) \quad (G85)$$

$$d) \quad \left. \frac{\partial^2 f}{\partial \alpha_1 \partial \alpha_2} \right|_{i,j,k} = \frac{-1}{2 h^2} \left(f_{i+1,j+1,k} - f_{i+1,j+2,k} - f_{i-1,j+1,k} + f_{i-1,j+2,k} \right) \quad (G86)$$

Second Mixed Regular Backward Derivative with Respect to α_1 and α_2

$$e) \quad \left. \frac{\partial^2 f}{\partial \alpha_1 \partial \alpha_2} \right|_{i,j,k} = \frac{1}{2 h^2} \left(f_{i-1,j+1,k} - f_{i-2,j+1,k} - f_{i-1,j-1,k} + f_{i-2,j-1,k} \right) \quad (G87)$$

$$f) \quad \left. \frac{\partial^2 f}{\partial \alpha_1 \partial \alpha_2} \right|_{i,j,k} = \frac{1}{2 h^2} \left(f_{i+1,j-1,k} - f_{i+1,j-2,k} - f_{i-1,j-1,k} + f_{i-1,j-2,k} \right) \quad (G88)$$

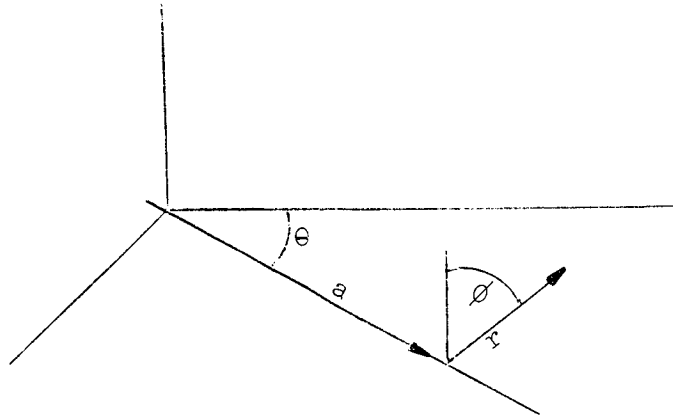
Second Mixed Regular Corner Derivative with Respect to α_1 and α_2

$$g) \quad \left. \frac{\partial^2 f}{\partial \alpha_1 \partial \alpha_2} \right|_{i,j,k} = \frac{1}{4 h^2} \left[8 \left(f_{i+1,j+1,k} - f_{i+1,j,k} - f_{i,j+1,k} + f_{i,j,k} \right) \right. \\ \left. - \left(f_{i+2,j+2,k} - f_{i+2,j,k} - f_{i,j+2,k} + f_{i,j,k} \right) \right] \quad (G89)$$

IV. DEVELOPMENT OF THIN SHELL MODEL FOR THIN LAYER AND ITS FINITE-DIFFERENCE EQUIVALENT

A. THE THIN SHELL IN SPHERICAL AND TOROIDAL COORDINATES

The thin-shell analysis used here has been derived from a rigorous, direct expansion in appropriate powers of z (the distance from shell center surface). The equations are derived in the toroidal system since the spherical case can be obtained from these by putting $a = 0$.



Strain-displacement relations are shown in Equations (G90) through (G95).

$$\epsilon_{rr} = \frac{\partial u}{\partial r} \quad (G90)$$

$$\epsilon_{r\phi} = \frac{1}{2} \left(\frac{\partial v}{\partial r} - \frac{v}{r} + \frac{1}{r} \frac{\partial u}{\partial \phi} \right) \quad (G91)$$

$$\epsilon_{r\theta} = \frac{1}{2} \left(\frac{\partial w}{\partial r} - \frac{w \sin \phi}{a + r \sin \phi} + \frac{1}{a + r \sin \phi} \frac{\partial u}{\partial \theta} \right) \quad (G92)$$

$$\epsilon_{\phi\phi} = \frac{1}{r} \frac{\partial v}{\partial \phi} + \frac{u}{r} \quad (G93)$$

$$\epsilon_{\theta\theta} = \frac{1}{a + r \sin \phi} \left(\frac{\partial w}{\partial \theta} + u \sin \phi + v \cos \phi \right) \quad (G94)$$

$$\epsilon_{\phi\theta} = \frac{1}{2} \left(\frac{1}{r} \frac{\partial w}{\partial \phi} + \frac{1}{a + r \sin \phi} \frac{\partial v}{\partial \theta} - \frac{w \cos \phi}{a + r \sin \phi} \right) \quad (G95)$$

The stress-strain relations are as follows:

$$\epsilon_{rr} = \frac{\sigma_{rr}}{E} - \frac{\nu}{E} \left(\sigma_{\phi\phi} + \sigma_{\theta\theta} \right) + \alpha T \quad (G96)$$

$$\epsilon_{r\phi} = \frac{\sigma_{r\phi}}{2G} \quad (G97)$$

$$\epsilon_{r\theta} = \frac{\sigma_{r\theta}}{2G} \quad (G98)$$

$$\epsilon_{\phi\theta} = \frac{\sigma_{\phi\theta}}{2G} \quad (G99)$$

$$\epsilon_{\phi\phi} = \frac{\sigma_{\phi\phi}}{E} - \frac{\nu}{E} \left(\sigma_{rr} + \sigma_{\theta\theta} \right) + \alpha T \quad (G100)$$

$$\epsilon_{\theta\theta} = \frac{\sigma_{\theta\theta}}{E} - \frac{\nu}{E} \left(\sigma_{rr} + \sigma_{\phi\phi} \right) + \alpha T \quad (G101)$$

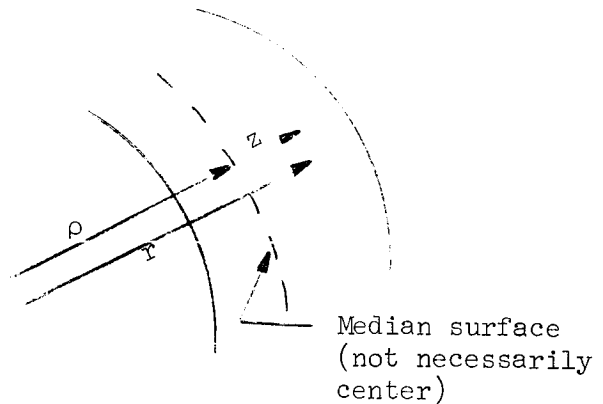
The first-order expansion* is

$$r = \rho + z$$

Thus,

$$\frac{1}{r} = \frac{1}{\rho} - \frac{z}{\rho^2}$$

$$u = u_0 + \frac{z}{\rho} u_1$$



etc., including ν , E , G , α , T since these are variables.

From Equations (G90), (G91), and (G92), we have

$$\epsilon_{rr_0} = \frac{u_1}{\rho} \quad (G102)$$

* Wherever terms are to be differentiated with respect to r and then combined with first-order terms, they must be carried to second-order in $\frac{z}{\rho}$, since the differentiation reduces the order by one.

$$\epsilon_{r\phi}_0 = \frac{v_1}{\rho} - \frac{v_0}{\rho} + \frac{1}{\rho} \frac{\partial u_0}{\partial \phi} \quad (G103)$$

$$\epsilon_{r\theta}_0 = \frac{w_1}{\rho} - \frac{w_0 \sin \phi}{s} - \frac{1}{s} \frac{\partial u_0}{\partial \theta} \quad (s = a + \sin \phi) \quad (G104)$$

Put

$$\epsilon_{\phi\phi} = \epsilon_{\phi\phi}_0 + \frac{z}{\rho} \epsilon_{\phi\phi}_1 \text{ etc.}$$

From Equations (G93), (G94), and (G95)

$$\epsilon_{\phi\phi}_0 = \frac{1}{\rho} \frac{\partial v_0}{\partial \phi} + \frac{u_0}{\rho} \quad (G105)$$

$$\epsilon_{\phi\phi}_1 = \frac{1}{\rho} \frac{\partial v_1}{\partial \phi} - \frac{1}{\rho} \frac{\partial v_0}{\partial \phi} + \frac{u_1}{\rho} - \frac{u_0}{\rho} \quad (G106)$$

$$\epsilon_{\theta\theta}_0 = \frac{1}{s} \left(- \frac{\partial w_0}{\partial \theta} + u_0 \sin \phi + v_0 \cos \phi \right) \quad (G107)$$

$$\begin{aligned} \epsilon_{\theta\theta}_1 = \frac{1}{s} \left(- \frac{\partial w_1}{\partial \theta} + u_1 \sin \phi + v_1 \cos \phi \right) \\ - \frac{\rho \sin \phi}{s^2} \left(- \frac{\partial w_0}{\partial \theta} + u_0 \sin \phi + v_0 \cos \phi \right) \end{aligned} \quad (G108)$$

$$\epsilon_{\phi\theta}_0 = \frac{1}{\rho} \frac{\partial w_0}{\partial \phi} - \frac{w_0 \cos \phi}{s} - \frac{1}{s} \frac{\partial v_0}{\partial \theta} \quad (G109)$$

$$\begin{aligned} \epsilon_{\phi\theta}_1 = \frac{1}{\rho} \left(\frac{\partial w_1}{\partial \phi} - \frac{\partial w_0}{\partial \phi} \right) - \frac{1}{s} \left(\frac{\partial v_1}{\partial \theta} + w_1 \cos \theta \right) \\ + \frac{\rho \sin \phi}{s^2} \left(\frac{\partial v_0}{\partial \theta} + w_0 \cos \phi \right) \end{aligned} \quad (G110)$$

From Equations (G96), (G97), (G98), (G102), (G103), and (G104) we

get

$$u_1 = \rho \left[\frac{\sigma_{rr}_0}{E_0} - \frac{v_0}{E_0} \left(\sigma_{\phi\phi}_0 + \sigma_{\theta\theta}_0 \right) + \alpha_0 T_0 \right] \quad (G111)$$

$$v_1 = v_0 - \frac{\partial u_0}{\partial \phi} + \rho \frac{\sigma_{r\phi}_0}{G_0} \quad (G112)$$

$$w_1 = \frac{\rho}{s} \left(w_0 \sin \phi + \frac{\partial u_0}{\partial \theta} \right) + \rho \frac{\sigma_{r\theta_0}}{G_0} \quad (G113)$$

For boundary stresses, second-order terms are needed because of later differentiations.

$$\sigma_{rr} = \sigma_{rr_0} + \frac{z}{\rho} \sigma_{rr_1} + \left(\frac{z}{\rho} \right)^2 \sigma_{rr_2}$$

At upper and lower surfaces $\left(\pm \frac{h}{2} \right)$, we have the pressures

$$- p_2 = \sigma_{rr_0} + \frac{h}{2\rho} \sigma_{rr_1} + \left(\frac{h}{2\rho} \right)^2 \sigma_{rr_2} \quad (\text{for upper surface})$$

$$- p_1 = \sigma_{rr_0} - \frac{h}{2\rho} \sigma_{rr_1} + \left(\frac{h}{2\rho} \right)^2 \sigma_{rr_2} \quad (\text{for lower surface})$$

Thus,

$$\sigma_{rr_1} = (p_1 - p_2) \frac{\rho}{h} \quad (G114)$$

$$\sigma_{rr_0} = - \left(\frac{p_1 + p_2}{2} \right) - \left(\frac{h}{2\rho} \right)^2 \sigma_{rr_2} \quad (G115)$$

For $\sigma_{r\phi}$, $\sigma_{r\theta}$, in terms of the upper and lower shear stresses, it is seen that

$$\sigma_{r\phi_1} = \left(\tau_{\phi_1} - \tau_{\phi_2} \right) \frac{\rho}{h} \quad (G116)$$

$$\sigma_{r\theta_1} = \left(\tau_{\theta_1} - \tau_{\theta_2} \right) \frac{\rho}{h} \quad (G117)$$

$$\sigma_{r\phi_0} = - \frac{(\tau_{\phi_1} + \tau_{\phi_2})}{2} - \left(\frac{h}{2\rho} \right)^2 \sigma_{r\phi_2} \quad (G118)$$

$$\sigma_{r\theta_0} = - \frac{(\tau_{\theta_1} + \tau_{\theta_2})}{2} - \left(\frac{h}{2\rho} \right)^2 \sigma_{r\theta_2} \quad (G119)$$

To put $\sigma_{\phi\phi_0} + \sigma_{\theta\theta_0}$ in Equation (G111) in terms of displacements, use Equations (G100) and (G101); thus

$$\epsilon_{\phi\phi}_o + \epsilon_{\theta\theta}_o = \frac{1 - v_o}{E_o} \left(\sigma_{\phi\phi}_o + \sigma_{\theta\theta}_o \right) - \frac{2v_o}{E_o} \sigma_{rr}_o + 2\alpha_o T_o$$

Using Equations (G105) and (G107), we have then

$$\begin{aligned} \frac{1 - v_o}{E_o} \left(\sigma_{\phi\phi}_o + \sigma_{\theta\theta}_o \right) &= \frac{1}{\rho} \frac{\partial v_o}{\partial \phi} + \frac{u_o}{\rho} - \frac{1}{s} \frac{\partial w_o}{\partial \theta} \\ &+ \frac{u_o \sin \phi + v_o \cos \phi}{s} + \frac{2v_o}{E_o} \sigma_{rr}_o - 2\alpha_o T_o \end{aligned} \quad (G120)$$

Then by putting Equation (G120) into Equation (G111), we get

$$\begin{aligned} u_1 &= - \frac{v_o}{1 - v_o} \left[\left(\frac{\partial v_o}{\partial \phi} + u_o \right) + \frac{\rho}{s} \left(- \frac{\partial w_o}{\partial \theta} + u_o \sin \phi + v_o \cos \phi \right) \right] \\ &+ \frac{\rho}{E_o} \frac{(1 - 2v_o)(1 + v_o)}{(1 - v_o)} \sigma_{rr}_o + \rho \left(\frac{1 + v_o}{1 - v_o} \right) \alpha_o T_o \end{aligned} \quad (G121)$$

Equations of equilibrium are as follows:

$$\frac{\partial \sigma_{rr}}{\partial r} + \frac{1}{r} \frac{\partial \sigma_{r\phi}}{\partial \phi} - \frac{1}{a + r \sin \phi} \frac{\partial \sigma_{r\theta}}{\partial \theta} = 0$$

$$\frac{\partial \sigma_{r\phi}}{\partial r} + \frac{1}{r} \frac{\partial \sigma_{\phi\phi}}{\partial \phi} - \frac{1}{a + r \sin \phi} \frac{\partial \sigma_{\phi\theta}}{\partial \theta} = 0$$

$$\frac{\partial \sigma_{r\theta}}{\partial r} + \frac{1}{r} \frac{\partial \sigma_{\phi\theta}}{\partial \phi} - \frac{1}{a + r \sin \phi} \frac{\partial \sigma_{\theta\theta}}{\partial \theta} = 0$$

These result in six equations in zero-order and first-order terms; thus

$$\sigma_{rr_1} + \frac{\partial \sigma_{r\phi}_o}{\partial \phi} - \frac{\rho}{s} \frac{\partial \sigma_{r\theta}_o}{\partial \theta} = 0 \quad (G122)$$

$$\sigma_{r\phi_1} + \frac{\partial \sigma_{\phi\phi}_o}{\partial \phi} - \frac{\rho}{s} \frac{\partial \sigma_{\phi\theta}_o}{\partial \theta} = 0 \quad (G123)$$

$$\sigma_{r\theta_1} + \frac{\partial \sigma_{\phi\theta}_o}{\partial \phi} - \frac{\rho}{s} \frac{\partial \sigma_{\theta\theta}_o}{\partial \theta} = 0 \quad (G124)$$

$$2 \sigma_{rr_2} + \frac{\partial \sigma_{r\phi_1}}{\partial \phi} - \frac{\rho}{s} \frac{\partial \sigma_{re_1}}{\partial \theta} - \frac{\partial \sigma_{r\phi_0}}{\partial \phi} + \frac{\rho^2 \sin \phi}{s^2} \frac{\partial \sigma_{re_0}}{\partial \theta} = 0 \quad (G125)$$

$$2 \sigma_{r\phi_2} + \frac{\partial \sigma_{\phi\phi_1}}{\partial \phi} - \frac{\rho}{s} \frac{\partial \sigma_{\phi e_1}}{\partial \theta} - \frac{\partial \sigma_{\phi\phi_0}}{\partial \phi} + \frac{\rho^2 \sin \phi}{s^2} \frac{\partial \sigma_{\phi e_0}}{\partial \theta} = 0 \quad (G126)$$

$$2 \sigma_{re_2} + \frac{\partial \sigma_{\phi e_1}}{\partial \phi} - \frac{\rho}{s} \frac{\partial \sigma_{ee_1}}{\partial \theta} - \frac{\partial \sigma_{\phi e_0}}{\partial \phi} + \frac{\rho^2 \sin \phi}{s^2} \frac{\partial \sigma_{ee_0}}{\partial \theta} = 0 \quad (G127)$$

Equations (G99), (G100), and (G101) give $\sigma_{\phi\phi}$, $\sigma_{\phi e}$, σ_{ee} in terms of the strains; thus

$$\sigma_{\phi\phi} = \frac{E}{1 - \nu^2} \left(\epsilon_{\phi\phi} + \nu \epsilon_{ee} \right) - \frac{E\alpha T}{1 - \nu} + \frac{\nu}{1 - \nu} \sigma_{rr} \quad (G128)$$

$$\sigma_{ee} = \frac{E}{1 - \nu^2} \left(\epsilon_{ee} + \nu \epsilon_{\phi\phi} \right) - \frac{E\alpha T}{1 - \nu} + \frac{\nu}{1 - \nu} \sigma_{rr} \quad (G129)$$

$$\sigma_{\phi e} = \frac{E}{1 + \nu} \epsilon_{\phi e} \quad (G130)$$

These relationships give six equations for the quantities $\sigma_{\phi\phi_0}$, $\sigma_{\phi\phi_1}$, etc. The zero-order expressions are

$$\sigma_{\phi\phi_0} = \text{Equation (G128) with all quantities given zero subscript} \\ (\text{e.g., } E_0, \epsilon_{\phi\phi_0}, \alpha_0, \text{ etc.}) \quad (G131)$$

$$\sigma_{ee_0} = \text{Equation (G129) with all quantities given zero subscript} \\ (\text{e.g., } E_0, \epsilon_{\phi\phi_0}, \alpha_0, \text{ etc.}) \quad (G132)$$

$$\sigma_{\phi e_0} = \text{Equation (G130) with all quantities given zero subscript} \\ (\text{e.g., } E_0, \epsilon_{\phi\phi_0}, \alpha_0, \text{ etc.}) \quad (G133)$$

The first-order expressions are much more complicated. Thus,

$$\begin{aligned}
\sigma_{\theta\theta_1} = & \frac{E_o}{1 - v_o^2} \epsilon_{\theta\theta_1} + \frac{E_o v_o}{1 - v_o^2} \epsilon_{\phi\phi_1} + \left[\frac{E_1}{1 - v_o^2} + \frac{2 E_o v_o v_1}{(1 - v_o^2)^2} \right] \epsilon_{\theta\theta_o} \\
& + \left[\frac{E_1 v_o}{1 - v_o^2} + \frac{E_o v_1 (1 + v_o^2)}{(1 - v_o^2)^2} \right] \epsilon_{\phi\phi_o} \\
& - \left[\frac{E_o \alpha_o T_1}{1 - v_o} + \frac{E_o \alpha_1 T_o}{1 - v_o} + \frac{E_1 \alpha_o T_o}{1 - v_o} + \frac{E_o \alpha_o T_o v_1}{(1 - v_o)^2} \right] \\
& + \left[\frac{v_o}{1 - v_o} \right] \sigma_{rr_1} + \frac{v_1}{(1 - v_o)^2} \sigma_{rr_o} \quad (G134)
\end{aligned}$$

$$\begin{aligned}
\sigma_{\phi\phi_1} = & \frac{E_o}{1 - v_o^2} \epsilon_{\phi\phi_1} + \frac{E_o v_o}{1 - v_o^2} \epsilon_{\theta\theta_1} + \left[\frac{E_1}{1 - v_o^2} + \frac{2 E_o v_o v_1}{(1 - v_o^2)^2} \right] \epsilon_{\phi\phi_o} \\
& + \left[\frac{E_1 v_o}{1 - v_o^2} + \frac{E_o v_1 (1 + v_o^2)}{(1 - v_o^2)^2} \right] \epsilon_{\theta\theta_o} \\
& - \left[\frac{E_o \alpha_o T_1}{1 - v_o} + \frac{E_o \alpha_1 T_o}{1 - v_o} + \frac{E_1 \alpha_o T_o}{1 - v_o} + \frac{E_o \alpha_o T_o v_1}{(1 - v_o)^2} \right] \\
& + \left[\frac{v_o}{1 - v_o} \right] \sigma_{rr_1} + \frac{v_1}{(1 - v_o)^2} \sigma_{rr_o} \quad (G135)
\end{aligned}$$

$$\sigma_{\phi\theta_1} = \frac{E_o}{1 + v_o} \epsilon_{\phi\theta_1} + \left[\frac{E_o}{1 + v_o} - \frac{E_o v_1}{(1 + v_o)^2} \right] \epsilon_{\phi\theta_o} \quad (G136)$$

Equations (G112), (G113), and (G121) are substituted into Equations (G106), (G108), and (G110); then Equations (G105) through (G110) are substituted into Equations (G131) through (G136). The resulting equations give $\sigma_{\theta\theta_o}$, $\sigma_{\theta\theta_1}$, $\sigma_{\phi\phi_o}$, $\sigma_{\phi\phi_1}$, $\sigma_{\phi\theta_o}$, $\sigma_{\phi\theta_1}$ in terms of u_o , u_1 , v_o , v_1 , w_o , w_1 , σ_{rr_o} , σ_{rr_1} , $\sigma_{r\theta_o}$, $\sigma_{r\theta_1}$.

Thus,

$$\sigma_{\phi\phi} \text{ is obtained from Equations (G105), (G107), and (G131)} \quad (\text{G137})$$

$$\sigma_{\phi\phi_1} \text{ is obtained from Equations (G105), (G106), (G107), (G108), (G112), (G113), (G121), and (G135).} \quad (\text{G138})$$

$$\sigma_{\theta\theta} \text{ is obtained from Equations (G105), (G107), and (G132)} \quad (\text{G139})$$

$$\sigma_{\theta\theta_1} \text{ is obtained from Equations (G134), (G105), (G106), (G107), (G108), (G112), (G113), and (G121)} \quad (\text{G140})$$

$$\sigma_{\phi\theta} \text{ is obtained from Equations (G109) and (G133)} \quad (\text{G141})$$

$$\sigma_{\phi\theta_1} \text{ is obtained from Equations (G109), (G110), (G112), (G113), and (G136)}$$

When Equations (G137) through (G142) are substituted into Equations (G122) through (G127), the result is six differential equations in the six quantities u_o , v_o , w_o , σ_{rr_2} , $\sigma_{r\phi_2}$, $\sigma_{r\theta_2}$ [σ_{rr_1} , σ_{rr_o} , etc. are already given in terms of surface stresses, Equations (G114) through (G119)]. The corresponding equations for the spherical system are obtained by letting $a = 0$.

The first-order expansions of the tangential stresses are

$$\sigma_{\phi\phi} = \sigma_{\phi\phi_o} + \left(\frac{z}{\rho}\right) \sigma_{\phi\phi_1} \quad (\text{G143})$$

$$\sigma_{\theta\theta} = \sigma_{\theta\theta_o} + \left(\frac{z}{\rho}\right) \sigma_{\theta\theta_1} \quad (\text{G144})$$

$$\sigma_{\phi\theta} = \sigma_{\phi\theta_o} + \left(\frac{z}{\rho}\right) \sigma_{\phi\theta_1} \quad (\text{G145})$$

where $\sigma_{\phi\phi_o}$, $\sigma_{\phi\phi_1}$, $\sigma_{\theta\theta_o}$, $\sigma_{\theta\theta_1}$, $\sigma_{\phi\theta_o}$, and $\sigma_{\phi\theta_1}$ have been derived in Equations (G137) through (G142). Substituting these equations into Equations (G143), (G144), and (G145), the stress equations in toroidal coordinates are obtained in terms of u_o , v_o , w_o , and σ_{rr_2} .

$$\begin{aligned}
\sigma_{\phi\phi} = & c_1 \left[\frac{1}{\rho} \right] \frac{\partial v_o}{\partial \phi} - c_1 \left[\frac{v_o}{s} \right] \frac{\partial w_o}{\partial \theta} + c_1 \left[\frac{v_o \cos \phi}{s} \right] v_o \\
& + c_1 \left[\frac{1}{\rho} + \frac{v_o \sin \phi}{s} \right] u_o - \frac{E_o \alpha_o T_o}{1-v_o} - \left[\frac{v_o}{1-v_o} \right] \left[\left(\frac{p_1+p_2}{2} \right) + \left(\frac{h}{2\rho} \right)^2 \sigma_{rr_2} \right] \\
& + c_2 \left[\frac{z}{\rho^2} \right] \frac{\partial v_o}{\partial \phi} - c_1 \left[\frac{z}{\rho^2} \right] \left[u_o + \frac{\partial^2 u_o}{\partial \phi^2} - \frac{\rho}{G_o} \frac{\partial \sigma_{r\phi_o}}{\partial \phi} \right] - c_3 \left[\frac{z}{\rho s} \right] \frac{\partial w_o}{\partial \theta} \\
& - c_1 \left[\frac{v_o z}{s} \right] \left[\frac{\sin \phi \cos \phi}{s} v_o + \frac{\sin^2 \phi u_o}{s} + \frac{1}{s} \frac{\partial^2 u_o}{\partial \theta^2} + \frac{1}{G_o} \frac{\partial \sigma_{r\theta_o}}{\partial \theta} \right] \\
& + c_3 \left[\frac{z}{\rho} \frac{\cos \phi}{s} \right] v_o + c_1 \left[\frac{v_o z}{\rho} \frac{\cos \phi}{s} \right] \left[v_o - \frac{\partial u_o}{\partial \phi} + \frac{\rho}{G_o} \sigma_{r\phi_o} \right] \\
& + \left[c_2 \left(\frac{z}{\rho^2} \right) + c_3 \left(\frac{z}{\rho} \frac{\sin \phi}{s} \right) \right] u_o \\
& + c_1 \left[\frac{z}{\rho^2} + \frac{v_o z}{\rho} \frac{\sin \phi}{s} \right] \left\{ \frac{-v_o}{1-v_o} \left[\frac{\partial v_o}{\partial \phi} + u_o + \frac{\rho}{s} \left(u_o \sin \phi + v_o \cos \phi - \frac{\partial w_o}{\partial \theta} \right) \right] \right. \\
& \left. + \left[\frac{\rho}{E_o} \frac{(1-2v_o)(1+v_o)}{1-v_o} \right] \sigma_{rr_o} + \left(\frac{1+v_o}{1-v_o} \right) \rho \alpha_o T_o \right\} \\
& - \left(\frac{z}{\rho} \right) c_4 + \left(\frac{z}{h} \right) \left[\frac{v_o}{1-v_o} \right] \left[(p_1-p_2) \right] - \frac{z}{\rho} \left[\frac{v_1}{(1-v_o)^2} \right] \left[\left(\frac{p_1+p_2}{2} \right) + \left(\frac{h}{2\rho} \right)^2 \sigma_{rr_2} \right]
\end{aligned}$$

Zero-order terms

First-order terms

(G146)

$$\begin{aligned}
\sigma_{\theta\theta} = & c_1 \left[\frac{v_o}{\rho} \right] \frac{\partial v_o}{\partial \phi} - c_1 \left[\frac{1}{s} \right] \frac{\partial w_o}{\partial \theta} + c_1 \left[\frac{\cos \phi}{s} \right] v_o \\
& + c_1 \left[\frac{\sin \phi}{s} + \frac{v_o}{\rho} \right] u_o - \frac{E_o \alpha_o T_o}{1-v_o} - \left[\frac{v_o}{1-v_o} \right] \left[\left(\frac{p_1+p_2}{2} \right) + \left(\frac{h}{2\rho} \right)^2 \sigma_{rr_2} \right] \\
& + c_3 \left[\frac{z}{\rho^2} \right] \frac{\partial v_o}{\partial \phi} - c_1 \left[\frac{v_o z}{\rho^2} \right] \left[u_o + \frac{\partial^2 u_o}{\partial \phi^2} - \frac{\rho}{G_o} \frac{\partial \sigma_{r\phi_o}}{\partial \phi} \right] - c_2 \left[\frac{z}{\rho s} \right] \frac{\partial w_o}{\partial \theta} \\
& - c_1 \left[\frac{z}{s} \right] \left[\frac{\sin^2 \phi}{s} u_o + \frac{\rho}{s} \frac{\partial^2 u_o}{\partial \theta^2} + \frac{\rho}{G_o} \frac{\partial \sigma_{r\theta_o}}{\partial \theta} \right] + c_1 \left[\frac{z \cos \phi}{\rho s} \right] \\
& \left[\left(1 - \frac{\rho}{s} \right) v_o - \frac{\partial u_o}{\partial \phi} + \frac{\rho}{G_o} \sigma_{r\phi_o} \right] \\
& + c_2 \left[\frac{z \cos \phi}{\rho s} \right] v_o + c_1 \left[\frac{z \sin \phi}{\rho s} + \frac{v_o z}{\rho^2} \right] \left\{ \frac{-v_o}{1-v_o} \left[\frac{\partial v_o}{\partial \phi} + u_o + \frac{\rho}{s} \left(u_o \sin \phi \right. \right. \right. \\
& \left. \left. + v_o \cos \phi - \frac{\partial w_o}{\partial \theta} \right) \right] + \left[\frac{\rho}{E_o} \frac{(1-2v_o)(1+v_o)}{1-v_o} \right] \sigma_{rr_o} + \left(\frac{1+v_o}{1-v_o} \right) \rho \alpha_o T_o \left. \right\} \\
& + c_2 \left[\frac{\sin \phi}{s} + c_3 \frac{1}{\rho} \right] \left[\frac{z}{\rho} \right] u_o - \left[\frac{z}{\rho} \right] c_4 + \frac{z}{h} \left[\frac{v_c}{1-v_o} \right] \left[(p_1-p_2) \right] \\
& - \frac{z}{\rho} \left[\frac{v_1}{(1-v_o)^2} \right] \left[\left(\frac{p_1+p_2}{2} \right) + \left(\frac{h}{2\rho} \right)^2 \sigma_{rr_2} \right]
\end{aligned}$$

Zero-order terms

First-order terms

(G147)

$$\begin{aligned}
\sigma_{\phi\theta} = & -c_5 \left[\frac{1}{s} \right] \frac{\partial v_o}{\partial \theta} + c_5 \left[\frac{1}{\rho} \right] \frac{\partial w_o}{\partial \phi} - c_5 \left[\frac{\cos \phi}{s} \right] w_o \quad \left. \vphantom{\sigma_{\phi\theta}} \right\} \text{Zero-order terms} \\
& - c_6 \left[\frac{z}{\rho s} \right] \frac{\partial v_o}{\partial \theta} - c_5 \left[\frac{z}{\rho s} \right] \left[\left(1 - \frac{\rho}{s} \sin \phi \right) \frac{\partial v_o}{\partial \theta} - \frac{\partial^2 u_o}{\partial \phi \partial \theta} + \frac{\rho}{G_o} \frac{\partial \sigma_{r\phi_o}}{\partial \theta} \right] \\
& + c_6 \left[\frac{z}{\rho^2} \right] \frac{\partial w_o}{\partial \phi} + c_5 \left[\frac{z}{\rho} \right] \left[\left(\frac{s \cos \phi + (\rho - r) \sin \phi \cos \phi}{s^2} \right) w_o \right. \\
& + \left. \left(\frac{\sin \phi}{s} - \frac{1}{\rho} \right) \frac{\partial w_o}{\partial \phi} + \frac{1}{s} \frac{\partial^2 u_o}{\partial \theta \partial \phi} - \frac{r \cos \phi}{s^2} \frac{\partial u_o}{\partial \theta} + \frac{1}{G_o} \frac{\partial \sigma_{r\theta_o}}{\partial \phi} \right] \\
& - c_6 \left[\frac{z}{\rho s} \cos \phi \right] w_o - c_5 \left[\frac{z}{s} \cos \phi \right] \left[\frac{\sin \phi}{s} w_o + \frac{1}{s} \frac{\partial u_o}{\partial \theta} + \frac{1}{G_o} \sigma_{r\theta_o} \right] \quad \left. \vphantom{\sigma_{\phi\theta}} \right\} \text{First-order terms} \quad (G148)
\end{aligned}$$

where

$$s = a + r \sin \phi \quad (G149)$$

$$c_1 = \frac{E_o}{1 - v_o^2} \quad (G150)$$

$$c_2 = \frac{E_1}{1 - v_o^2} + \frac{2E_o v_o v_1}{(1 - v_o^2)^2} \quad (G151)$$

$$c_3 = \frac{E_1 v_o}{1 - v_o^2} + \frac{E_o v_1 (1 + v_o^2)}{(1 - v_o^2)^2} \quad (G152)$$

$$c_4 = \frac{E_o \alpha_o T_1}{1 - v_o} + \frac{E_o \alpha_1 T_o}{1 - v_o} + \frac{E_1 \alpha_o T_o}{1 - v_o} + \frac{E_o \alpha_o T_o v_1}{(1 - v_o)^2} \quad (G153)$$

$$C_5 = \frac{E_o}{1 + \nu_o} \quad (G154)$$

$$C_6 = \frac{E_1}{1 + \nu_o} - \frac{E_o \nu_1}{(1 + \nu_o)^2} \quad (G155)$$

B. THIN SHELL SOLUTION: FINITE DIFFERENCE FORMULATION

The generalized finite-difference analog formulation is directly applicable to the thin-shell solution with the following changes: coordinate α_1 will be eliminated since it corresponds to the radial direction,* and subscript i will be dropped to conform with the above statement.

The finite-difference solution of the equilibrium equations will first be obtained for the general case where the grid spacing is assumed to be irregular. Then a solution will also be obtained for regular grid spacing. In both cases the first, second, and mixed derivatives are needed for the central, forward, and backward grid combinations. A typical general grid spacing is shown in Figure G5.

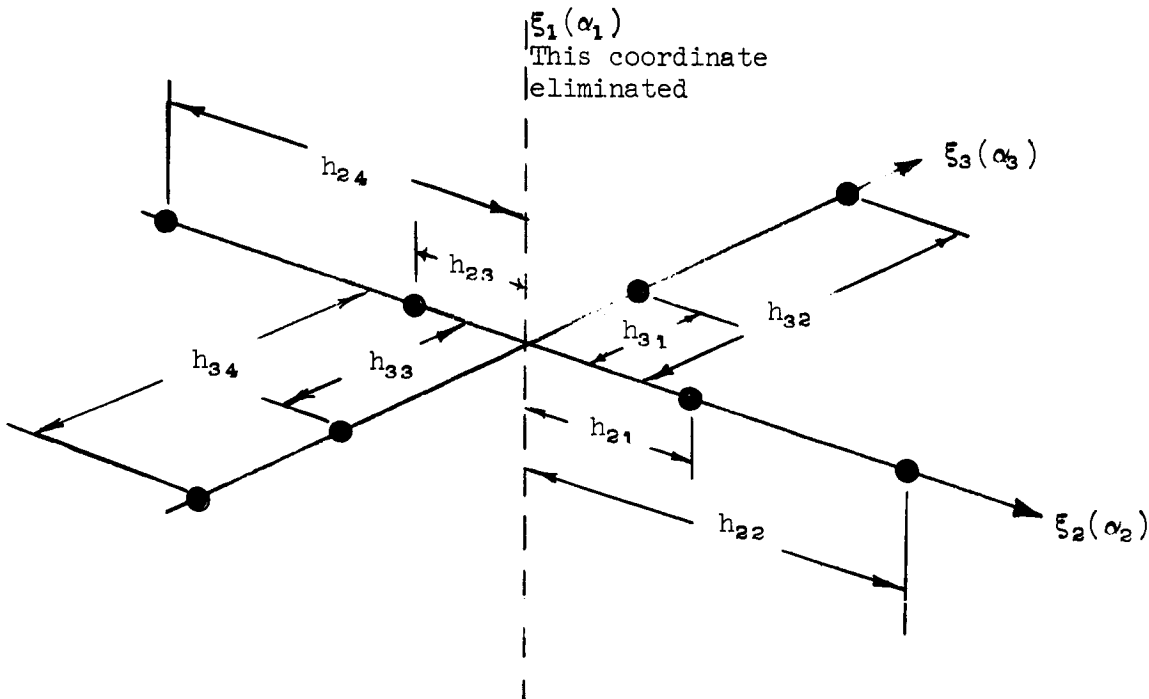


Fig.G5 - Coordinates of Irregular Mesh Intervals

*The partial derivatives in the equilibrium equations given in Equation (G36) are taken with respect to α_2 and α_3 (ϕ and θ , respectively).

The increments in the vicinity of a node will be designated by the following notations, in accordance with Figure G5.

$$\begin{aligned}
 h_{21} &= (\alpha_2)_{i+1} - (\alpha_2)_i & h_{31} &= (\alpha_3)_{i+1} - (\alpha_3)_i \\
 h_{22} &= (\alpha_2)_{i+2} - (\alpha_2)_i & h_{32} &= (\alpha_3)_{i+2} - (\alpha_3)_i \\
 h_{23} &= (\alpha_2)_i - (\alpha_2)_{i-1} & h_{33} &= (\alpha_3)_i - (\alpha_3)_{i-1} \\
 h_{24} &= (\alpha_2)_i - (\alpha_2)_{i-2} & h_{34} &= (\alpha_3)_i - (\alpha_3)_{i-2}
 \end{aligned} \tag{G156}$$

From Equation (G36) it can be seen that $f(\xi_1, \xi_2, \xi_3)$, the function of the coordinates with the origin at i, j, k , is

$$\begin{aligned}
 f(\xi_1, \xi_2, \xi_3) &= f_{i,j,k} + B_1 \xi_1 + B_2 \xi_2 + B_3 \xi_3 + B_4 \xi_1 \xi_2 + B_5 \xi_2 \xi_3 + B_6 \xi_3 \xi_1 \\
 &\quad + B_7 \xi_1^2 + B_8 \xi_2^2 + B_9 \xi_3^2 + B_{10} \xi_1 \xi_2 \xi_3 + B_{11} \xi_1 \xi_2^2 \\
 &\quad + B_{12} \xi_1 \xi_3^2 + B_{13} \xi_1^2 \xi_2 + B_{14} \xi_2 \xi_3^2 + \dots
 \end{aligned}$$

The first and second derivatives of $f(\alpha_1, \alpha_2, \alpha_3)$ with respect to α_2 and α_3 are obtained from Equation (G36).

$$\begin{aligned}
 \left. \frac{\partial f}{\partial \alpha_2} \right|_{i,j,k} &= \left. \frac{\partial f}{\partial \xi_2} \right|_{o,o,o} = B_2, \quad \left. \frac{\partial f}{\partial \alpha_3} \right|_{i,j,k} = \left. \frac{\partial f}{\partial \xi_3} \right|_{o,o,o} = B_3, \\
 \left. \frac{\partial^2 f}{\partial \alpha_2 \partial \alpha_3} \right|_{i,j,k} &= \left. \frac{\partial^2 f}{\partial \xi_2 \partial \xi_3} \right|_{o,o,o} = B_5, \quad \left. \frac{\partial^2 f}{\partial \alpha_2^2} \right|_{i,j,k} = \left. \frac{\partial^2 f}{\partial \xi_2^2} \right|_{o,o,o} = 2B_8, \\
 \left. \frac{\partial^2 f}{\partial \alpha_3^2} \right|_{i,j,k} &= \left. \frac{\partial^2 f}{\partial \xi_3^2} \right|_{o,o,o} = 2B_9
 \end{aligned} \tag{G157}$$

The constants B_i are evaluated in terms of the function at these nodes and the grid spacing as shown in Figure G5 by considering the values of $f(\xi_1, \xi_2, \xi_3)$ at the eight

nodes adjacent to j,k, and proceeding in a manner similar to that outlined on pages G30 and G31.

1. General Case - Irregular Grid Spacing

a. Central Derivatives

$$\left. \frac{\partial f}{\partial \alpha_2} \right|_{j,k} = \frac{h_{23}^2 f_{j+1,k} + (h_{21}^2 - h_{23}^2) f_{j,k} - h_{21}^2 f_{j-1,k}}{h_{21} h_{23} (h_{21} + h_{23})} \quad (G158)$$

$$\left. \frac{\partial f}{\partial \alpha_3} \right|_{j,k} = \frac{h_{33}^2 f_{j,k+1} + (h_{31}^2 - h_{33}^2) f_{j,k} - h_{31}^2 f_{j,k-1}}{h_{31} h_{33} (h_{31} + h_{33})} \quad (G159)$$

$$\begin{aligned} \left. \frac{\partial^2 f}{\partial \alpha_2 \partial \alpha_3} \right|_{j,k} = & \frac{1}{h_{31} h_{33} (h_{21} + h_{23})(h_{31} + h_{33})} \left[h_{33}^2 (f_{j+1,k+1} - f_{j-1,k+1}) \right. \\ & \left. - (h_{33}^2 - h_{31}^2)(f_{j+1,k} - f_{j-1,k}) - h_{31}^2 (f_{j+1,k-1} - f_{j-1,k-1}) \right] \quad (G160) \end{aligned}$$

$$\begin{aligned} \left. \frac{\partial^2 f}{\partial \alpha_2 \partial \alpha_3} \right|_{j,k} = & \frac{1}{h_{21} h_{23} (h_{21} + h_{23})(h_{31} + h_{33})} \left[h_{23}^2 (f_{j+1,k+1} - f_{j+1,k-1}) \right. \\ & \left. - (h_{23}^2 - h_{21}^2)(f_{j,k+1} - f_{j,k-1}) - h_{21}^2 (f_{j-1,k+1} - f_{j-1,k-1}) \right] \quad (G161) \end{aligned}$$

$$\left. \frac{\partial^2 f}{\partial \alpha_2^2} \right|_{j,k} = \frac{2 [h_{23}^2 f_{j+1,k} - (h_{21} + h_{23}) f_{j,k} + h_{21}^2 f_{j-1,k}]}{h_{21} h_{23} (h_{21} + h_{23})} \quad (G162)$$

$$\left. \frac{\partial^2 f}{\partial \alpha_3^2} \right|_{j,k} = \frac{2 [h_{33}^2 f_{j,k+1} - (h_{31} + h_{33}) f_{j,k} + h_{31}^2 f_{j,k-1}]}{h_{31} h_{33} (h_{31} + h_{33})} \quad (G163)$$

b. Forward Derivatives

$$\left. \frac{\partial f}{\partial \alpha_2} \right|_{j,k} = \frac{h_{22}^2 f_{j+1,k} - (h_{22}^2 - h_{21}^2) f_{j,k} - h_{21}^2 f_{j+2,k}}{h_{21} h_{22} (h_{22} - h_{21})} \quad (G164)$$

$$\left. \frac{\partial f}{\partial \alpha_3} \right|_{j,k} = \frac{h_{32}^2 f_{j,k+1} - (h_{32}^2 - h_{31}^2) f_{j,k} - h_{31}^2 f_{j,k+2}}{h_{31} h_{32} (h_{32} - h_{31})} \quad (G165)$$

$$\left. \frac{\partial^2 f}{\partial \alpha_2 \partial \alpha_3} \right|_{j,k} = \frac{1}{h_{31} h_{33} (h_{21} - h_{22})(h_{31} + h_{33})} \left[h_{33}^2 (f_{j+1,k+1} - f_{j+1,k} - f_{j+2,k+1} + f_{j+2,k}) - h_{31}^2 (f_{j+1,k-1} - f_{j+1,k} - f_{j+2,k-1} + f_{j+2,k}) \right] \quad (G166)$$

$$\left. \frac{\partial^2 f}{\partial \alpha_2 \partial \alpha_3} \right|_{j,k} = \frac{1}{h_{21} h_{23} (h_{31} - h_{32})(h_{21} + h_{23})} \left[h_{23}^2 (f_{j+1,k+1} - f_{j,k+1} - f_{j+1,k+2} + f_{j,k+2}) - h_{21}^2 (f_{j-1,k+1} - f_{j,k+1} - f_{j-1,k+2} + f_{j,k+2}) \right] \quad (G167)$$

$$\left. \frac{\partial^2 f}{\partial \alpha_2^2} \right|_{j,k} = \frac{2 \left[-h_{22} f_{j+1,k} + (h_{22} - h_{21}) f_{j,k} + h_{21} f_{j+2,k} \right]}{h_{21} h_{22} (h_{22} - h_{21})} \quad (G168)$$

$$\left. \frac{\partial^2 f}{\partial \alpha_3^2} \right|_{j,k} = \frac{2 \left[-h_{32} f_{j,k+1} + (h_{32} - h_{31}) f_{j,k} + h_{31} f_{j,k+2} \right]}{h_{31} h_{32} (h_{32} - h_{31})} \quad (G169)$$

c. Backward Derivatives

$$\left. \frac{\partial f}{\partial \alpha_2} \right|_{j,k} = \frac{h_{23} f_{j-2,k} + (h_{24}^2 - h_{23}^2) f_{j,k} - h_{24}^2 f_{j-1,k}}{h_{23} h_{24} (h_{24} - h_{23})} \quad (G170)$$

$$\left. \frac{\partial f}{\partial \alpha_3} \right|_{j,k} = \frac{h_{33}^2 f_{j,k-2} + (h_{34}^2 - h_{33}^2) f_{j,k} - h_{34}^2 f_{j,k-1}}{h_{33} h_{34} (h_{34} - h_{33})} \quad (G171)$$

$$\left. \frac{\partial^2 f}{\partial \alpha_2 \partial \alpha_3} \right|_{j,k} = \frac{1}{h_{31} h_{33} (h_{23} - h_{24})(h_{31} + h_{33})} \left[h_{33}^2 (f_{j-1,k+1} - f_{j-2,k+1} \right. \\ \left. + f_{j-2,k} - f_{j-1,k}) - h_{31}^2 (f_{j-1,k-1} - f_{j-1,k} - f_{j-2,k-1} + f_{j-2,k}) \right] \quad (G172)$$

$$\left. \frac{\partial^2 f}{\partial \alpha_2 \partial \alpha_3} \right|_{j,k} = \frac{1}{h_{21} h_{23} (h_{33} - h_{34})(h_{21} + h_{23})} \left[h_{23}^2 (f_{j+1,k-1} - f_{j+1,k-2} \right. \\ \left. + f_{j,k-2} - f_{j,k-1}) - h_{21}^2 (f_{j-1,k-1} - f_{j,k-1} - f_{j-1,k-2} + f_{j,k-2}) \right] \quad (G173)$$

$$\left. \frac{\partial^2 f}{\partial \alpha_2^2} \right|_{j,k} = \frac{2 \left[h_{23} f_{j-2,k} + (h_{24} - h_{23}) f_{j,k} - h_{24} f_{j-1,k} \right]}{h_{23} h_{24} (h_{24} - h_{23})} \quad (G174)$$

$$\left. \frac{\partial^2 f}{\partial \alpha_3^2} \right|_{j,k} = \frac{2 \left[h_{33} f_{j,k-2} + (h_{34} - h_{33}) f_{j,k} - h_{34} f_{j,k-1} \right]}{h_{33} h_{34} (h_{34} - h_{33})} \quad (G175)$$

2. General Case - Regular Grid Spacing

When the grid spacing is regular, then

$$h_2 = h_{21} = h_{23} = \frac{1}{2} h_{22} = \frac{1}{2} h_{24} \quad (G176)$$

$$h_3 = h_{31} = h_{33} = \frac{1}{2} h_{32} = \frac{1}{2} h_{34}$$

Substituting the conditions of Equation (G176) into Equations (G158) through (G175) the first, second, and mixed derivatives for the central, forward, and backward regular grid spacing combinations are obtained.

a. Central Derivatives

$$\left. \frac{\partial f}{\partial \alpha_2} \right|_{j,k} = \frac{f_{j+1,k} - f_{j-1,k}}{2h_2} \quad (G177)$$

$$\left. \frac{\partial f}{\partial \alpha_3} \right|_{j,k} = \frac{f_{j,k+1} - f_{j,k-1}}{2h_3} \quad (G178)$$

$$\left. \frac{\partial^2 f}{\partial \alpha_2 \partial \alpha_3} \right|_{j,k} = \frac{1}{4h_2 h_3} \left[f_{j+1,k+1} - f_{j+1,k-1} - f_{j-1,k+1} + f_{j-1,k-1} \right] \quad (G179)$$

$$\left. \frac{\partial^2 f}{\partial \alpha_2^2} \right|_{j,k} = \frac{1}{h_2^2} \left[f_{j+1,k} - 2f_{j,k} + f_{j-1,k} \right] \quad (G180)$$

$$\left. \frac{\partial^2 f}{\partial \alpha_3^2} \right|_{j,k} = \frac{1}{h_3^2} \left[f_{j,k+1} - 2f_{j,k} + f_{j,k-1} \right] \quad (G181)$$

b. Forward Derivatives

$$\left. \frac{\partial f}{\partial \alpha_2} \right|_{j,k} = \frac{1}{2h_2} \left[4f_{j+1,k} - 3f_{j,k} - f_{j+2,k} \right] \quad (G182)$$

$$\left. \frac{\partial f}{\partial \alpha_3} \right|_{j,k} = \frac{1}{2h_3} \left[4f_{j,k+1} - 3f_{j,k} - f_{j,k+2} \right] \quad (G183)$$

$$\left. \frac{\partial^2 f}{\partial \alpha_2 \partial \alpha_3} \right|_{j,k} = \frac{-1}{2h_2 h_3} \left[f_{j+1,k+1} - f_{j+2,k+1} - f_{j+1,k-1} + f_{j+2,k-1} \right] \quad (G184)$$

$$\left. \frac{\partial^2 f}{\partial \alpha_2 \partial \alpha_3} \right|_{j,k} = \frac{-1}{2h_2 h_3} \left[f_{j+1,k+1} - f_{j+1,k+2} - f_{j-1,k+1} + f_{j-1,k+2} \right] \quad (G185)$$

$$\left. \frac{\partial^2 f}{\partial \alpha_2^2} \right|_{j,k} = \frac{1}{h_2^2} \left[-2f_{j+1,k} + f_{j,k} + f_{j+2,k} \right] \quad (G186)$$

$$\left. \frac{\partial^2 f}{\partial \alpha_3^2} \right|_{j,k} = \frac{1}{h_3^2} \left[-2f_{j,k+1} + f_{j,k} + f_{j,k+2} \right] \quad (G187)$$

C. Backward Derivatives

$$\left. \frac{\partial f}{\partial \alpha_2} \right|_{j,k} = \frac{1}{2h_2} \left[f_{j-2,k} + 3f_{j,k} - 4f_{j-1,k} \right] \quad (G188)$$

$$\left. \frac{\partial f}{\partial \alpha_3} \right|_{j,k} = \frac{1}{2h_3} \left[f_{j,k-2} + 3f_{j,k} - 4f_{j,k-1} \right] \quad (G189)$$

$$\left. \frac{\partial^2 f}{\partial \alpha_2 \partial \alpha_3} \right|_{j,k} = \frac{-1}{2h_2 h_3} \left[f_{j-1,k+1} - f_{j-2,k+1} - f_{j-1,k-1} + f_{j-2,k-1} \right] \quad (G190)$$

$$\left. \frac{\partial^2 f}{\partial \alpha_2 \partial \alpha_3} \right|_{j,k} = \frac{-1}{2h_2 h_3} \left[f_{j+1,k-1} - f_{j+1,k-2} - f_{j-1,k-1} + f_{j-1,k-2} \right] \quad (G191)$$

$$\left. \frac{\partial^2 f}{\partial \alpha_3^2} \right|_{j,k} = \frac{1}{h_3^2} \left[f_{j,k-2} + f_{j,k} - 2f_{j,k-1} \right] \quad (G193)$$

V. OVERALL THREE-DIMENSIONAL BOUNDARY CONDITIONS

Given the notation and equations previously developed, and with Figures G6a, G6b, and G6c illustrating the geometry involved, the following described boundary and compatibility conditions are those which will govern the solution to the full three-dimensional heat shield problem for the two significant cases ("fixed" and "free-free" conditions, respectively, at the structural juncture surface).

A. On inside and outside boundary surfaces OB and IB for both cases:

$$\tau_{rr} = \tau_{r\phi} = \tau_{r\theta} = 0$$

B. On interface surfaces I1 and I2 for both cases (index denotes which material medium is indicated):

$$\left. \begin{aligned} [\tau_{rr}]_{i+1} &= [\tau_{rr}]_i \\ [\tau_{r\phi}]_{i+1} &= [\tau_{r\phi}]_i \\ [\tau_{r\theta}]_{i+1} &= [\tau_{r\theta}]_i \end{aligned} \right\} \quad i = 1, 2$$

C. In the $r - \phi$ plane for which $\theta = 0^\circ$ for both cases:

$$w = \frac{\partial^2 w}{\partial \theta^2} = \frac{\partial u}{\partial \theta} = \frac{\partial v}{\partial \theta} = 0$$

D. In the $r - \phi$ plane for which $\theta = 90^\circ$ for both cases:

$$u(r, \phi, 90^\circ) = u(r, -\phi, 90^\circ)$$

$$v(r, \phi, 90^\circ) = v(r, -\phi, 90^\circ)$$

$$w(r, \phi, 90^\circ) = -w(r, -\phi, 90^\circ)$$

$$\left[\frac{\partial u}{\partial \theta} \right]_{\phi=\phi_1} = \left[- \frac{\partial u}{\partial \theta} \right]_{\phi=-\phi_1}$$

$$\left[\frac{\partial v}{\partial \theta} \right]_{\phi=\phi_1} = \left[- \frac{\partial v}{\partial \theta} \right]_{\phi=-\phi_1}$$

$$\left[\frac{\partial w}{\partial \theta} \right]_{\phi=\phi_1} = \left[- \frac{\partial w}{\partial \theta} \right]_{\phi=-\phi_1}$$

E. On the structural juncture plane (SJ) for the "fixed" case:

$$u = v = w = 0$$

F. On the structural juncture plane (SJ) for the "free-free" case:

$$\tau_{\phi\phi} = \tau_{r\phi} = \tau_{\theta\phi} = 0$$

G. Along the circles at the intersections between the structural juncture plane and each interface surface for the "free-free" case (IP1, IP2), the conditions of Condition F are replaced by (index denotes which material medium is indicated):

$$\left[\frac{\partial v}{\partial r} \right]_{i+1} = \left[\frac{\partial v}{\partial r} \right]_i$$

$$\left[\frac{\partial w}{\partial r} \right]_{i+1} = \left[\frac{\partial w}{\partial r} \right]_i$$

$$\begin{aligned} \left[\frac{\partial u}{\partial r} \right]_{i+1} - \left[\frac{\partial u}{\partial r} \right]_i + \left(\frac{2}{r} \right) \left\{ \left[\frac{\mu}{\lambda} \right]_{i+1} - \left[\frac{\mu}{\lambda} \right]_i \right\} \left(u + \frac{\partial v}{\partial \phi} \right) \\ + \left[\left(3 + \frac{2\mu}{\lambda} \right) \int_{T_0}^T \alpha dT \right]_{i+1} - \left[\left(3 + \frac{2\mu}{\lambda} \right) \int_{T_0}^T \alpha dT \right]_i = 0 \end{aligned} \quad i = 1, 2$$

H. Along the circles at the intersections between the structural juncture plane and each of the boundary surfaces OB and IB for the "free-free" case (PI, PO), the normal stress condition ($\tau_{\phi\phi} = 0$) of Condition F is replaced by

$$u = r \frac{\partial u}{\partial r} - \frac{\partial v}{\partial \phi}$$

AB is edge view of plane
of temperature and thickness
distributional symmetry.

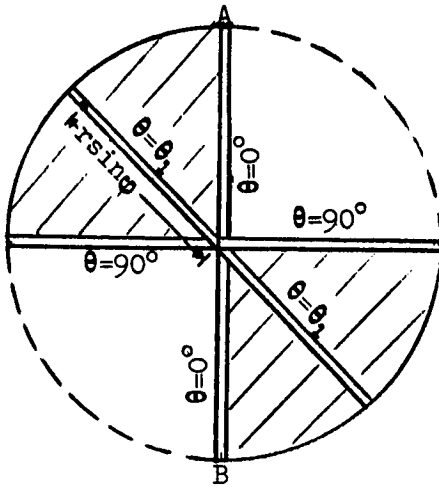


Fig. G6a - Front View of Shield

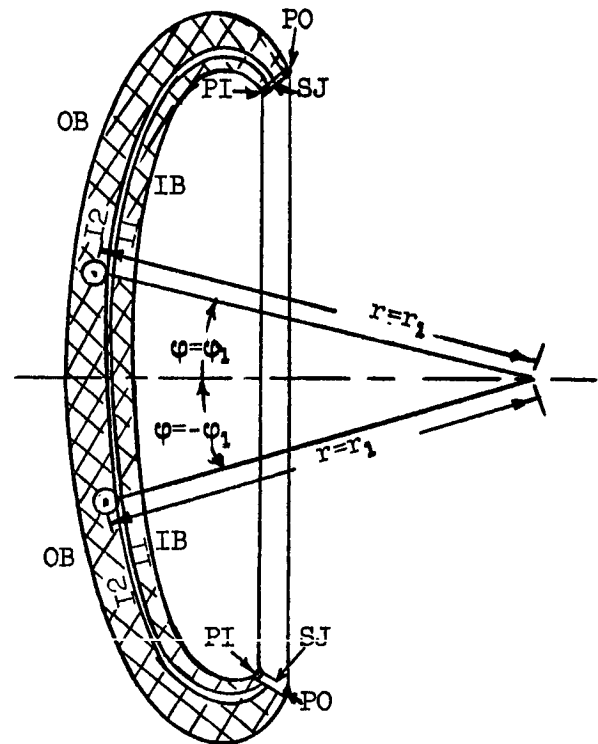


Fig. G6b - Cross Section of Shield

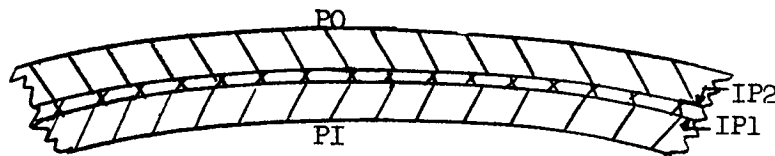


Fig. G6c - Portion of Structural Juncture Surface SJ

Definitions:

- r - Radial direction (both sphere and torus)
- IB - Inside boundary surface
- OB - Outside boundary surface
- SJ - Structural juncture plane
- IP1, IP2 - Intersections between the structural juncture plane and each surface interface
- PI, PO - Intersections between the structural juncture plane and each of the boundary surfaces OB and IB

VI. PROGRAMING OF THE FINITE-DIFFERENCE MODEL(S) AND ATTEMPTS TO SOLVE THE EQUATIONS BY THE OVER-RELAXATION APPROACH AND BY DIRECT MATRIX INVERSION

A. OVER-RELAXATION APPROACH

The first approach in an attempt to solve the displacement and stress equations was point relaxation. The reason for this choice was the apparent success of this method in the previous work by Morgan and Christensen. Repeated attempts with different values of the over-relaxation experiments were not successful. Evidently the reason for the difficulty in point relaxation was the incapability of this method to bring in the effects of boundary conditions.

An alternative form (i.e., line relaxation) was then attempted. The first trial with line relaxation was made utilizing a radial line to the boundary. Repeated attempts were made with this method with various boundary conditions. It was demonstrated that this method is also incapable of meeting the remaining boundary conditions. A "long line" was then attempted. It was again demonstrated that all boundary conditions could not be met simultaneously; the solution diverged as it proceeded radially outward.

The basic difficulty in these methods seems to lie in the formulation of an acceptable and consistent system of boundary conditions. Similar difficulties have been reported in the literature for very much simpler cases.

B. DIRECT MATRIX INVERSION

A second approach (i.e., direct matrix inversion) was then attempted. Due to the relatively small size of the computer memory, the mesh size was too large to achieve a successful solution. However, direct matrix inversion cannot be ruled out as a method for the solution of this problem. It should be noted that the very short time allotted to the attempts at the solution using relaxation and direct inversion methods did not allow a complete exploration of these procedures.

VII. ALTERNATIVE METHODS AND RECOMMENDATIONS

Additional methods for the solution of the complete nonaxisymmetric case were investigated. It was determined that the possibility exists for a direct matrix inversion solution of the whole problem provided machine language is used throughout for the programing. An additional memory capability utilizing magnetic tape or memory disks would be employed.

A second approach was also investigated which would utilize equivalent analog circuits to transform the equations into a set, the behavior of which is well known. This approach seems to offer another possibility of solving the complete three-dimensional nonaxisymmetric problem.